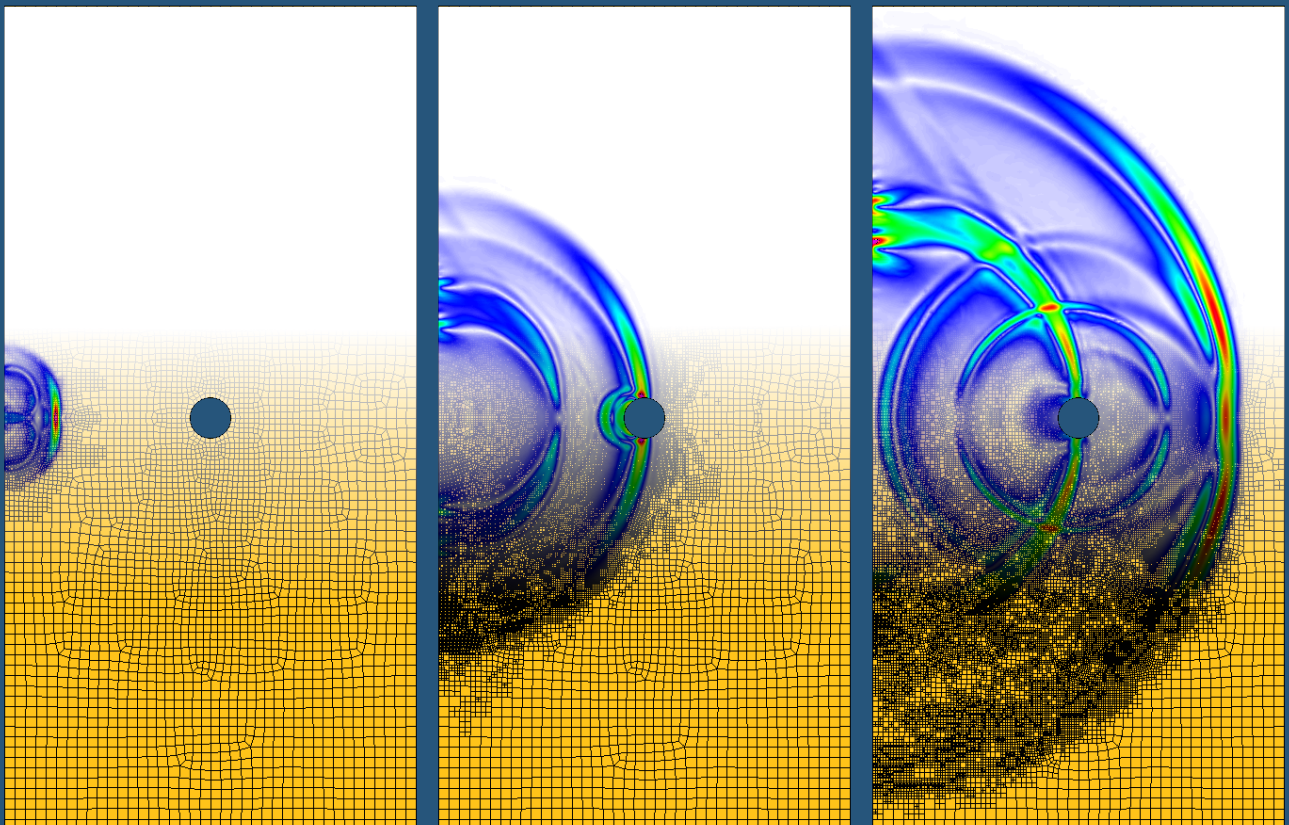


# ERROR ASSESSMENT AND ADAPTIVITY FOR STRUCTURAL TRANSIENT DYNAMICS

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Doctoral Thesis  
Barcelona, October 2013



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Advisors: Pedro Díez and Núria Parés

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## ABSTRACT

The finite element method is a valuable tool for simulating complex physical phenomena. However, any finite element based simulation has an intrinsic amount of error with respect to the exact solution of the selected physical model. Being aware of this error is of notorious importance if sensitive engineering decisions are taken on the basis of the numerical results. Assessing the error in elliptic problems (as structural statics) is a well known problem. However, assessing the error in structural transient dynamics is still ongoing research.

The present thesis aims at contributing on error assessment techniques for structural transient dynamics. First, a new approach is introduced to compute bounds of the error measured in some quantity of interest. The proposed methodology yields error bounds with better quality than the already available approaches. Second, an efficient methodology to compute approximations of the error in the quantity of interest is introduced. The proposed technique uses modal analysis to compute the solution of the adjoint problem associated with the selected quantity of interest. The resulting error estimate is very well suited for time-dependent problems, because the cost of computing the estimate at each time step is very low. Third, a space-time adaptive strategy is proposed. The local error indicators driving the adaptive process are computed using the previously mentioned modal-based error estimate. The resulting adapted approximations are more accurate than the ones obtained with a straightforward uniform mesh refinement. That is, the adapted computations lead to lower errors in the quantity of interest than the non-adapted ones for the same number of space-time elements. Fourth, a new type of quantities of interest are introduced for error assessment in time-dependent problems. These quantities (referred as timeline-dependent quantities of interest) are scalar time-dependent outputs of the transient solution and are better suited to time-dependent problems than the standard scalar ones. The error in timeline-dependent quantities is efficiently assessed using the modal-based description of the adjoint solution.

The thesis contributions are enclosed in five papers which are attached to the thesis document.

## KEYWORDS

Structural dynamics · finite element method · error assessment · error bounds  
space-time adaptivity · quantity of interest · adjoint problem · modal analysis



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# Thesis overview

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## 1 Introduction

### 1.1 Motivation

Finite element-based simulations have become a fundamental tool in engineering analysis. These techniques are valuable to simulate physical phenomena when experiments are too expensive or even impracticable. However, any finite element approximation has two inherent sources of error, the *modeling* error and the *discretization* error. Consequently, both errors have to be controlled to provide a reliable numerical solution. This is particularly important if sensitive decisions are taken on the basis of the numerical results.

The modeling error is the difference between *reality* and the selected mathematical model. The mathematical model is typically described by a set of partial differential equations (plus suitable initial and boundary conditions) which are approximated with numerical tools. The discretization error is the difference between the unknown *exact* solution of the mathematical model and the *computable* numerical approximation. The present work restricts to assessing the discretization error. Thus, the exact solution of the mathematical model is taken as the *truth* solution in the present error analysis. Assessing the modeling error is out of the scope of this work.

The numerical approximation is associated with a *discretization* of the computational domain based on a *computational mesh* or grid. Consequently, the choice of a *good enough* mesh is crucial to have a reliable solution. In practice, the quality of the computational mesh is bounded by the available computer resources, and therefore, a compromise between cost and quality has to be found.

*A posteriori* error assessment techniques allow the user of the finite element soft-

ware to be aware of which is the error associated with the selected discretization. This information is used to accept or reject the numerical approximation on the basis of the required accuracy for a specific application. Moreover, error assessment techniques provide local error information describing the distribution of the error over the computational domain. This information is used to build an *adapted* discretization. A fine mesh is employed only at the zones with large error contribution, making an efficient use of the computational resources.

This work focuses in structural transient dynamics. That is, structural problems under impulsive loads exiting a high frequency spectrum. In this situation, the solution of the problem is typically approximated using direct time-integration schemes instead of modal analysis, because the required number of vibration modes to characterize the solution is very high. The applications of structural transient dynamics include a high variety of elastic wave propagation problems. For instance, vulnerability of structures under explosions or impacts, or applications in geophysics as the propagation of earthquake waves.

Assessing the error in structural dynamics is particularly relevant because, as compared to the standard elliptic problems, the discretization errors are generated and propagated less intuitively or predictably. Moreover, the applications of structural transient dynamics include safety-related issues in buildings and large infrastructures. Consequently, a quality certification of the numerical solution is important in this context.

Nowadays, the error assessment and adaptive techniques are well established for elliptic problems as steady-state linear elasticity and heat transfer. However, these methodologies are still under development for structural transient dynamics (also referred as *elastodynamics*) and other second-order hyperbolic problems. Assessing the error in structural dynamics is a challenging topic. First, the available error estimates are expensive in terms of CPU time and memory requirements. Second, computing sharp error bounds is specially demanding using the available techniques. And third, the quantities of interest available for assessing the error are not particularly well suited for time-dependent problems. These difficulties compromise the application of the current error assessment techniques in practical engineering examples. Consequently, further research is required to overcome these difficulties.

## 1.2 State-of-the-art

Over the last three decades, numerous a posteriori error assessment techniques for finite element analysis have been proposed. Representative state-of-the-art reviews and books on this topic are introduced by Ainsworth and Oden (2000, 1997), Ladevèze and Pelle (2001), Rannacher (2001), Stein (2003), Gratsch and Bathe (2005), and Díez et al. (2010). These works focus mainly in elliptic problems and do not include the last advances particular to structural transient dynamics. A detailed review of the available error assessment techniques for structural transient dynamics is presented in the appended paper A. The state-of-the-art review presented in this introduction is a succinct version of the one found in paper A.

A posteriori error assessment techniques aim at assessing a particular error measure (i.e. a representative scalar value associated with the error) instead of approximating the error field at each point of the computational domain. Approximating the error field generally requires computing a reference solution using a much finer discretization, which is unaffordable in practice. Two different types of error measures are considered in the literature leading to two different types of error estimates: 1) energy-like error estimates and 2) goal-oriented error estimates.

### 1) Energy-like error estimates (also referred as *global* error estimates)

The error measure is defined as a *global* norm of the error (integrated over the whole computational domain). The standard norm considered for error assessment in elliptic problems is the norm induced by the bilinear form of the corresponding weak equations. This specific norm is referred as the *energy* norm because it is related with the energy of the underlying physical model. For instance, the energy norm in steady-state linear elasticity coincides with (the square root of) the potential elastic energy. Assessing the error energy norm is particularly straightforward in elliptic problems, because this particular error measure is closely related with the residual. Representative works assessing the error energy norm in elliptic problems are the pioneering references on error assessment by Babuška and Rheinboldt (1978), Ladevèze and Leguillon (1983), and Zienkiewicz and Zhu (1987).

The energy-like error measures for structural transient dynamics do not only include the potential elastic energy, but also the kinetic energy as well as the dissipated energy for problems containing a finite amount of damping. Refer-

ences by Li and Wiberg (1998), Wiberg and Li (1999), Schleupen and Ramm (2000), and Aubry et al. (1999) propose error estimates assessing the kinetic and elastic energy of the error. On the other hand, references by Ladevèze and Waeytens (2009), Waeytens (2010), Ladevèze (2008), and Waeytens et al. (2012) propose computable bounds of the dissipated energy associated with the error.

**2) Goal-oriented error estimates** (also referred as *local* error estimates)

In the context, the error measure is defined by means of a functional extracting a single representative scalar value of the solution of the problem. The end-user of the finite element software can define the functional such that the extracted value is a *quantity of interest* of the problem with a relevant physical meaning (e.g. the average of the solution in a specific *local* region of the domain). The error to be assessed is the error in the quantity of interest. That is, the difference between the quantity of interest associated with the exact solution and the quantity of interest associated with the finite element approximation. The techniques assessing the error in the quantity of interest are generally referred as *goal-oriented*.

Quantities of interest are error measures more meaningful than the standard energy-like global norms. However, assessing the error in an arbitrary quantity of interest requires approximating an auxiliary problem referred as the *adjoint* or *dual* problem. Thus, goal-oriented error estimates are usually more expensive than energy-like ones.

Goal-oriented error estimates are originally proposed for elliptic problems by Paraschivoiu et al. (1997), Cirak and Ramm (1998), and Prudhomme and Oden (1999). These techniques are extended to other problem types. For instance, estimates for the advection-diffusion-reaction equation are discussed by Parés et al. (2009). Similar approaches for the Stokes problem are presented by Larson et al. (2010). An extension to parabolic time-dependent problems is introduced by Parés et al. (2008a,b), and Díez and Calderón (2007). Moreover, the same type of tools are discussed for coupled problems by Larson and Bengzon (2008), Larson et al. (2008), Fick et al. (2010), Van Der Zee et al. (2011), and Asner et al. (2012). Finally, goal-oriented error estimates are also proposed for structural transient dynamics by Waeytens et al. (2012), Bangerth et al. (2010),

Schleupen and Ramm (2000), and Fuentes et al. (2006).

The specific techniques for assessing the error in structural transient dynamics are briefly presented in the following. The interested reader can find the specific details in the appended paper A. For the sake of presentation, the error assessment techniques are grouped here in three types: 1) recovery-based estimates, 2) the dual weighted residual method and 3) the constitutive relation error method. This classification is based on the different methodologies used to derive the error estimates.

### 1) Recovery-based error estimates

In the context of steady-state elasticity and other elliptic problems, recovery-based error estimates provide approximations of the error energy norm and local error indicators used for mesh adaptivity. The error estimate is derived as follows. First, the error energy norm is written in terms of the error in stresses using the complementary energy. The error in stresses is defined as the difference between the (unknown) exact stresses and the stresses associated with the finite element solution. Hence, the error estimate is obtained replacing the unknown exact stress by an enhanced version of the available finite element stress. The local error contributions are obtained by restricting the integrals involved in the definition of the complementary energy to the elements of the computational mesh.

The technique providing the enhanced stress field consists in computing (or *recovering*) a *continuous* stress as a post-process of the *discontinuous* finite element stress. There are two main approaches to compute the continuous stress. Either solving a global problem involving all the degrees of freedom of the mesh [Zienkiewicz and Zhu (1987)], or solving local problems involving a small subset of degrees of freedom [Zienkiewicz and Zhu (1992a,b)].

Recovery-based estimates are applied to structural dynamics to provide local error information driving mesh adaptivity, see references by Li and Wiberg (1998), Wiberg and Li (1999), Schleupen and Ramm (2000), and Erhart et al. (2006). The local error indicators are obtained performing at each time step the standard recovery techniques designed for steady-state problems. The elastic energy of the error is assessed using the previously presented stress recovery. However, the same approach do not holds for assessing the kinetic energy of the error, because it requires computing an enhanced version of the velocities.

Note that the finite element velocity is already continuous at the inter-element boundaries. Consequently, a specific recovery procedure is introduced for the velocities, see references by Wiberg and Li (1994) and Wiberg et al. (1999) for details.

The recovery techniques applied to time-dependent problems give approximations of the space discretization error, but they are not sufficient to assess the time discretization error. Hence, the recovery techniques allow to adapt only the space discretization. The time-discretization is adapted by introducing alternative error estimates for the time discretization, see reference by Schleupen and Ramm (2000) for details.

A complete review on recovery estimates for different problem types is introduced by Wiberg et al. (1997).

## 2) The dual weighted residual method

The dual residual method provides approximations of the error in the quantity of interest as well as local error indicators used for mesh adaptivity. The error estimate is derived in two stages. First, the error in the quantity of interest is expressed in terms of the exact solution of the adjoint problem and of the weak residual associated with the numerical approximation. Then, the error estimate is obtained by replacing the exact solution of the adjoint problem by a suitable numerical approximation, see the work by Rannacher and Stutzmeier (1997) for details. The local error indicators are obtained by restricting the integrals involved in the weak residual to the elements of the computational mesh.

The key ingredient of the dual weighted residual method is computing a suitable approximation of the adjoint solution. Here, the word “suitable” means that the adjoint approximation has to belong to a richer interpolation space than the one used for approximating the solution of original problem. This is required to avoid the cancellation of the assessed error by Galerkin orthogonality. The adjoint approximation is obtained either applying recovery techniques to a coarse-mesh approximation of the adjoint solution or solving the adjoint problem using higher order elements. Note that, this latter approach might be unaffordable in large examples.

The weighted residual method is applied to a high variety of problem types, even non-linear problems, as is shown in the review papers by Rannacher (2001), and



Becker and Rannacher (2001). This methodology is originally introduced for elliptic problems by Rannacher and Stuttmeier (1997). The extension to linear structural transient dynamics is proposed by Bangerth et al. (2010), Schleupen and Ramm (2000), Bangerth (1998), Rannacher (2001), Bangerth and Rannacher (1999), and Bangerth and Rannacher (2001). The non-linear dynamic case is considered by Fuentes et al. (2006). In the context of structural dynamics and other time-dependent problems, the dual weighted residual method is expensive in terms of CPU time and memory requirements. This is because the space-time adjoint solution has to be computed and stored as a whole (i.e. at each mesh vertex and time step) before computing the error estimate.

The rationale of the dual weighted residual method is used to assess the error in global norms as well. In the context of second order hyperbolic problems, Eriksson et al. (1996) propose an estimate for the  $L^2$  norm of the error at the final time of the computation. Following a similar approach, Aubry et al. (1999) propose an estimate of the total error energy (kinetic plus elastic). The resulting error estimate is completely *explicit* in the sense that it is computable as a direct post-process of the residual, without solving any auxiliary problem. For these particular error measures, the adjoint problem is only an auxiliary mathematical artifact used to derive the estimate. Consequently, the adjoint problem is not approximated.

### 3) Constitutive relation error method

The constitutive relation method furnishes bounds of the error energy norm as well as the error in the quantity of interest. Computing error bounds with this methodology requires building a stress-based approximation of the problem (fulfilling the equilibrium/momentum equations and the Neumann boundary conditions). The difference between the equilibrated stress and the stress associated with the finite element approximation is a computable error in stresses corresponding to the non-verification of the constitutive relation.

In the context of elliptic problems, the bound of the error energy norm is derived using the result by Prager and Synge (1947) which states that the complementary energy of the error in the constitutive relation is an upper bound of the error energy norm. The error in the quantity of interest is assessed by combining error bounds for the original and adjoint problems. Hence, a finite element

approximation as well as an equilibrated stress field are required for the adjoint problem.

The error bounds are computable once the equilibrated stresses are available. Stress equilibration techniques are proposed by Ladevèze and Leguillon (1983), Ainsworth and Oden (1997, 2000), and Parés et al. (2006). A comparison of different stress equilibration procedures is presented by Pled et al. (2011). Constitutive relation error estimates are *implicit* in the sense that the underlying stress equilibration technique is based on solving auxiliary local problems. In many contexts, constitutive relation error are equivalent to other implicit residual type error estimates, for instance the ones proposed by Ainsworth and Oden (1997), Ainsworth and Oden (2000), and Parés et al. (2006).

The constitutive relation error method is introduced in the literature originally for linear elliptic problems by Ladevèze and Leguillon (1983). The method is extended later to deal with a wide range of problem types. For instance, parabolic problems are considered by Chamoin and Ladevèze (2008). Non-linear problems are considered by Ladevèze and Moës (1999), Ladevèze et al. (2000), Ladevèze and Moës (1997), and Gallimard et al. (2000). The constitutive relation error method is applied to structural dynamics by Ladevèze (2008), Ladevèze and Waeytens (2009), Waeytens (2010), and Waeytens et al. (2012).

Computing error bounds in structural transient dynamics using the constitutive relation method has two main difficulties. First, the standard stress equilibration techniques designed for steady-state problems have to be repeated at each time step which is computationally demanding. Second, deriving the error bounds requires that the formulation of the problem includes some non-zero amount of damping. That is, the bounding properties are lost in the case of pure elasticity. In practice, the computed error bounds are very pessimistic for small values of the viscosity.

### 1.3 Objectives and document layout

The present thesis aims at contributing in the research field of a posteriori error assessment for structural dynamics by proposing new error estimates addressing the following difficulties: 1) the poor quality of the computable error bounds, 2) the

cost of computing goal-oriented error estimates, and 3) the limitation of standard quantities of interest when dealing with time-dependent problems.

**1) Enhancing the quality of goal-oriented error bounds.**

Further research is required to improve the quality of the available error bounds in structural transient dynamics. In the context of elliptic problems, the error bounds are derived using the result by Prager and Synge (1947) which holds because the bilinear form of the weak problem is an inner product. In structural dynamics, the associated bilinear form is not symmetric (hence, not an inner product), which makes computing error bounds challenging.

References by Parés et al. (2008a,b) derive error bounds for the time-dependent convection-diffusion-reaction equation. These references propose a methodology to deal with a non-symmetric bilinear form. Consequently, an analogous approach is investigated here for structural dynamics. The alternative error bounds are presented in section 3.1 and discussed in detail in paper B.

**2) Enhancing the efficiency of goal-oriented error estimates**

The available goal-oriented error estimates for structural dynamics consider direct time-integration methods for approximating the solution of the adjoint problem. The resulting error estimates are expensive in terms of memory requirements because the adjoint solution has to be computed and stored at each mesh node and time step. Moreover, error estimates generally require performing post-process operations (i.e stress recovery or equilibration) at each time step which might be unaffordable in large problems. Modal analysis is investigated as an alternative way to efficiently compute and store the adjoint solution. The modal-based error estimate is presented in section 3.2 and discussed in details in paper C. Moreover, the proposed error estimate is used for space-time adaptivity in section 3.3 and in paper D.

**3) Going beyond standard quantities of interest for time-dependent problems**

As previously announced, the quantities of interest available in the literature for error assessment are expressed in terms of a functional extracting a single representative scalar value of the solution. A quantity of interest for steady-state problems is usually the average of the unknown solution in a sub-region of the

computational domain. However, in time dependent problems, the definition of the quantity of interest must involve not only a spatial sub-domain but also a time interval of interest. The choice of this time frame is not always obvious for the end-user. This is because a single scalar value does not provide enough pieces of information about the whole time-space solution.

The preferred quantities of interest in time-dependent problems are time-dependent scalar functions instead of scalar values. For instance, the history of the average of the solution in a space sub-region of the computational domain. These type of quantities are referred in the following as *timeline*-dependent quantities of interest in contrast to the standard *scalar* quantities. Timeline-dependent quantities of interest are better suited to time-dependent problems because they preclude selecting the time frame. The error assessment strategy for timeline-dependent quantities is to be investigated. The resulting error estimates are presented in section 3.4 and in paper C.

The remainder of this document is structured in two parts. The first one is an overview of the thesis work. This includes the formal definition of the equations of structural dynamics and the error to be assessed, an overview of the main contributions, and the conclusions and further research. The second part consist of five appended papers where the contributions are discussed in detail. Paper A presents a comprehensive state-of-the-art review on error assessment for structural transient dynamics. Paper B discusses alternative error bounds. Paper C presents the modal-based approximation of the adjoint solution and the error assessment strategy for timeline-dependent quantities of interest. Paper D introduces an space-time adaptive strategy based on the modal description of the adjoint solution. Finally, paper E details the mesh refinement and un-refinement procedure considered in the adaptive strategy presented in paper D.

## 2 Problem description

### 2.1 Governing equations

A visco-elastic body occupies an open bounded domain  $\Omega \subset \mathbb{R}^d$ ,  $d \leq 3$ , with boundary  $\partial\Omega$ . The boundary is divided in two disjoint parts,  $\Gamma_N$  and  $\Gamma_D$  such that  $\partial\Omega = \bar{\Gamma}_N \cup \bar{\Gamma}_D$  and the considered time interval is  $I := (0, T]$ . Under the assumption of small

perturbations, the evolution of displacements  $\mathbf{u}(\mathbf{x}, t)$  and stresses  $\boldsymbol{\sigma}(\mathbf{x}, t)$ , for  $\mathbf{x} \in \Omega$  and  $t \in I$ , is described by the visco-elastodynamic equations

$$\rho(\ddot{\mathbf{u}} + a_1 \dot{\mathbf{u}}) - \nabla \cdot \boldsymbol{\sigma} = \mathbf{f} \quad \text{in } \Omega \times I, \quad (1a)$$

$$\mathbf{u} = \mathbf{0} \quad \text{on } \Gamma_D \times I, \quad (1b)$$

$$\boldsymbol{\sigma} \cdot \mathbf{n} = \mathbf{g} \quad \text{on } \Gamma_N \times I, \quad (1c)$$

$$\mathbf{u} = \mathbf{u}_0 \quad \text{at } \Omega \times \{0\}, \quad (1d)$$

$$\dot{\mathbf{u}} = \mathbf{v}_0 \quad \text{at } \Omega \times \{0\}, \quad (1e)$$

where an upper dot indicates derivation with respect to time, that is  $(\dot{\bullet}) := \frac{d}{dt}(\bullet)$ , and  $\mathbf{n}$  denotes the outward unit normal to  $\partial\Omega$ . The input data includes the mass density  $\rho = \rho(\mathbf{x}) > 0$ , the first Rayleigh coefficient  $a_1 \geq 0$ , the body force  $\mathbf{f} = \mathbf{f}(\mathbf{x}, t)$  and the traction  $\mathbf{g} = \mathbf{g}(\mathbf{x}, t)$  acting on the Neumann boundary  $\Gamma_N \times I$ . The initial conditions for displacements and velocities are  $\mathbf{u}_0 = \mathbf{u}_0(\mathbf{x})$  and  $\mathbf{v}_0 = \mathbf{v}_0(\mathbf{x})$  respectively. For the sake of simplicity and without any loss of generality, Dirichlet conditions (1b) are taken as homogeneous.

The set of equations (1) is closed with the constitutive law,

$$\boldsymbol{\sigma} := \mathcal{C} : \boldsymbol{\varepsilon}(\mathbf{u} + a_2 \dot{\mathbf{u}}), \quad (2)$$

where the parameter  $a_2 \geq 0$  is the second Rayleigh coefficient, the tensor  $\mathcal{C}$  is the standard 4th-order elastic Hooke tensor. The strains are given by the kinematic relation corresponding to small perturbations,  $\boldsymbol{\varepsilon}(\mathbf{w}) := \frac{1}{2} (\nabla \mathbf{w} + (\nabla \mathbf{w})^T)$ .

**Remark 1.** *The displacement field  $\mathbf{u}$  defined in equations (1) is the unknown exact solution which is taken as the truth in the following error assessment analysis.*

## 2.2 Numerical approximation

The input data of any a posteriori error assessment technique is an approximation of the exact solution of the underlying mathematical problem. In the following, the numerical approximation of problem (1) is referred as  $\tilde{\mathbf{u}} \approx \mathbf{u}$ . There are two main alternatives to compute this approximation. Either using finite elements for the space discretization and finite differences for the time discretization (e.g. the well known method proposed by Newmark (1959)) or using finite elements for both the space and time discretizations, see the work by Hughes and Hulbert (1988). Note however, these

are not the only available approximation techniques. Other approximation methods are based on finite volumes (Lee et al. (2013)), spectral elements (Komatitsch et al. (1999)) and boundary elements (Bouchona and Sánchez-Sesma (2007)). A detailed presentation of the available approximation methods in structural dynamics is out of the scope of this thesis overview. The reader is referred to the paper A or to references by Bangerth et al. (2010), and Bathe (1996) for specific details.

Most of the approximation methods are based on separated discretizations for the space and time domains (i.e. discretizing the whole space-time domain  $\Omega \times I$  with an unstructured mesh is non-standard). The time domain  $I$  is discretized using a time grid  $\mathcal{T} := \{t_0, t_1, \dots, t_N\}$ , where  $0 = t_0 < t_1 < \dots < t_N = T$  are the grid points. These points define the time intervals  $I_n := (t_{n-1}, t_n]$  with time step length  $\Delta t_n := t_n - t_{n-1}$ ,  $n = 1, \dots, N$ . On the other hand, the space domain  $\Omega$  is discretized using a finite element mesh. The set of all mesh elements is denoted by  $\mathcal{P}$ .

**Remark 2.** *In the context of mesh adaptivity for transient problems, the space finite element mesh  $\mathcal{P}$  is allowed to be different at each time point  $t_n \in \mathcal{T}$ . In that case, the finite element mesh associated with the time point  $t_n$  is denoted as  $\mathcal{P}_n$ . Consequently, the approximation method has to be able to properly transfer the numerical solution from one mesh to the other without lose of information. A detailed methodology to deal with changing meshes is found in the paper D.*

Problem (1) is typically discretized in space using standard finite elements. The finite element mesh  $\mathcal{P}$  is associated with a functional space, namely  $\mathbf{V}_0^{H,p}$ , containing continuous piecewise polynomial functions of degree  $p$ . The upper-script  $H$  stands for the characteristic mesh element size of the elements in  $\mathcal{P}$ . The discretization of (the weak version of) problem (1) in the space  $\mathbf{V}_0^{H,p}$  leads to a system of Ordinary Differential Equations (ODE). Solving this system yields the numerical approximation  $\tilde{\mathbf{u}}$ . The system of ODE is solved either with finite differences, as proposed by Newmark (1959), or using finite elements, see works by Eriksson et al. (1996), Johnson (1993), and Hughes and Hulbert (1988).

The approximation  $\tilde{\mathbf{u}}$  has to fulfill particular properties to be a valid input for the error analysis. Some error estimates (e.g. the ones proposed in the papers B and C) require only that the approximation  $\tilde{\mathbf{u}}$  is regular enough. This allows computing the numerical approximation with many different approximation methods. On the other hand, other error assessment strategies, e.g. the one proposed in appended paper D, require that the approximation  $\tilde{\mathbf{u}}$  is solved with a specific method. This

is because the error estimate requires the Galerkin orthogonality property associated with a particular discrete solution.

### 2.3 Error to be assessed

The discretization error associated with the approximation  $\tilde{\mathbf{u}}$  is defined as  $\mathbf{e} := \mathbf{u} - \tilde{\mathbf{u}}$ . A brute-force approach to approximate the unknown error  $\mathbf{e}$  is computing an *overkill* or *reference* solution of problem (1), namely  $\mathbf{u}^{\text{ovk}}$ . This overkill solution is computed using a much finer discretization than the one for  $\tilde{\mathbf{u}}$ . The overkill discretization is built by an  $H$ - or  $p$ -refinement of the finite element mesh  $\mathcal{P}$  and by adding more time points into the time partition  $\mathcal{T}$ . Then, the error is approximated replacing the exact solution  $\mathbf{u}$  by the overkill approximation  $\mathbf{u}^{\text{ovk}}$ , namely  $\mathbf{e} \approx \mathbf{u}^{\text{ovk}} - \tilde{\mathbf{u}}$ . Note that computing the overkill solution is unaffordable in large examples. Thus, a posteriori error assessment techniques aim at assessing the error in a more affordable way.

As previously announced, the error to be assessed is a specific error measure (an scalar value) instead of the error field  $\mathbf{e}(\mathbf{x}, t)$ . The error measure considered here is a quantity of interest associated with a functional  $L^\mathcal{O}(\cdot)$ . A commonly used quantity of interest in elastodynamics is represented by the linear functional

$$\begin{aligned} L^\mathcal{O}(\mathbf{w}) := & \int_0^T (\mathbf{f}^\mathcal{O}(t), \dot{\mathbf{w}}(t)) \, dt + \int_0^T (\mathbf{g}^\mathcal{O}(t), \dot{\mathbf{w}}(t))_{\Gamma_N} \, dt \\ & + m(\mathbf{v}^\mathcal{O}, \dot{\mathbf{w}}(T)) + a(\mathbf{u}^\mathcal{O}, \mathbf{w}(T)), \end{aligned} \quad (3)$$

where  $\mathbf{f}^\mathcal{O}$ ,  $\mathbf{g}^\mathcal{O}$ ,  $\mathbf{v}^\mathcal{O}$  and  $\mathbf{u}^\mathcal{O}$  are the data characterizing the quantity of interest. The weighting functions  $\mathbf{f}^\mathcal{O}$  and  $\mathbf{g}^\mathcal{O}$  allow to define weighted averages of velocities integrated in  $\Omega \times I$  and  $\Gamma_N \times I$  respectively. The fields  $\mathbf{v}^\mathcal{O}$  and  $\mathbf{u}^\mathcal{O}$  play the role of weighting functions defining averages of velocities and strains at the final simulation time  $T$ . In the definition of the quantity of interest (3), the following notations are used

$$\begin{aligned} (\mathbf{v}, \mathbf{w}) := & \int_\Omega \mathbf{v} \cdot \mathbf{w} \, d\Omega, \quad (\mathbf{v}, \mathbf{w})_{\Gamma_N} := \int_{\Gamma_N} \mathbf{v} \cdot \mathbf{w} \, d\Gamma, \quad m(\mathbf{v}, \mathbf{w}) := \int_\Omega \rho \mathbf{v} \cdot \mathbf{w} \, d\Omega, \quad \text{and} \\ a(\mathbf{v}, \mathbf{w}) := & \int_\Omega \boldsymbol{\varepsilon}(\mathbf{v}) : \mathbf{C} : \boldsymbol{\varepsilon}(\mathbf{w}) \, d\Omega. \end{aligned}$$

The error to be assessed is the error in the quantity of interest,  $s^e := s - \tilde{s}$ , defined as the difference between the exact quantity of interest  $s := L^\mathcal{O}(\mathbf{u})$  and the computed one  $\tilde{s} := L^\mathcal{O}(\tilde{\mathbf{u}})$ . In the following, it is assumed that the functional  $L^\mathcal{O}(\cdot)$  is

linear. Thus,  $s^e = L^{\mathcal{O}}(\mathbf{e})$ . However, non-linear functionals can also be handled after a linearization, see references by Bangerth et al. (2010), and Fuentes et al. (2006) for details.

## 2.4 Adjoint problem

Assessing the error in the quantity of interest requires introducing an auxiliary problem associated with functional  $L^{\mathcal{O}}(\cdot)$ , referred as *adjoint* or *dual* problem. The adjoint problem allows rewriting the error in the quantity of interest in a more convenient way for error assessment. The problem defining the exact adjoint solution  $\mathbf{u}^d$  reads:

$$\rho(\ddot{\mathbf{u}}^d - a_1 \dot{\mathbf{u}}^d) - \nabla \cdot \boldsymbol{\sigma}^d(\mathbf{u}^d) = -\mathbf{f}^{\mathcal{O}} \quad \text{in } \Omega \times I, \quad (4a)$$

$$\mathbf{u}^d = \mathbf{0} \quad \text{on } \Gamma_D \times I, \quad (4b)$$

$$\boldsymbol{\sigma}^d(\mathbf{u}^d) \cdot \mathbf{n} = -\mathbf{g}^{\mathcal{O}} \quad \text{on } \Gamma_N \times I, \quad (4c)$$

$$\mathbf{u}^d = \mathbf{u}^{\mathcal{O}} \quad \text{at } \Omega \times \{T\}, \quad (4d)$$

$$\dot{\mathbf{u}}^d = \mathbf{v}^{\mathcal{O}} \quad \text{at } \Omega \times \{T\}, \quad (4e)$$

with the constitutive law

$$\boldsymbol{\sigma}^d(\mathbf{u}^d) := \mathbf{C} : \boldsymbol{\varepsilon}(\mathbf{u}^d - a_2 \dot{\mathbf{u}}^d). \quad (5)$$

Note that the definition of the adjoint problem (4) depends on the selected quantity of interest. That is, the external loads and *final* conditions of the adjoint problem are determined by the definition of quantity of interest in equation (3).

The adjoint problem (4) has exactly the same structure as the original (1) if integrated backwards in time starting from the final conditions (4d) and (4e). Thus, any of the available approximation techniques for elastodynamics can be considered for approximating the adjoint.

**Remark 3** (Illustrative example). *The following example illustrates the adjoint problem given in (4) in a one dimensional example. The spatial computational domain is  $\Omega = (0, 1)$  m, the boundaries are  $\Gamma_N = \{0 \text{ m}\}$  and  $\Gamma_D = \{1 \text{ m}\}$ , and the time interval is  $I = (0, 2]$  s. The material properties are  $E = 1 \text{ Pa}$ ,  $\nu = 0$ ,  $\rho = 1, \text{ kg/m}^3$  and  $a_1 = a_2 = 0 \text{ s}$ .*

*The adjoint problem illustrated in this remark is associated with the quantity of interest*

$$L^{\mathcal{O}}(w) = \int_I \int_{\Omega} \alpha(t) \beta(x) \dot{w}(x, t) \, dx \, dt, \quad (6)$$



where  $\alpha(t)$  and  $\beta(x)$  are the functions defined in figure 1. Note that the quantity

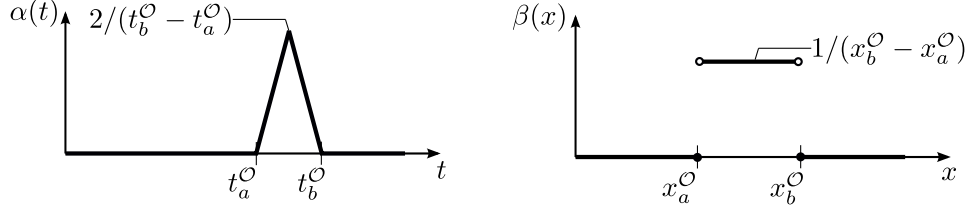


Figure 1: Definition of  $\alpha(t)$  and  $\beta(x)$ , (left) and (right) respectively.

of interest (6) corresponds to take  $\mathbf{g}^O = \mathbf{v}^O = \mathbf{u}^O = \mathbf{0}$  and  $\mathbf{f}^O = \alpha(t)\beta(x)$  in equation (3) and provides a weighted average of velocities in the space-time region  $S^O = (x_a^O, x_b^O) \times (t_a^O, t_b^O)$ , see figure 2. In this example, the region  $S^O$  is characterized by  $x_a^O = 0.2$  m  $x_b^O = 0.3$  m,  $t_a^O = 1.8$  s and  $t_b^O = 1.9$  s.

The adjoint problem associated with quantity (6) is plotted in figure 2. Note that the quantity of interest acts as the external loading of the adjoint problem. The quantity of interest generates a perturbation in the space-time region of interest  $S^O$  which propagates along the characteristic lines backwards in time. The adjoint solution is indeed the region of influence of the quantity of interest. That is, any perturbation taking place where the adjoint solution is zero has no influence in the quantity of interest.

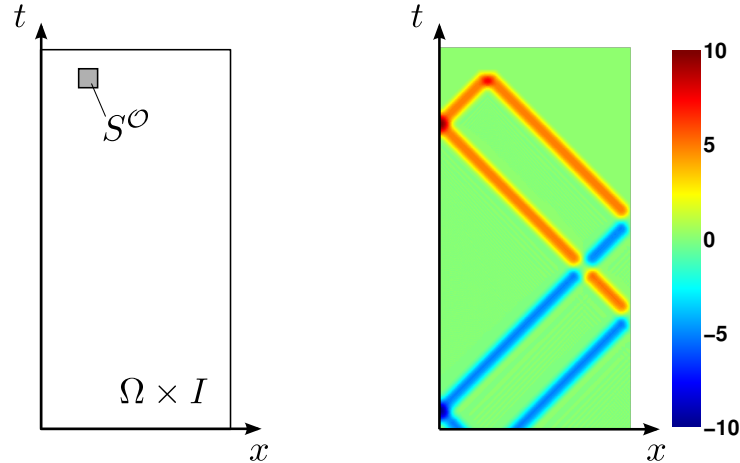


Figure 2: Illustration of the adjoint problem for the quantity of interest given in equation (6) (average of velocities in the region  $S^O$ ). Definition of the space-time domain  $\Omega \times I$  and region of interest  $S^O$  (left). Adjoint velocities [m/s] (right).

### 3 Contributions

This section is an overview of the major thesis contributions. The contributions are presented stressing out which are the main novelties, the key ideas of the proposed methods, and numerical examples.

#### 3.1 Goal-oriented error bounds

This section presents computable error bounds for structural dynamics. First, the strategy proposed by Waeytens (2010) is extended to deal with a linear Kelvin-Voigt visco-elastic model instead of a Maxwell model. This allows a simpler derivation of the error bounds allowing to concentrate in the mathematical difficulties. Note that the proposed approach is general enough to deal with more sophisticated visco-elastic models. Second, alternative error bounds are proposed improving the estimates introduced by Waeytens (2010). These contributions are enclosed in the appended papers A and B. The main rationale is summarized here.

The goal is to compute two scalar values  $\eta_L$  and  $\eta_U$  such that

$$\eta_L \leq L^O(\mathbf{e}) \leq \eta_U. \quad (7)$$

Computing the error bounds  $\eta_L$  and  $\eta_U$  with the constitutive relation method requires that the problem contains a finite amount of damping. In the following, it is assumed that  $a_1 = 0$  and  $a_2 > 0$  in order to meet this hypothesis. With this choice, all the damping introduced in the formulation comes from the constitutive relation (2) which is seen as a particular type of Kelvin-Voigt visco-elastic model.

The key ingredient allowing to compute the error bounds is building admissible pairs  $(\tilde{\mathbf{u}}, \tilde{\boldsymbol{\sigma}})$  and  $(\tilde{\mathbf{u}}^d, \tilde{\boldsymbol{\sigma}}^d)$  for the original and the adjoint problems. The admissible pair  $(\tilde{\mathbf{u}}, \tilde{\boldsymbol{\sigma}})$  for the original problem consists of a *Kinematically admissible* (K-admissible) displacement  $\tilde{\mathbf{u}}$  and a *Dynamically admissible* (D-admissible) stress tensor  $\tilde{\boldsymbol{\sigma}}$ .

The K-admissible displacement  $\tilde{\mathbf{u}}$  fulfills the initial and Dirichlet boundary conditions of the original problem (1). Typically, the K-admissible displacement is the computed numerical approximation  $\tilde{\mathbf{u}}$ . For that reason, the notation is the same for both for the K-admissible displacement and the numerical approximation. On the other hand, the D-admissible stress  $\tilde{\boldsymbol{\sigma}}$  is computed such that it is in dynamic equilibrium with the inertia force associated with the K-admissible displacement, namely  $-\rho\ddot{\tilde{\mathbf{u}}}$ , the body force  $\mathbf{f}$ , and boundary traction  $\mathbf{g}$  appearing in (1).

The adjoint admissible pair  $(\tilde{\mathbf{u}}^d, \tilde{\boldsymbol{\sigma}}^d)$  fulfills analogous properties but referred to the adjoint problem. Computing a D-admissible stress requires performing standard equilibration techniques designed for steady-state elasticity at each time point in the time grid  $\mathcal{T}$ . A more detailed definition of the admissible pairs  $(\tilde{\mathbf{u}}, \tilde{\boldsymbol{\sigma}})$  and  $(\tilde{\mathbf{u}}^d, \tilde{\boldsymbol{\sigma}}^d)$  and the methodology to compute them is found in the appended paper B.

The admissible pairs define the following errors in stresses

$$\tilde{\boldsymbol{\sigma}}^e := \tilde{\boldsymbol{\sigma}} - \boldsymbol{\sigma}(\tilde{\mathbf{u}}) \quad \text{and} \quad \tilde{\boldsymbol{\sigma}}^{d,e} := \tilde{\boldsymbol{\sigma}}^d - \boldsymbol{\sigma}(\tilde{\mathbf{u}}^d), \quad (8)$$

which are a measure of the non-verification of the constitutive relations (2) and (5) respectively. Note that the stress errors  $\tilde{\boldsymbol{\sigma}}^e$  and  $\tilde{\boldsymbol{\sigma}}^{d,e}$  are computable once the admissible pairs  $(\tilde{\mathbf{u}}, \tilde{\boldsymbol{\sigma}})$  and  $(\tilde{\mathbf{u}}^d, \tilde{\boldsymbol{\sigma}}^d)$  are available. The norms  $\|\tilde{\boldsymbol{\sigma}}^e\|_\sigma$  and  $\|\tilde{\boldsymbol{\sigma}}^{d,e}\|_\sigma$  are referred as the *constitutive relation error* for the original and adjoint problems following the terminology by Ladevèze and Pelle (2001). The space-time stress norm used to measure the error is

$$\|\boldsymbol{\tau}\|_\sigma := \left( \frac{1}{a_2} \int_I \int_\Omega \boldsymbol{\tau} : \mathbf{C}^{-1} : \boldsymbol{\tau} \, d\Omega \, dt \right)^{1/2}. \quad (9)$$

Note that the norm  $\|\cdot\|_\sigma$  is related with the dissipated energy due to the damping coefficient  $a_2$ . For this reason, the non-zero viscosity hypothesis has to be fulfilled in order to compute error bounds with this technique.

The error in the quantity of interest is bounded following two stages. First, the value  $L^\mathcal{O}(\mathbf{e})$  is bounded in terms of the energy norm of non-computable errors. Second, the non-computable errors are bounded using the constitutive relation errors  $\|\tilde{\boldsymbol{\sigma}}^e\|_\sigma$  and  $\|\tilde{\boldsymbol{\sigma}}^{d,e}\|_\sigma$ .

The first approach to bound the error  $L^\mathcal{O}(\mathbf{e})$  is already introduced by Waeytens (2010). The value  $L^\mathcal{O}(\mathbf{e})$  is bounded in terms of the unknown error  $\mathbf{e}$  as follows

$$|L^\mathcal{O}(\mathbf{e}) - \tilde{k}_1| \leq \|\tilde{\boldsymbol{\sigma}}^{d,e}\|_\sigma \|\mathbf{e}\|, \quad (10)$$

where  $\tilde{k}_1$  is a computable value, see paper B for a detailed proof. The norm  $\|\cdot\|$  is related with the stress norm  $\|\cdot\|_\sigma$  but taking displacements as argument, namely

$$\|\mathbf{w}\| := \left( a_2 \int_I \int_\Omega \boldsymbol{\varepsilon}(\dot{\mathbf{w}}) : \mathbf{C} : \boldsymbol{\varepsilon}(\dot{\mathbf{w}}) \, d\Omega \, dt \right)^{1/2}. \quad (11)$$

Then, the unknown error energy  $\|\mathbf{e}\|$  is bounded using the upper bound property of the constitutive relation error  $\|\tilde{\boldsymbol{\sigma}}^e\|_\sigma$

$$\|\mathbf{e}\| \leq \|\tilde{\boldsymbol{\sigma}}^e\|_\sigma, \quad (12)$$

see appended paper B. Hence, the computable bounds  $\eta_L$  and  $\eta_U$  are readily obtained by combining expressions (12) and (10), namely

$$\zeta_U := \|\tilde{\boldsymbol{\sigma}}^{\text{d,e}}\|_\sigma \|\tilde{\boldsymbol{\sigma}}^{\text{e}}\|_\sigma + \tilde{k}_1, \quad (13\text{a})$$

$$\zeta_L := -\|\tilde{\boldsymbol{\sigma}}^{\text{d,e}}\|_\sigma \|\tilde{\boldsymbol{\sigma}}^{\text{e}}\|_\sigma + \tilde{k}_1. \quad (13\text{b})$$

The error representation (10) is obtained using the Cauchy-Schwarz inequality which typically induces a large overestimation of the assessed error. This makes the error bounds given in (13) not sharp, with an unrealistic and impractical bound gap.

Alternative error bounds are proposed leading to a sharper bound gap than the one associated with the bounds (13). There are two equivalent ways to derive the alternative bounds. Either introducing auxiliary symmetric error equations, see paper B, or using an auxiliary error in stresses, see paper A. For the sake of brevity, only the latter approach is presented in this overview.

The alternative approach requires introducing an auxiliary stress field that stands for the error with respect to the averaged viscous stress, namely

$$\boldsymbol{\sigma}_{\text{ave}}^{\text{e},\nu} := \boldsymbol{\sigma}^\nu - \frac{1}{2} (\tilde{\boldsymbol{\sigma}}^\nu + a_2 \mathbf{C} : \boldsymbol{\varepsilon}(\dot{\tilde{\mathbf{u}}})) , \quad (14)$$

where  $\boldsymbol{\sigma}^\nu := a_2 \mathbf{C} : \boldsymbol{\varepsilon}(\dot{\mathbf{u}})$  is the viscous stress associated with the exact solution,  $\tilde{\boldsymbol{\sigma}}^\nu := \tilde{\boldsymbol{\sigma}} - \mathbf{C} : \boldsymbol{\varepsilon}(\dot{\tilde{\mathbf{u}}})$  is the viscous stress associated with the D-admissible field, and  $a_2 \mathbf{C} : \boldsymbol{\varepsilon}(\dot{\tilde{\mathbf{u}}})$  is the viscous stress associated with the K-admissible field. Note that  $\boldsymbol{\sigma}_{\text{ave}}^{\text{e},\nu}$  is not computable because it includes the exact solution  $\mathbf{u}$  in the term  $\boldsymbol{\sigma}^\nu$ . The stress  $\boldsymbol{\sigma}_{\text{ave}}^{\text{e},\nu}$  is introduced as a mathematical artefact allowing to rewrite the error in the quantity of interest as

$$|L^\mathcal{O}(\mathbf{e}) - \tilde{k}_2| \leq \|\tilde{\boldsymbol{\sigma}}^{\text{d,e}}\|_\sigma \|\boldsymbol{\sigma}_{\text{ave}}^{\text{e},\nu}\|_\sigma, \quad (15)$$

where  $\tilde{k}_2$  is a computable value. The computable bounds for the error in the quantity of interest are obtained introducing the following upper bound for the unknown value  $\|\boldsymbol{\sigma}_{\text{ave}}^{\text{e},\nu}\|_\sigma$ ,

$$\|\boldsymbol{\sigma}_{\text{ave}}^{\text{e},\nu}\|_\sigma \leq \frac{1}{2} \|\tilde{\boldsymbol{\sigma}}^{\text{e}}\|_\sigma. \quad (16)$$

The detailed proofs of equations (15) and (16) are given in paper A. Using expressions (15) and (16), the alternative bounds are readily obtained:

$$\zeta_U := \frac{1}{2} \|\tilde{\boldsymbol{\sigma}}^{\text{d,e}}\|_\sigma \|\tilde{\boldsymbol{\sigma}}^{\text{e}}\|_\sigma + \tilde{k}_2, \quad (17\text{a})$$

$$\zeta_L := -\frac{1}{2} \|\tilde{\boldsymbol{\sigma}}^{\text{d,e}}\|_\sigma \|\tilde{\boldsymbol{\sigma}}^{\text{e}}\|_\sigma + \tilde{k}_2. \quad (17\text{b})$$

The bound corresponding to (17) is  $\zeta_U - \zeta_L = \|\tilde{\boldsymbol{\sigma}}^{\text{d,e}}\|_\sigma \|\tilde{\boldsymbol{\sigma}}^{\text{e}}\|_\sigma$  whereas the bound gap in (13) is  $\zeta_U - \zeta_L = 2\|\tilde{\boldsymbol{\sigma}}^{\text{d,e}}\|_\sigma \|\tilde{\boldsymbol{\sigma}}^{\text{e}}\|_\sigma$ . Note that the bound gap in equation (17) is the half of the bound gap corresponding to equation (13). Consequently, the proposed error bounds (17) provides a sharper error assessment.

The numerical results included in paper B show that the proposed bounds (17) are indeed sharper than previously available ones (13). The computed bounds are very pessimistic for materials with a small amount of viscosity even for the new bounds (17). The numerical tests also reveal that the bound gap is reduced as the mesh is refined and, consequently, the strategy provides sharp bounds for fine enough meshes. Nevertheless, in practice, for low viscosity, the meshes providing accurate bounds are not computationally affordable. Therefore, further research is needed to explore alternative pertinent bounds for nearly elastic problems.

**Remark 4** (Illustrative example). *This example illustrates the performance of the proposed computable bounds in a 2D wave propagation problem. This is a reduced version of the second numerical example found in paper B. The problem geometry, see figure 3, is a rectangular plate  $\Omega := (-0.5, 0.5) \times (0, 0.5)$  m<sup>2</sup> clamped at the bottom side, initially at rest ( $\mathbf{u}_0 = \mathbf{v}_0 = \mathbf{0}$ ), which is loaded with the impulsive traction*

$$\mathbf{g}(t) = \begin{cases} -g(t)\mathbf{e}_2 & \text{on } \Gamma_g, \\ 0 & \text{elsewhere,} \end{cases} \quad (18)$$

with  $\Gamma_g := [(0.075, 0.125) \cup (-0.075, -0.125)] \times \{0.5\}$  m,  $\mathbf{e}_2 := (0, 1)$  and  $g(t)$  is the impulsive time-dependent function defined in figure 3 with parameters  $g_{\max} = 30$  Pa and  $t_g = 0.005$  s. No body force is acting in this example ( $\mathbf{f} = \mathbf{0}$ ). The material properties of the plate are Young's modulus  $E = 8/3$  Pa, Poisson's ratio  $\nu = 1/3$ , the density  $\rho = 1$  kg/m<sup>3</sup> and the damping coefficients  $a_1 = 0$  s<sup>-1</sup>,  $a_2 = 10^{-4}, 10^{-2}$  s. Note that two different values of the parameter  $a_2$  are considered. The final simulation time is  $T = 0.25$  s. The plain stress hypothesis is considered.

The external loading generates elastic waves propagating along the plate and reaching to the region of interest  $\Omega^\circ$ . The quantity of interest is an average of the vertical component of the velocity in this region during a time interval selected such that the wave is noticeable in  $\Omega^\circ$ , see figure 4. This quantity is defined as

$$L^\circ(\mathbf{w}) = \int_0^T \alpha(t)(\boldsymbol{\lambda}^\circ, \dot{\mathbf{w}}(t)) dt, \quad \text{where } \boldsymbol{\lambda}^\circ(\mathbf{x}) := \begin{cases} \frac{-\mathbf{e}_2}{\text{meas}(\Omega^\circ)} & \mathbf{x} \in \Omega^\circ \\ \mathbf{0} & \text{elsewhere} \end{cases}$$

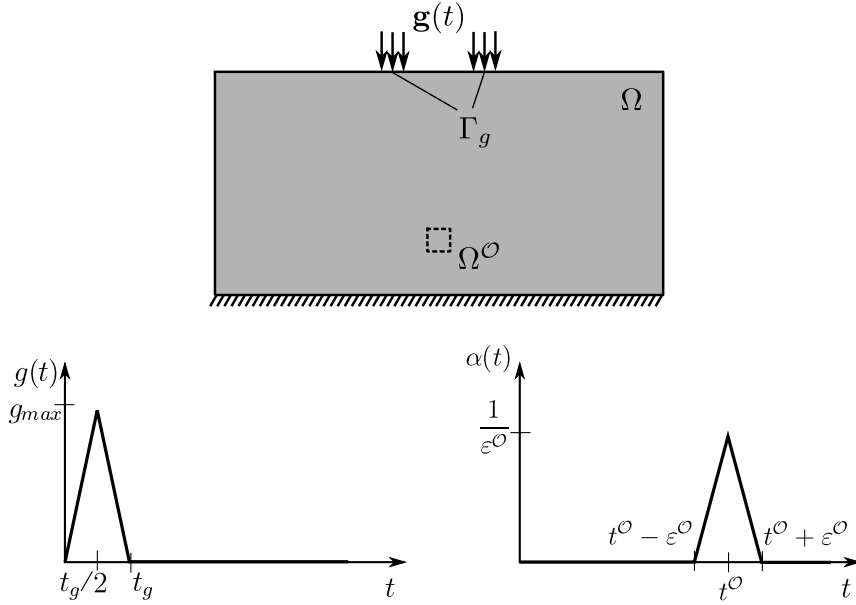


Figure 3: Problem geometry (top), time dependence of external load (left) and time dependence of the auxiliary function introduced to define the quantity of interest (right).

and the time dependent function  $\alpha(t)$  is defined in figure 3 with parameters  $\varepsilon^\circ = 0.01$  s and  $t^\circ = 0.217$  s. Note that the definition of the quantity of interest corresponds to take  $\mathbf{f}^\circ(\mathbf{x}, t) = \alpha(t)\boldsymbol{\lambda}^\circ(\mathbf{x})$  and  $\mathbf{g}^\circ = \mathbf{u}^\circ = \mathbf{v}^\circ = \mathbf{0}$  in equation (3).

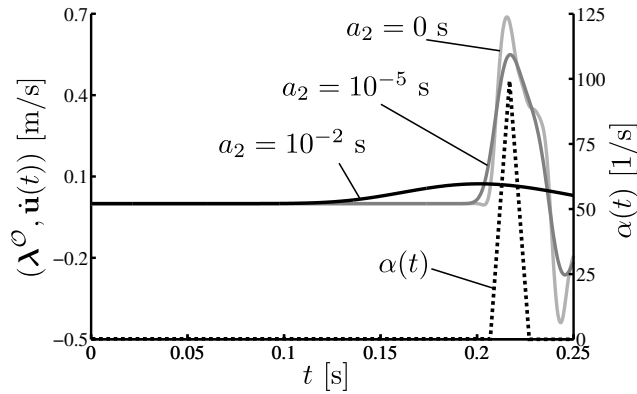


Figure 4: Time evolution of the average of the vertical velocity in the region  $\Omega^\circ$ , namely  $(\boldsymbol{\lambda}^\circ, \dot{\mathbf{u}}(t))$ , for three values of the viscosity (left y-axis). Time evolution of the weighting function  $\alpha(t)$  used to define the quantity of interest (right y-axis).

In this example, the computed error bounds,  $\eta_L$  and  $\eta_U$  are used to compute bounds of the exact quantity of interest  $L^\circ(\mathbf{u})$ , namely  $\tilde{\eta}_L \leq L^\circ(\mathbf{u}) \leq \tilde{\eta}_U$  with  $\tilde{\eta}_L := \eta_L +$

$L^{\mathcal{O}}(\tilde{\mathbf{u}})$  and  $\tilde{\eta}_{\mathcal{U}} := \eta_{\mathcal{U}} + L^{\mathcal{O}}(\tilde{\mathbf{u}})$ . Figure 5 shows the effectivity of the computed bounds  $\tilde{\eta}_{\mathcal{L}}$  and  $\tilde{\eta}_{\mathcal{U}}$  with respect the exact quantity of interest  $s = L^{\mathcal{O}}(\mathbf{u})$  which is computed using an overkill solution. Note that the bounds are sharper as the value of the viscosity increases or as the element size decreases. In particular, the quality of the bounds is very poor for small values of the viscosity. On the other hand, the proposed new bounds reduce in 50% the bound gap with respect to the previous approach. Note however that for the smallest values of the viscosity, this reduction is not sufficient to have bounds applicable in practical engineering examples.

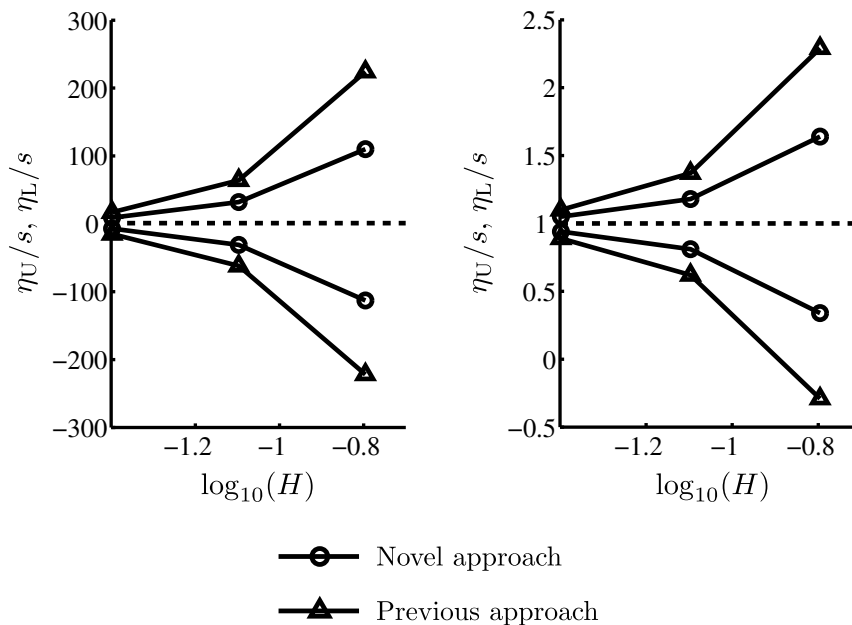


Figure 5: Convergence of the computed bounds for different values of element size and for two values of the viscosity-related parameter  $a_2 = 10^{-4}$  s (left) and  $a_2 = 10^{-2}$  s (right).

### 3.2 Modal-based goal-oriented error assessment

This section presents a novel approach to compute the adjoint problem arising in goal-oriented error assessment. The proposed approximation technique is based on the well-known modal analysis which is considered to approximate the time-dependence of the structural dynamic equations in many contexts, see for instance the book by Bathe (1996).

The modal-based strategy is particularly well suited for computing the adjoint problem associated with some particular quantities of interest. Following this approach, the adjoint solution is computed and stored for each vibration mode instead of for each time step. Moreover, the use of post-processing techniques in the space domain can be readily applied to the (space) description of the modes. This is performed just once for every relevant mode, with no need of carrying out the post-processing at each time step. Thus, the cost per time step is low.

The modal-based adjoint approximation aims at providing an efficient goal-oriented error assessment. The poor quality of the error bounds introduced in section 3.1 and the cost of computing the D-admissible fields suggest using the modal-based strategy to compute error approximations instead of error bounds. Note however, the proposed methodology is general enough to compute also error bounds.

The dual weighted residual method is considered here in order to compute approximations of the error in the quantity of interest. Following this approach, the error in the quantity of interest is expressed in terms of the exact solution of the adjoint problem as

$$L^{\mathcal{O}}(\mathbf{e}) = R(\mathbf{u}^{\text{d}}), \quad (19)$$

where  $R(\cdot)$  is the weak residual (integrated in space and time) associated with the approximation  $\tilde{\mathbf{u}}$ , namely

$$\begin{aligned} R(\mathbf{w}) &:= \int_0^T [l(\dot{\mathbf{w}}(t); t) - m(\ddot{\tilde{\mathbf{u}}}(t) + a_1 \dot{\tilde{\mathbf{u}}}(t), \dot{\mathbf{w}}(t)) - a(\tilde{\mathbf{u}}(t) + a_2 \dot{\tilde{\mathbf{u}}}(t), \dot{\mathbf{w}}(t))] dt \\ &\quad + m(\mathbf{v}_0 - \dot{\tilde{\mathbf{u}}}(0^+), \dot{\mathbf{w}}(0^+)) + a(\mathbf{u}_0 - \tilde{\mathbf{u}}(0^+), \mathbf{w}(0^+)), \quad \text{with} \\ l(\mathbf{w}; t) &:= (\mathbf{f}(t), \mathbf{w}) + (\mathbf{g}(t), \mathbf{w})_{\Gamma_N}. \end{aligned}$$

The error representation (19) allows obtaining the error in the quantity of interest provided that the exact solution of the adjoint problem is available. Conversely, if an accurate approximation of the adjoint solution is available, say  $\tilde{\mathbf{u}}^{\text{d}}$ , the error in the quantity of interest is estimated as

$$L^{\mathcal{O}}(\mathbf{e}) \approx R(\tilde{\mathbf{u}}^{\text{d}}) =: \tilde{s}^{\text{e}}. \quad (20)$$

The quality of the approximation  $\tilde{\mathbf{u}}^{\text{d}}$  is critical to obtain accurate estimates of the error in the scalar quantity of interest. The major novelty of the present approach is using modal analysis to compute the approximation  $\tilde{\mathbf{u}}^{\text{d}}$ .

Approximating function  $\tilde{\mathbf{u}}^{\text{d}}$  with modal analysis requires introducing a semidiscrete version (discrete in space and exact in time) of the adjoint problem (4). The



semidiscrete problem reads: find  $\mathbf{u}^{\text{d},H,p+1}(t) \in \mathcal{V}_0^{H,p+1}$  verifying the final conditions  $\mathbf{u}^{\text{d},H,p+1}(T) = \mathbf{u}^\mathcal{O}$  and  $\dot{\mathbf{u}}^{\text{d},H,p+1}(T) = \mathbf{v}^\mathcal{O}$  and such that for all  $t \in I$

$$\begin{aligned} m(\ddot{\mathbf{u}}^{\text{d},H,p+1}(t) - a_1 \dot{\mathbf{u}}^{\text{d},H,p+1}(t), \mathbf{w}) + a(\mathbf{u}^{\text{d},H,p+1}(t) - a_2 \dot{\mathbf{u}}^{\text{d},H,p+1}(t), \mathbf{w}) \\ = -l^\mathcal{O}(\mathbf{w}; t), \end{aligned} \quad (21)$$

for all test functions  $\mathbf{w} \in \mathcal{V}_0^{H,p+1}$ , where  $l^\mathcal{O}(\mathbf{w}; t) := (\mathbf{f}^\mathcal{O}(t), \mathbf{w}) + (\mathbf{g}^\mathcal{O}(t), \mathbf{w})_{\Gamma_N}$ . The finite element space  $\mathcal{V}_0^{H,p+1}$ , introduced in the definition of the semidiscrete problem (21) is obtained by increasing the polynomial degree of the space  $\mathcal{V}_0^{H,p}$  used to compute the numerical solution  $\tilde{\mathbf{u}}$ .

**Remark 5.** *The spacial resolution of the adjoint approximation  $\tilde{\mathbf{u}}^{\text{d}}$  has to be richer than the one of the numerical approximation  $\tilde{\mathbf{u}}$ . Otherwise, the error is underestimated when plugging the approximation  $\tilde{\mathbf{u}}^{\text{d}}$  into the residual  $R(\cdot)$  by an effect analogous to the Galerkin orthogonality. For this reason, the functional space used to define the semidiscrete problem (21) is  $\mathcal{V}_0^{H,p+1}$  instead of  $\mathcal{V}_0^{H,p}$ .*

A modal-based approximation of problem (21) requires introducing the generalized eigenvalue problem: find  $(\tilde{\omega}, \tilde{\mathbf{q}}) \in \mathbb{R} \times \mathcal{V}_0^{H,p+1}$  such that

$$a(\tilde{\mathbf{q}}, \mathbf{w}) = (\tilde{\omega})^2 m(\tilde{\mathbf{q}}, \mathbf{w}) \quad \forall \mathbf{w} \in \mathcal{V}_0^{H,p+1}. \quad (22)$$

The  $i$ -th eigenpair solution of this problem is referred as  $(\tilde{\omega}_i, \tilde{\mathbf{q}}_i)$ . Note that the number of eigenpairs is the number of degrees of freedom in the functional space  $\mathcal{V}_0^{H,p+1}$ , denoted by  $N_{\text{dof}}$ . Typically, the eigenpairs are sorted from low to high frequencies, namely  $\tilde{\omega}_1 \leq \tilde{\omega}_2 \leq \dots \leq \tilde{\omega}_{N_{\text{dof}}}$ , and eigenvectors are normalized to be orthonormal with respect the product  $m(\cdot, \cdot)$ , i.e.

$$m(\tilde{\mathbf{q}}_i, \tilde{\mathbf{q}}_j) = \delta_{ij}, \quad 1 \leq i, j \leq N_{\text{dof}}. \quad (23)$$

The complexity of the system of ODEs resulting from (21) is considerably reduced by expressing the semidiscrete adjoint solution  $\mathbf{u}^{\text{d},H,p+1}(\mathbf{x}, t)$  as a combination of the eigenvectors  $\tilde{\mathbf{q}}_i, i = 1, \dots, N_{\text{dof}}$ , namely

$$\mathbf{u}^{\text{d},H,p+1}(\mathbf{x}, t) = \sum_{i=1}^{N_{\text{dof}}} \tilde{\mathbf{q}}_i(\mathbf{x}) \tilde{y}_i(t). \quad (24)$$

Thus, the system of ODEs (21) is transformed into the uncoupled set of scalar ordinary differential equations (which can be solved analytically in many practical cases)

$$\ddot{\tilde{y}}_i - [a_1 + a_2(\tilde{\omega}_i)^2]\dot{\tilde{y}}_i + (\tilde{\omega}_i)^2\tilde{y}_i = -\tilde{l}_i, \quad (25a)$$

$$\tilde{y}_i(T) = \tilde{u}_i, \quad (25b)$$

$$\dot{\tilde{y}}_i(T) = \tilde{v}_i, \quad (25c)$$

where the r.h.s. terms  $\tilde{l}_i$ ,  $\tilde{u}_i$  and  $\tilde{v}_i$  are computed using the data characterizing the quantity of interest (35) and the eigenvector  $\tilde{\mathbf{q}}_i$ ,

$$\tilde{l}_i(t) := (\mathbf{f}^\mathcal{O}(t), \tilde{\mathbf{q}}_i) + (\mathbf{g}^\mathcal{O}(t), \tilde{\mathbf{q}}_i)_{\Gamma_N}, \quad u_i := m(\mathbf{u}^\mathcal{O}, \tilde{\mathbf{q}}_i) \quad \text{and} \quad v_i := m(\mathbf{v}^\mathcal{O}, \tilde{\mathbf{q}}_i). \quad (26)$$

The cost of computing  $M$  vibration modes scales as, see reference by Bathe (1996),

$$\mathcal{O}(N_{\text{dof}} \cdot N_{\text{bw}}^2) + \mathcal{O}(N_{\text{dof}} \cdot N_{\text{bw}} \cdot M) + \mathcal{O}(N_{\text{dof}} \cdot M^2),$$

where  $N_{\text{bw}}$  denotes the half-bandwidth of the finite element matrices associated with the functional space  $\mathcal{V}_0^{H,p+1}$ . Thus, the modal-based approach is not computationally affordable unless the modal description (24) is truncated up to the first  $M$  terms, being  $M \ll N_{\text{dof}}$ . Consequently, the adjoint approximation  $\tilde{\mathbf{u}}^d$  is defined as the truncated expansion

$$\tilde{\mathbf{u}}^d(\mathbf{x}, t) := \sum_{i=1}^M \tilde{\mathbf{q}}_i(\mathbf{x}) \tilde{y}_i(t). \quad (27)$$

Note that the number of required vibration modes  $M$  has to be selected such that the truncated high frequency modes (for  $i > M$ ) are negligible in (24). This is equivalent to assume that for  $i > M$  the values of  $\tilde{l}_i$ ,  $\tilde{u}_i$  and  $\tilde{v}_i$ , as defined in (26), are close to zero, and consequently  $\tilde{y}_i(t) \approx 0$ . This is guaranteed if the data  $\mathbf{f}^\mathcal{O}$ ,  $\mathbf{g}^\mathcal{O}$ ,  $\mathbf{u}^\mathcal{O}$  and  $\mathbf{v}^\mathcal{O}$  are well captured by the expansion of the first  $M$  eigenvectors.

The optimal choice to get an efficient response with the modal-based approach is selecting a quantity of interest defined using only the first vibration mode. For instance,

$$L^\mathcal{O}(\mathbf{w}) := m(\alpha_v \tilde{\mathbf{q}}_1, \dot{\mathbf{w}}(T)) + a(\alpha_u \tilde{\mathbf{q}}_1, \mathbf{w}(T)), \quad (28)$$

corresponding to take  $\mathbf{f}^\mathcal{O} = \mathbf{g}^\mathcal{O} = \mathbf{0}$ ,  $\mathbf{v}^\mathcal{O} = \alpha_v \tilde{\mathbf{q}}_1$  and  $\mathbf{u}^\mathcal{O} = \alpha_u \tilde{\mathbf{q}}_1$  in equation (3). The constants  $\alpha_v$  and  $\alpha_u$  are introduced in order to obtain consistent dimensions in (28). This quantity is a sum of averages of velocities and strains (or stresses) at time  $T$ . It can be interpreted as the projection of function  $\mathbf{w}$  to the first vibration mode at time  $T$ . This quantity of interest is computationally inexpensive because requires computing only one vibration mode ( $M = 1$ ).

A second choice is defining a quantity using the first  $M$  vibration modes, namely

$$L^{\mathcal{O}}(\mathbf{w}) = \sum_{i=1}^M (m(\alpha_{v,i}\mathbf{q}_i, \dot{\mathbf{w}}(T)) + a(\alpha_{u,i}\mathbf{q}_i, \mathbf{w}(T))),$$

corresponding to take  $\mathbf{f}^{\mathcal{O}} = \mathbf{g}^{\mathcal{O}} = \mathbf{0}$ ,  $\mathbf{v}^{\mathcal{O}} = \sum_{i=1}^M \alpha_{v,i}\tilde{\mathbf{q}}_i$  and  $\mathbf{u}^{\mathcal{O}} = \sum_{i=1}^M \alpha_{u,i}\tilde{\mathbf{q}}_i$  in equation (3). This quantity of interest represents more meaningful averages of velocities and strains at the final time  $T$ . For instance, a pseudo average of the velocities at time  $t = T$  in a subregion of the computational domain  $\Omega$  can be defined by properly selecting the coefficients  $\alpha_{v,i}$  and  $\alpha_{u,i}$ , see the first numerical example in paper D.

A third suitable quantity of interest is the average of displacements at the final time of the computation

$$L^{\mathcal{O}}(\mathbf{w}) := (\boldsymbol{\lambda}^{\mathcal{O}}, \mathbf{w}(T)) + (\boldsymbol{\lambda}_{\Gamma_N}^{\mathcal{O}}, \mathbf{w}(T))_{\Gamma_N}, \quad (29)$$

where the data  $\boldsymbol{\lambda}^{\mathcal{O}}$  and  $\boldsymbol{\lambda}_{\Gamma_N}^{\mathcal{O}}$  are weighting functions allowing to localize the average of displacements in some subdomains in  $\Omega$  and  $\Gamma_N$  respectively. The quantity (29) has to be rewritten in the same form as the generic quantity (3) in order to compute its associated enhanced approximation  $\tilde{\mathbf{u}}^{\mathcal{d}}$  using the rationale presented above. Thus, the quantity (29) is rewritten as

$$L^{\mathcal{O}}(\mathbf{w}) = a(\tilde{\mathbf{u}}^{\mathcal{O}}, \mathbf{u}(T)),$$

taking  $\mathbf{f}^{\mathcal{O}} = \mathbf{0}$ ,  $\mathbf{g}^{\mathcal{O}} = \mathbf{0}$ ,  $\mathbf{v}^{\mathcal{O}} = \mathbf{0}$  and  $\mathbf{u}^{\mathcal{O}} = \tilde{\mathbf{u}}^{\mathcal{O}}$  in equation (3), being  $\tilde{\mathbf{u}}^{\mathcal{O}}$  the solution of the static problem: find  $\tilde{\mathbf{u}}^{\mathcal{O}} \in \mathcal{V}_0^{H,p+1}$  such that

$$a(\tilde{\mathbf{u}}^{\mathcal{O}}, \mathbf{w}) = (\boldsymbol{\lambda}^{\mathcal{O}}, \mathbf{w}) + (\boldsymbol{\lambda}_{\Gamma_N}^{\mathcal{O}}, \mathbf{w})_{\Gamma_N} \quad \forall \mathbf{w} \in \mathcal{V}_0^{H,p+1}. \quad (30)$$

This quantity is more meaningful than the previous ones, but it requires computing several vibration modes ( $M > 1$ ) in order to properly capture  $\tilde{\mathbf{u}}^{\mathcal{O}}$  by the expansion of  $\tilde{\mathbf{q}}_i$ ,  $i = 1, \dots, M$ .

The numerical examples in paper C show that the proposed modal-based error estimate is accurate and accounts for both the space and time discretization errors. As previously announced, assessing the error in the quantity of interest (29) requires computing  $M > 1$  vibration modes. The results in paper C show that  $M = 60$  vibration modes provide an accurate error assessment in the examples considered therein.

The modal-based error estimate is the basis of the contributions presented in the following sections 3.3 and 3.4.

### 3.3 Modal-based goal-oriented adaptivity

The proposed adaptive technique is similar to the one discussed by Bangerth and Rannacher (1999), and Bangerth et al. (2010). The major novelty of the present approach is using modal analysis to approximate the adjoint solution instead of direct time-integration methods. The specific details of the technique are found in paper D.

The proposed space-time adaptive strategy aims at finding an optimal time discretization  $\mathcal{T}$  and an optimal space discretization  $\mathcal{P}_n$  at each time point  $t_n \in \mathcal{T}$  such that the assessed error  $\tilde{s}^e$  is under a user-defined tolerance  $s_{\text{tol}}^e$ , namely

$$|\tilde{s}^e| \leq s_{\text{tol}}^e. \quad (31)$$

Two main ingredients are required to achieve the optimal space-time discretization: 1) a procedure allowing to locally refine and un-refine the space and time discretizations, and 2) local error information allowing to identify which regions of the space-time domain have larger (or smaller) error contributions and therefore which regions have to be refined (or unrefined).

Adding and removing points form the time grid  $\mathcal{T}$  is trivial because the time interval  $I$  is a one dimensional domain. However, the strategy to refine and un-refine the space meshes  $\mathcal{P}_n$  is more involved. Here, a hierarchical tree-based mesh refinement strategy similar to the ones proposed by Demkowicz et al. (1989), and Yerry and Shephard (1983) is considered. In this context, the computational meshes  $\mathcal{P}_n$ ,  $n = 0, \dots, N$  are obtained recursively splitting the elements of an initial background mesh denoted as  $\mathcal{P}_{\text{bg}}$ , see figure 6. The particular refinement and un-refinement technique adopted here is detailed in paper E.

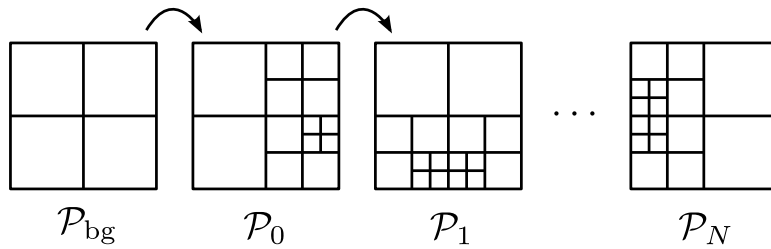


Figure 6: A hierarchical tree-based technique is used to build the space meshes  $\mathcal{P}_n$ ,  $n = 0, \dots, N$  starting from the background mesh  $\mathcal{P}_{\text{bg}}$ .

The tree-based data structure enormously facilitates the mesh refinement and un-refinement operations and also transferring information between different meshes.

However, this approach introduces *hanging* or *irregular* nodes. A constrained finite element approximation (i.e. Lagrange multipliers) is used to enforce the continuity of the finite element solution at the hanging nodes, see paper D.

The local information driving the adaptive procedure is given by the modal-based error estimate  $\tilde{s}^e$ . Note that the value  $\tilde{s}^e$  accounts for both the space and time discretization errors. Therefore, this single value does not give enough information allowing to decide whether the space or time discretizations (or both) have to be refined to reduce error.

In order to properly split the space and time error components, the adaptive strategy requires that the numerical solution fulfills the discrete version of a space-time variational problem. With this choice, a weak residual (integrated both in space and time) associated with the numerical solution is readily introduced. The splitting procedure uses the fact that the residual vanishes for the functions in the test space, that is Galerkin orthogonality holds.

Among the possible space-time variational formulations available for transient elastodynamics, the double field *time-continuous Galerkin method* considered by Eriksson et al. (1996), and Bangerth et al. (2010) is the selected numerical solver. Note, however, that the following rationale can be easily extended to other space-time variational formulations, for instance, the one proposed by Johnson (1993) or the one proposed by Hughes and Hulbert (1988), and Hulbert and Hughes (1990).

The selected double-field method introduces the velocity  $\dot{\mathbf{u}}$  as a new unknown of the problem. The numerical approximation is a pair  $\tilde{\mathbf{U}} := [\tilde{\mathbf{u}}_u, \tilde{\mathbf{u}}_v]$  consisting of independent approximations for displacements  $\tilde{\mathbf{u}}_u \approx \mathbf{u}$  and velocities. That is, the velocity  $\mathbf{u}_v$  is not strongly enforced to coincide with  $\dot{\mathbf{u}}_u$ . The relation between the approximated displacements and velocities is imposed weakly and therefore it yields  $\tilde{\mathbf{u}}_v \approx \dot{\mathbf{u}}$  instead of  $\tilde{\mathbf{u}}_v = \dot{\mathbf{u}}$ . The discretization error is defined as a double field function as well,

$$\mathbf{E} := [\mathbf{e}_u, \mathbf{e}_v] := [\mathbf{u} - \tilde{\mathbf{u}}_u, \dot{\mathbf{u}} - \tilde{\mathbf{u}}_v],$$

where  $\mathbf{e}_u$  and  $\mathbf{e}_v$  are the errors in displacements and velocities respectively.

The methodology used to assess the double field error  $\mathbf{E}$  is completely analogous to the one used for the single field error  $\mathbf{e}$ . The only difference is a technical modification in the definition of the quantity of interest  $L^{\mathcal{O}}(\cdot)$  and the residual  $R(\cdot)$  in order to

take as input a generic double field function  $\mathbf{W} = [\mathbf{w}_u, \mathbf{w}_v]$ . That is

$$\begin{aligned}
 L_{\text{DF}}^{\mathcal{O}}(\mathbf{W}) &:= \int_0^T l^{\mathcal{O}}(\mathbf{w}_v(t); t) dt \\
 &\quad + m(\mathbf{v}^{\mathcal{O}}, \mathbf{w}_v(T)) + a(\mathbf{u}^{\mathcal{O}}, \mathbf{w}_u(T)), \\
 R_{\text{DF}}(\mathbf{W}) &:= \int_0^T l(\mathbf{w}_v(t); t) dt + \\
 &\quad - \int_0^T m(\dot{\tilde{\mathbf{u}}}_v(t) + a_1 \tilde{\mathbf{u}}_v(t), \mathbf{w}_v(t)) dt \\
 &\quad - \int_0^T a(\tilde{\mathbf{u}}_u(t) + a_2 \tilde{\mathbf{u}}_v(t), \mathbf{w}_v(t)) dt \\
 &\quad - \int_0^T a(\dot{\tilde{\mathbf{u}}}_u(t) - \tilde{\mathbf{u}}_v(t), \mathbf{w}_u(t)) dt \\
 &\quad + m(\mathbf{v}_0 - \tilde{\mathbf{u}}_v(0^+), \mathbf{w}_v(0^+)) + a(\mathbf{u}_0 - \tilde{\mathbf{u}}_u(0^+), \mathbf{w}_u(0^+)).
 \end{aligned}$$

Note that the definition of the double-field quantity of interest  $L_{\text{DF}}^{\mathcal{O}}(\cdot)$  is consistent with the single-field quantity  $L^{\mathcal{O}}(\cdot)$  in the sense that, for a generic function  $\mathbf{w}$ ,  $L^{\mathcal{O}}(\mathbf{w}) = L_{\text{DF}}^{\mathcal{O}}([\mathbf{w}, \dot{\mathbf{w}}])$ . Note also that the residual  $R_{\text{DF}}(\cdot)$  has an extra term accounting for the non-verification of the relation between displacements and velocities.

The error in the quantity of interest  $s_{\text{DF}}^e := L_{\text{DF}}^{\mathcal{O}}(\mathbf{E})$  is assessed using an error representation analogous to equation (20), namely

$$L_{\text{DF}}^{\mathcal{O}}(\mathbf{E}) \approx R_{\text{DF}}(\tilde{\mathbf{U}}^d) =: \tilde{s}_{\text{DF}}^e, \quad (32)$$

where  $\tilde{\mathbf{U}}^d := [\tilde{\mathbf{u}}^d, \dot{\tilde{\mathbf{u}}}^d]$ . The adjoint solution  $\tilde{\mathbf{U}}^d$  is computed with the modal-based approach using the background mesh  $\mathcal{P}_{\text{bg}}$  (with element of order  $p + 1$ ). That is,  $\tilde{\mathbf{u}}^d(t), \dot{\tilde{\mathbf{u}}}^d(t) \in \mathcal{V}_0^{\text{bg}, p+1}$ , for  $t \in I$ , where  $\mathcal{V}_0^{\text{bg}, p+1}$  denotes the finite element space associated with the  $p + 1$  version of the background mesh. The background mesh  $\mathcal{P}_{\text{bg}}$  is selected for approximating the adjoint in order to simplify the representation of  $\tilde{\mathbf{U}}^d$  in the adapted meshes  $\mathcal{P}_n$ ,  $n = 0, \dots, N$ . As previously announced, having a  $p + 1$  approximation degree for the adjoint solution  $\tilde{\mathbf{U}}^d$  precludes the Galerkin orthogonality effect and the corresponding underestimation of the error. For the sake of a simpler notation, the subscript DF is omitted in the following.

Separating the space and time error contributions to  $\tilde{s}^e$  requires introducing the projection operators  $\mathbf{\Pi}^H$  and  $\mathbf{\Pi}^{\Delta t}$  associated with the space and time discretizations. The space projection  $\mathbf{\Pi}^H$  applied to a generic double field function  $\mathbf{W}$  returns a function  $\mathbf{\Pi}^H \mathbf{W}$  which is discrete in space (i.e. a continuous piecewise polynomial)

but not discrete in time. On the other hand, the time projection  $\mathbf{\Pi}^{\Delta t}$  returns a function which is discrete in time (a piecewise constant function) but not discrete in space, see figure 7. A formal definition of the operators  $\mathbf{\Pi}^H$  and  $\mathbf{\Pi}^{\Delta t}$  is given in paper D.

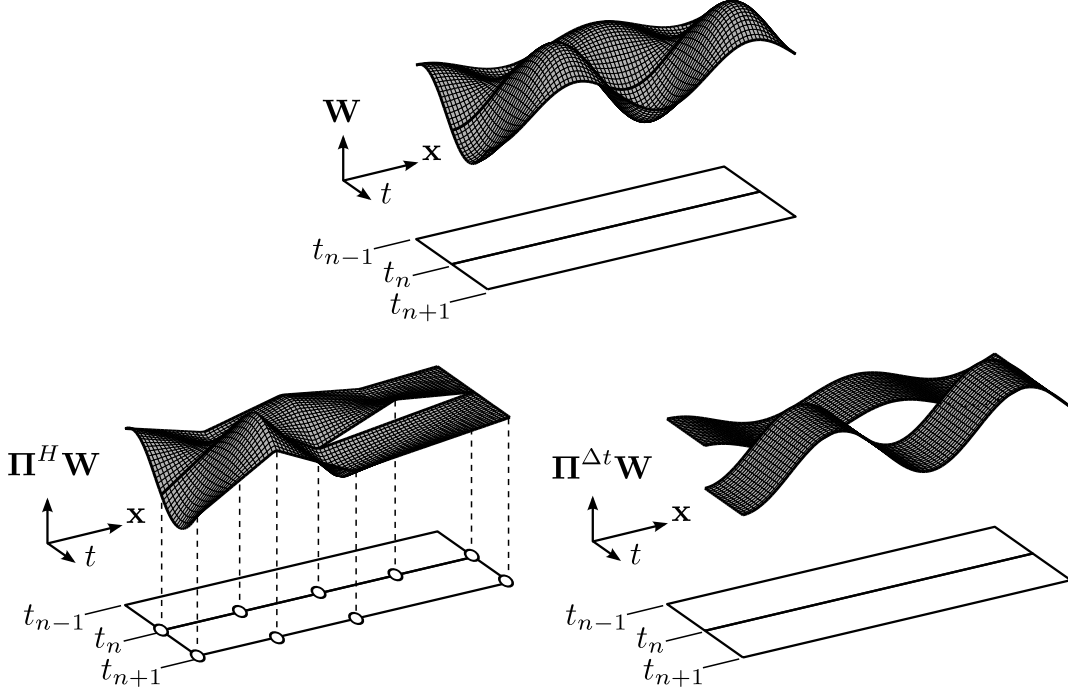


Figure 7: Illustration of the projection operators  $\mathbf{\Pi}^H$  and  $\mathbf{\Pi}^{\Delta t}$ . The figure displays (one field of) the original function  $\mathbf{W}$  inside the time intervals  $I_n = (t_{n-1}, t_n]$  and  $I_{n+1} = (t_n, t_{n+1}]$  (top) along with its projections in space and time  $\mathbf{\Pi}^H \mathbf{W}$  (left) and  $\mathbf{\Pi}^{\Delta t} \mathbf{W}$  (right).

The space and time errors are separated rewriting the value  $\tilde{s}^e$  as

$$\begin{aligned} \tilde{s}^e &= R(\tilde{\mathbf{U}}^d) \\ &= R(\tilde{\mathbf{U}}^d) - R(\mathbf{\Pi}^H \mathbf{\Pi}^{\Delta t} \tilde{\mathbf{U}}^d) \quad (\text{Galerkin orthogonality}) \\ &= R(\tilde{\mathbf{U}}^d) - R(\mathbf{\Pi}^H \tilde{\mathbf{U}}^d) + R(\mathbf{\Pi}^H \tilde{\mathbf{U}}^d) - R(\mathbf{\Pi}^H \mathbf{\Pi}^{\Delta t} \tilde{\mathbf{U}}^d). \end{aligned}$$

Hence, using the linearity of the residual  $R(\cdot)$  one has

$$\tilde{s}^e = \tilde{s}_s^e + \tilde{s}_t^e, \quad (33)$$

where  $\tilde{s}_s^e := R(\tilde{\mathbf{U}}^d - \mathbf{\Pi}^H \tilde{\mathbf{U}}^d)$  and  $\tilde{s}_t^e := R(\mathbf{\Pi}^H \tilde{\mathbf{U}}^d - \mathbf{\Pi}^H \mathbf{\Pi}^{\Delta t} \tilde{\mathbf{U}}^d)$ . The terms  $\tilde{s}_s^e$  and  $\tilde{s}_t^e$  are associated with the space and time discretization errors respectively. Note

that  $s_s^e$  tends to zero as the space discretization is refined because  $\mathbf{\Pi}^H \mathbf{U}^d$  tends to  $\mathbf{U}^d$ . Similarly,  $s_t^e$  tends to zero as the time discretization is refined because  $\mathbf{\Pi}^{\Delta t} \mathbf{U}^d$  tends to  $\mathbf{U}^d$ . The space and time error components  $s_s^e$  and  $s_t^e$  are used as refinement indicators because they can be reduced independently by respectively enriching the space and time discretizations. Note that the Galerkin orthogonality property is used to separate the space and time error contributions.

The sub-regions of the space-time domain  $\Omega \times I$  having a larger error contribution are identified by restricting the integrals involved in the values  $\tilde{s}_s^e$  and  $\tilde{s}_t^e$  to the mesh elements and time steps. This allows to adaptively refine the time grid  $\mathcal{T}$  and the finite element meshes  $\mathcal{P}_n$ ,  $n = 0, \dots, N$  to meet the desired error in the quantity of interest  $s_{\text{tol}}^e$ .

Note that changing the space discretization at each time step  $t_n \in \mathcal{T}$  is not computationally affordable. This is because remeshing operations, matrix assembly and data transfer between different meshes are costly operations and cannot, in general, be performed at each time step. Here, an adaptive strategy organized in time-blocks, similar to the one proposed by Carey et al. (2010), is adopted in order to reduce the number of mesh changes.

The blockwise adaptive strategy requires splitting the time interval  $I$  into  $N^{\text{bk}}$  time intervals (or time blocks), namely

$$I_m^{\text{bk}} := \left( \frac{T}{N^{\text{bk}}}(m-1), \frac{T}{N^{\text{bk}}}m \right], \quad m = 1, \dots, N^{\text{bk}}.$$

The blockwise adaptive strategy consists in taking the same space mesh inside each time interval  $I_m^{\text{bk}}$ , this mesh is denoted as  $\mathcal{P}_m^{\text{bk}}$  for  $m = 1, \dots, N^{\text{bk}}$ , see figure 8. Note that with this definition the computational meshes  $\mathcal{P}_n$  associated with the time points  $t_n \in I_m^{\text{bk}}$  are such that  $\mathcal{P}_n = \mathcal{P}_m^{\text{bk}}$ .

Additionally, the time step length is assumed to be constant inside the intervals  $I_m^{\text{bk}}$  and denoted by  $\Delta t_m^{\text{bk}}$ . Consequently, the time step length  $\Delta t_n$  associated with times  $t_n \in I_m^{\text{bk}}$  are such that  $\Delta t_n = \Delta t_m^{\text{bk}}$ , see figure 8.

Following this approach and notation, the adaptive strategy is reformulated as computing the optimal space meshes  $\mathcal{P}_m^{\text{bk}}$  and time step lengths  $\Delta t_m^{\text{bk}}$ , for all the time intervals  $I_m^{\text{bk}}$ ,  $m = 1, \dots, N^{\text{bk}}$  such that the associated numerical solution fulfills (31).

Once the adjoint solution is computed and stored, the main stages of the adaptive procedure are summarized as follows. The numerical solution is computed sequentially starting from the first time block  $I_1^{\text{bk}}$  until the last one  $I_{N^{\text{bk}}}^{\text{bk}}$ . In each time slab



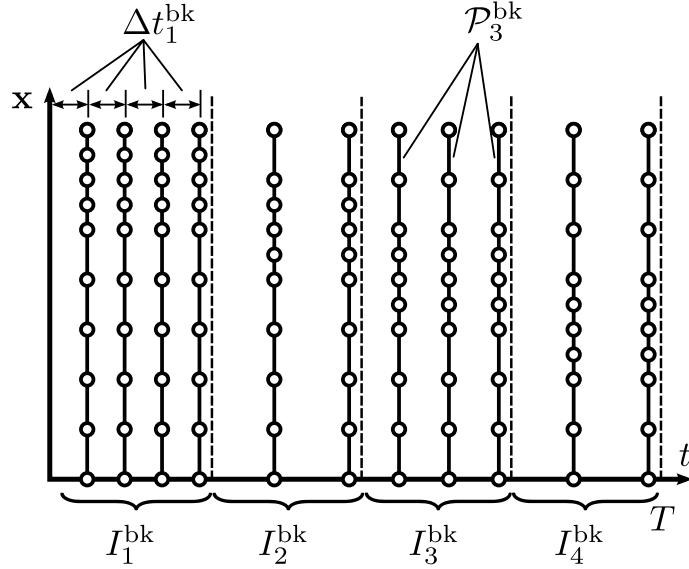


Figure 8: The space mesh is assumed to be constant inside the time intervals  $I_m^{\text{bk}}$ . In the same way, the time step length is also assumed to be constant inside each interval  $I_m^{\text{bk}}$ .

$I_m^{\text{bk}}$ , the numerical solution is computed and the corresponding local error contributions are estimated. The computed solution in  $I_m^{\text{bk}}$  is *accepted* or *rejected* using the information given by the local error contributions. The specific acceptability criterion is detailed in paper D. If the solution is accepted, the loop goes to the following time interval  $I_{m+1}^{\text{bk}}$ . Else, the space or time discretization (or both) associated with the interval  $I_m^{\text{bk}}$  are adapted using the local error information and the solution is re-computed in  $I_m^{\text{bk}}$ . The process of adapting the discretization and computing the numerical solution is repeated in the interval  $I_m^{\text{bk}}$  until the solution is accepted, see algorithm 1.

The numerical examples in paper D show that the proposed strategy furnishes adapted solutions fulfilling the user-defined error tolerance. That is, both the assessed and computed errors are below the user-defined error value. Moreover, the discretizations obtained with the proposed adaptive strategy are more efficient than the ones obtained with a uniform refinement of all mesh elements and time steps. The adapted discretizations give more accurate results than the non-adaptive ones for the same number of space-time elements.

**Remark 6** (Illustrative example). *This example illustrates the performance of the*

**Data:**

*Problem statement:* Problem geometry  $(\Omega, \Gamma_N, \Gamma_D)$ , final time  $(T)$ , material data  $(E, \nu, \rho)$ , loads and initial conditions  $(\mathbf{f}, \mathbf{g}, \mathbf{u}_0, \mathbf{v}_0)$ .

*Problem discretization:* background computational mesh  $(\mathcal{P}_{\text{bg}})$ .

*Error control:* data defining the quantity of interest  $(\mathbf{f}^\mathcal{O}, \mathbf{g}^\mathcal{O}, \mathbf{u}^\mathcal{O}, \mathbf{v}^\mathcal{O})$  and number of vibration modes  $M$ .

*Adaptivity parameters:* Number of time blocks  $(N^{\text{bk}})$ , prescribed error  $(s_{\text{tol}}^e)$  and other tuning parameters.

**Result:** Numerical approximation  $\tilde{\mathbf{U}}$ ; adapted space meshes  $\mathcal{P}_m^{\text{bk}}$  and time steps  $\Delta t_m^{\text{bk}}, m = 1, \dots, N^{\text{bk}}$ ; and error estimate  $\tilde{s}^e$  fulfilling  $|\tilde{s}^e| \leq s_{\text{tol}}^e$ .

// Modal analysis

Generate higher order space  $\mathcal{V}_0^{\text{bg}, p+1}$ ;

Compute the eigenpairs  $(\tilde{\omega}_i, \tilde{\mathbf{q}}_i), i = 1, \dots, M$  in the space  $\mathcal{V}_0^{\text{bg}, p+1}$ ;

// Adjoint problem (modal solution)

Compute the values  $\tilde{l}_i, \tilde{u}_i, \tilde{v}_i$  (using  $\mathbf{f}^\mathcal{O}, \mathbf{g}^\mathcal{O}, \mathbf{u}^\mathcal{O}, \mathbf{v}^\mathcal{O}$  and  $\tilde{\mathbf{q}}_i, i = 1, \dots, M$ );

Compute the time dependent functions  $\tilde{y}_i(t)$  (using  $\tilde{l}_i, \tilde{u}_i, \tilde{v}_i$  and  $\tilde{\omega}_i, i = 1, \dots, M$ );

// Problem computation, error assessment and adaptivity

Initialize discretization:  $\mathcal{P}_1^{\text{bk}} = \mathcal{P}_{\text{bg}}, \Delta t_1^{\text{bk}} = T/N^{\text{bk}}$ ;

**for**  $m = 1 \dots N^{\text{bk}}$  **do**

**repeat**

        // Compute solution and error estimate

        Compute solution  $\tilde{\mathbf{U}}$  in the time interval  $I_m^{\text{bk}}$  and estimate error contributions;

        // Mesh adaptivity

**if** *The solution is not accepted in  $I_m^{\text{bk}}$*  **then**

            Refine/unrefine the spatial mesh  $\mathcal{P}_m^{\text{bk}}$  and/or the time step  $\Delta t_m^{\text{bk}}$ ;

**end**

**until** *The solution is accepted in  $I_m^{\text{bk}}$* ;

    Set initial discretization for the next time interval:  $\mathcal{P}_{m+1}^{\text{bk}} = \mathcal{P}_m^{\text{bk}}$ ,

$\Delta t_{m+1}^{\text{bk}} = \Delta t_m^{\text{bk}}$ ;

**end**

Algorithm 1: Algorithm for problem approximation with error control and space-time mesh adaptivity.

*proposed space-time adaptive strategy in a 2D wave propagation problem. This example is a shorter version than the first numerical example in the appended paper D. The computational domain  $\Omega$  is a perforated rectangular plate,  $\Omega := (-0.5, 0.5) \times (0, 0.5) \setminus \Omega_0$  m<sup>2</sup>, with  $\Omega_0 := \{(x, y) \in \mathbb{R}^2 : x^2 + (y - 0.25)^2 \leq 0.025^2\}$  m<sup>2</sup>, see figure 9.*

The plate is clamped at the bottom side and the horizontal displacement is blocked at both vertical sides. The plate is initially at rest,  $\mathbf{u}_0 = \mathbf{v}_0 = \mathbf{0}$ , and loaded with the time dependent traction

$$\mathbf{g}(t) = \begin{cases} -g(t)\mathbf{e}_2 & \text{on } \Gamma_g, \\ 0 & \text{elsewhere,} \end{cases} \quad (34)$$

with  $\Gamma_g := (-0.025, 0.025) \times \{0.5\}$  m,  $\mathbf{e}_2 := (0, 1)$  and  $g(t)$  is the impulsive time-dependent function defined in figure 9 with parameters  $g_{\max} = 30$  Pa and  $t_g = 0.005$  s. No body force is acting in this example,  $\mathbf{f} = \mathbf{0}$ . The material properties of the plate are Young's modulus  $E = 8/3$  Pa, Poisson's ratio  $\nu = 1/3$ , the density  $\rho = 1$  kg/m<sup>3</sup> and the damping coefficients  $a_1 = 0$  d<sup>-1</sup>,  $a_2 = 10^{-4}$  s. The final simulation time is  $T = 0.25$  s. The plain stress hypothesis is considered.

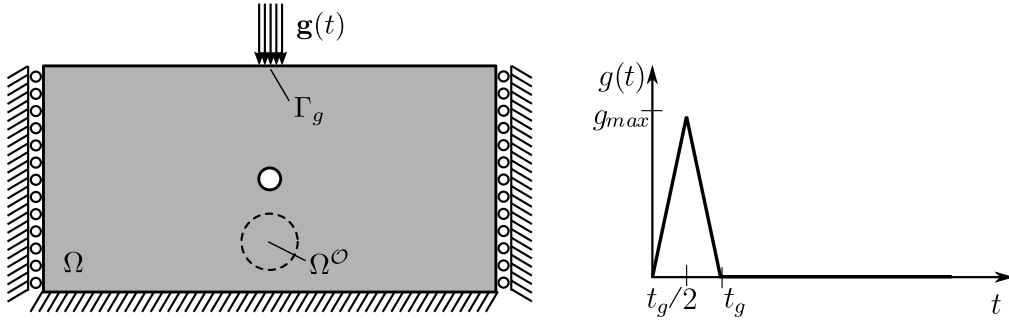


Figure 9: Definition of the problem geometry (left) and time-dependence of the external load (right).

The quantity of interest considered in this example is a weighted average of the velocities, namely

$$L^\circ(\mathbf{W}) := m(\mathbf{v}^\circ, \mathbf{w}_v(T)).$$

The weighting function  $\mathbf{v}^\circ$  considered here is plotted in figure 10 and defines a pseudo average of the vertical component of the final velocity near the region of interest  $\Omega^\circ := \{(x, y \in \mathbb{R}^2 : x^2 + (y - 0.1)^2 < 0.075^2)\}$  m<sup>2</sup>, see figure 9. Note that the  $x$ -component of  $\mathbf{v}^\circ$  is small compared to its  $y$ -component and, moreover, function  $\mathbf{v}^\circ$  takes larger values near  $\Omega^\circ$ . The weighting function  $\mathbf{v}^\circ$  is exactly represented using the expansion of the first  $M = 60$  vibration modes of the problem. Thus, the quantity of interest is well suited for the modal-based error estimate, because only  $M = 60$  vibration modes have to be computed to approximate the adjoint solution.

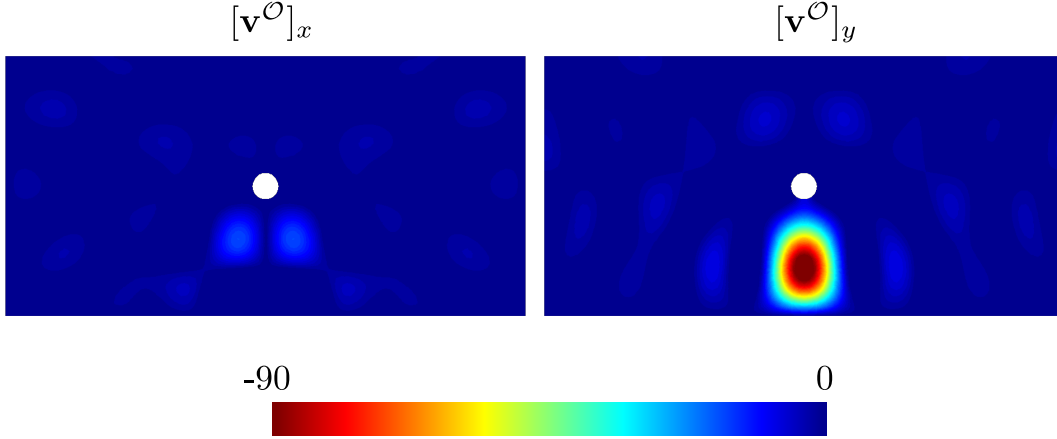


Figure 10: Weighting function  $\mathbf{v}^{\mathcal{O}}$  ( $x$ - and  $y$ -components) defining the quantity of interest  $L^{\mathcal{O}}(\cdot)$ .

Figure 11 shows several snapshots of an adapted numerical solution obtained with the proposed methodology imposing a target error  $s_{\text{tol}}^e = 5 \cdot 10^{-5}$  m/s corresponding to a 2.1% of the computed quantity  $\tilde{s} := L^{\mathcal{O}}(\tilde{\mathbf{U}}) = 2.4242 \cdot 10^{-2}$  m/s. The particular description of all the parameters used in this computation are found in paper D. The prescribed target error is fulfilled quite sharply with respect to the assessed error. That is, the assessed error  $\tilde{s}^e = -1.5756 \cdot 10^{-5}$  m/s fulfills  $|\tilde{s}^e| \leq s_{\text{tol}}^e$ , but  $|\tilde{s}^e|$  and  $s_{\text{tol}}^e$  are of the same order of magnitude. Moreover, the error with respect to an overkill solution, namely  $s_{\text{ovk}}^e := -1.5125 \cdot 10^{-5}$  m/s, is also below (in absolute value) the user-defined value  $s_{\text{tol}}^e$ . Note that the assessed error is a good approximation of the overkill error. That is, the effectivity of the error estimate,  $\tilde{s}^e/s_{\text{ovk}}^e = 1.041$ , is near one. The methodology to compute the overkill solution is also detailed in paper D.

Figure 12 shows the convergence of the estimates. As expected, the use of an adaptive refinement strategy leads to better approximations for the quantity of interest with less computational cost. The adapted solutions have a lower error than the uniform approximations for the same number of space-time cells.

### 3.4 Error assessment for timeline-quantities of interest

Selecting a scalar quantity of interest in time-dependent problems it is not always obvious. This is because, in many cases, a single scalar value does not give enough

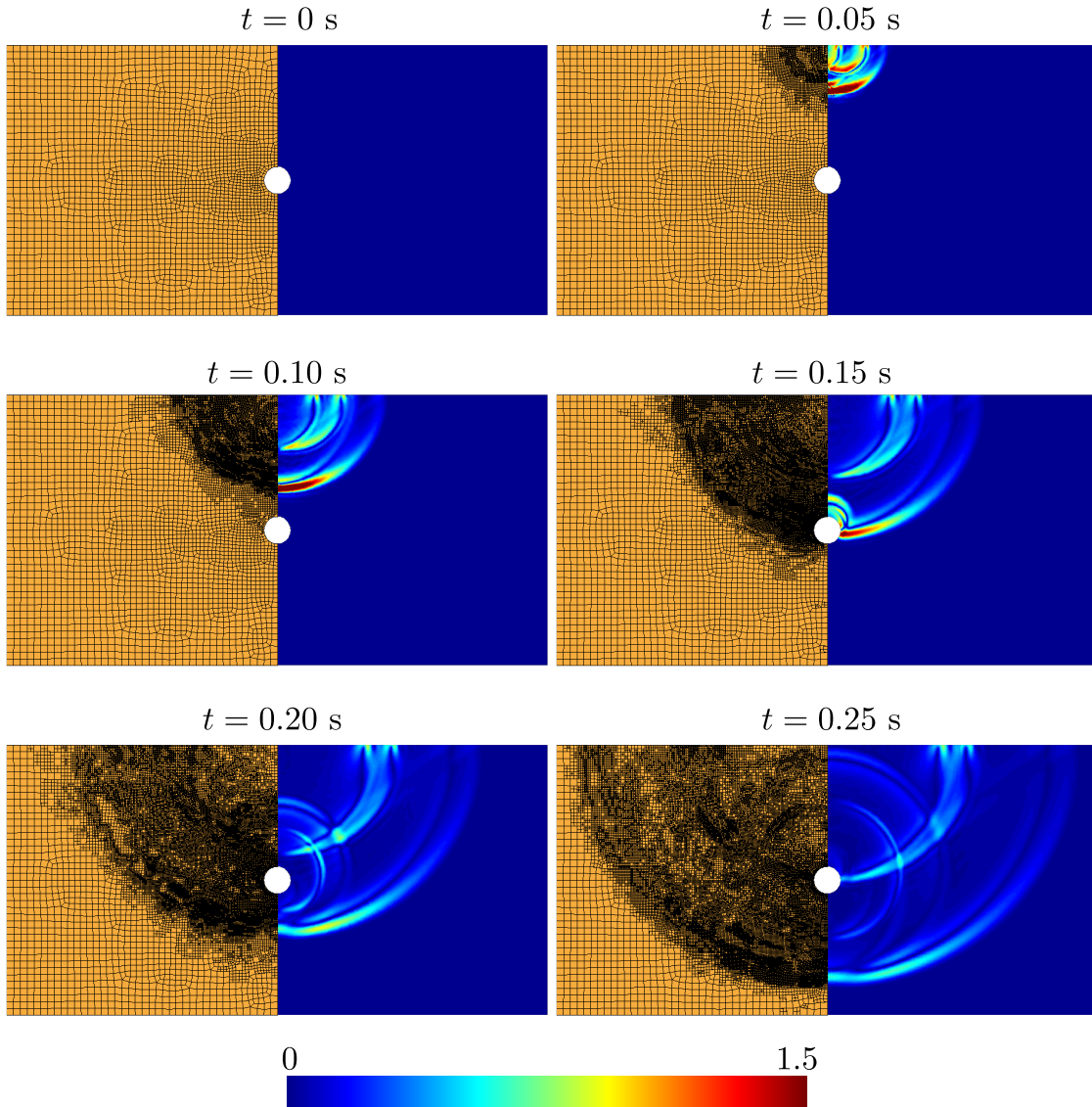


Figure 11: Snapshots of the computed solution (magnitude of velocities in m/s) and the computational mesh at several time points for the adapted solution verifying the prescribed target error  $s_{\text{tol}}^e = 5 \cdot 10^{-5}$  m/s.

pieces of information about a space-time solution. The preferred quantities of interest in time-dependent problems are typically the history (or evolution) of the space-average of the solution in a subregion of the domain. These quantities are time-dependent functions instead of single scalar values. This suggests introducing the so called timeline-dependent quantities of interest for error assessment in transient

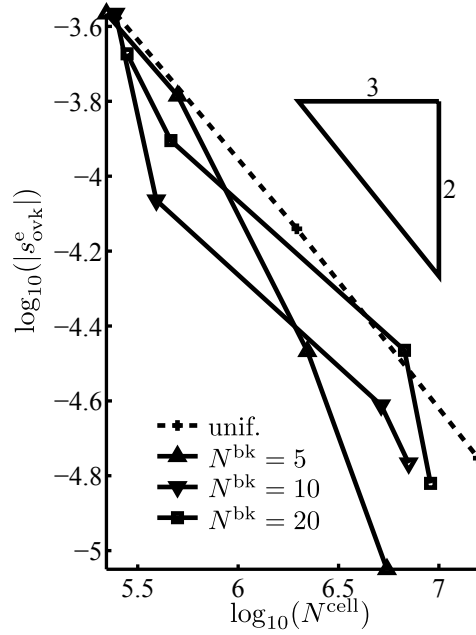


Figure 12: Error convergence for the adapted and uniform computations vs the number of used space-time cells ( $N^{\text{cells}}$ ). The adaptive solutions are obtained using three different number of time blocks  $N^{\text{bk}}$ .

problems.

Timeline-dependent quantities are associated with a bounded mapping  $L_{\text{TL}}^{\mathcal{O}}(\cdot)$  taking the space-time solution  $\mathbf{u}(\mathbf{x}, t)$  and returning a time-dependent scalar function  $s(t)$ , see figure 13. Note that the functional  $L_{\text{TL}}^{\mathcal{O}}(\cdot)$  is a different mathematical object than the functional  $L^{\mathcal{O}}(\cdot)$  associated with the standard quantities of interest, because  $L^{\mathcal{O}}(\cdot)$  returns a single scalar value (and not a time-dependent function). A convenient expression for  $L_{\text{TL}}^{\mathcal{O}}(\cdot)$  is defined as an extension of the functional  $L^{\mathcal{O}}(\cdot)$  defined in (3), namely

$$[L_{\text{TL}}^{\mathcal{O}}(\mathbf{w})](t) := \int_0^t (\mathbf{f}^{\mathcal{O}}(\tau), \dot{\mathbf{w}}(\tau)) \, d\tau + \int_0^t (\mathbf{g}^{\mathcal{O}}(\tau), \dot{\mathbf{w}}(\tau))_{\Gamma_N} \, d\tau + (\rho \mathbf{v}^{\mathcal{O}}, \dot{\mathbf{w}}(t)) + a(\mathbf{u}^{\mathcal{O}}, \mathbf{w}(t)), \quad (35)$$

where the functions  $\mathbf{f}^{\mathcal{O}}$  and  $\mathbf{g}^{\mathcal{O}}$  define weighted space-time averages of the solution in the interior domain  $\Omega$  or the Neumann boundary  $\Gamma_N$ , respectively, in the time interval  $[0, t]$  for a generic time  $t \in I$ . On the other hand, functions  $\mathbf{v}^{\mathcal{O}}$  and  $\mathbf{u}^{\mathcal{O}}$  define weighted space-averages of the velocities and displacements, respectively, at a generic time point  $t \in I$ . For the sake of simplicity, the notation  $L_{\text{TL}}^{\mathcal{O}}(\mathbf{w}; t) := [L_{\text{TL}}^{\mathcal{O}}(\mathbf{w})](t)$  is

introduced.

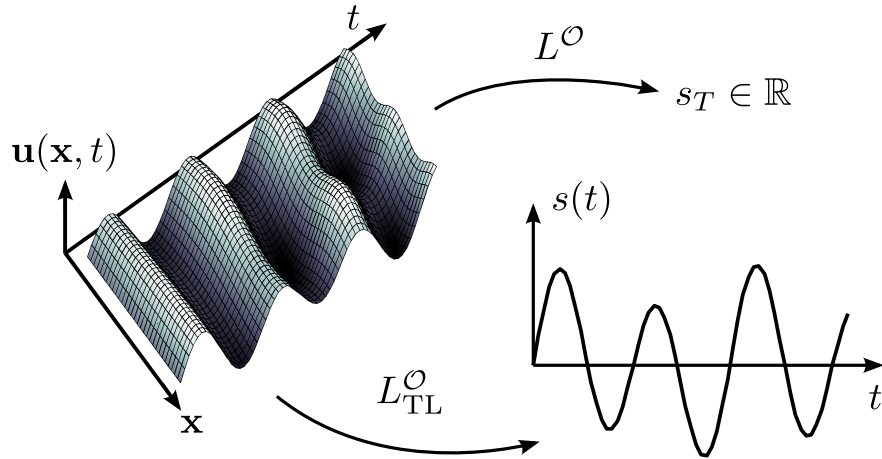


Figure 13: Illustration of scalar and timeline-dependent quantities of interest. The functional  $L^O(\cdot)$  maps the time-space solution  $\mathbf{u}(\mathbf{x}, t)$  into a scalar value  $s_T \in \mathbb{R}$ . The operator  $L^O_{TL}(\cdot)$  transforms  $\mathbf{u}(\mathbf{x}, t)$  into a time-dependent function  $s(t)$ .

The error to be assessed is the error in the timeline-dependent quantity of interest, namely

$$s^e(t) := s(t) - \tilde{s}(t),$$

where  $s(t) := L^O_{TL}(\mathbf{u}; t)$  is the exact timeline quantity and  $\tilde{s}(t) := L^O_{TL}(\tilde{\mathbf{u}}; t)$  is the computed one. Assessing the time-dependent function  $s^e(t)$  requires introducing an error representation similar to the one presented in equation (19) for the scalar quantity of interest. Thus, an auxiliary problem, analogous to the adjoint problem (4), has to be introduced for the timeline quantity  $L^O_{TL}(\cdot)$ .

Note that for a given time  $t \in I$ , the functional  $L^O_{TL}(\cdot)$  restricted to this time instance, namely  $L^O(\cdot) = L^O_{TL}(\cdot; t)$ , is a standard quantity of interest taking  $t$  as the *final* time. The associated adjoint problem is analogous to the one presented in (4) but replacing the final time  $T$  for the particular time instance  $t \in I$ . That is, the

adjoint solution  $\mathbf{u}_t^d$  associated with  $L_{\text{TL}}^{\mathcal{O}}(\cdot; t)$  is the solution of

$$\rho(\ddot{\mathbf{u}}_t^d - a_1 \dot{\mathbf{u}}_t^d) - \nabla \cdot \boldsymbol{\sigma}_t^d = -\mathbf{f}^{\mathcal{O}} \quad \text{in } \Omega \times [0, t], \quad (36a)$$

$$\mathbf{u}_t^d = \mathbf{0} \quad \text{on } \Gamma_D \times [0, t], \quad (36b)$$

$$\boldsymbol{\sigma}_t^d \cdot \mathbf{n} = -\mathbf{g}^{\mathcal{O}} \quad \text{on } \Gamma_N \times [0, t], \quad (36c)$$

$$\mathbf{u}_t^d = \mathbf{u}^{\mathcal{O}} \quad \text{at } \Omega \times \{t\}, \quad (36d)$$

$$\dot{\mathbf{u}}_t^d = \mathbf{v}^{\mathcal{O}} \quad \text{at } \Omega \times \{t\}, \quad (36e)$$

with the constitutive law

$$\boldsymbol{\sigma}_t^d := \mathbf{C} : \boldsymbol{\varepsilon}(\mathbf{u}_t^d - a_2 \dot{\mathbf{u}}_t^d). \quad (37)$$

The solution of equation (36) is denoted by  $\mathbf{u}_t^d$  emphasizing that there is a different adjoint solution for each time  $t$ . Consequently, equation (36) describes a family of adjoint problems, one for each time  $t \in I$ .

For a particular instance of time  $t \in I$ , the error representation of the value  $s^e(t)$  is similar to the standard scalar case (20) but taking the adjoint solution  $\mathbf{u}_t^d$  and restricting the residual  $R(\cdot)$  to the time interval  $[0, t]$ . That is

$$s^e(t) = R_t(\mathbf{u}_t^d), \quad (38)$$

where

$$\begin{aligned} R_t(\mathbf{w}) := & \int_0^t [l(\dot{\mathbf{w}}(\tau); \tau) - m(\ddot{\mathbf{u}}(\tau) + a_1 \dot{\mathbf{u}}(\tau), \dot{\mathbf{w}}(\tau)) - a(\tilde{\mathbf{u}}(\tau) + a_2 \dot{\mathbf{u}}(\tau), \dot{\mathbf{w}}(\tau))] \, d\tau \\ & + m(\mathbf{v}_0 - \dot{\tilde{\mathbf{u}}}(0^+), \dot{\mathbf{w}}(0^+)) + a(\mathbf{u}_0 - \tilde{\mathbf{u}}(0^+), \mathbf{w}(0^+)). \end{aligned}$$

Hence, an estimate for  $s^e(t)$  is obtained injecting an adjoint approximation  $\tilde{\mathbf{u}}_t^d \approx \mathbf{u}_t^d$  in equation (38), namely

$$s^e(t) \approx R_t(\tilde{\mathbf{u}}_t^d) =: \tilde{s}^e(t). \quad (39)$$

In practice, it is computationally unaffordable to independently compute the all the infinite approximations of the solutions  $\mathbf{u}_t^d$ , one for each time  $t \in I$ , and then using them in equation (38) to assess  $s^e(t)$ . However, for the particular case of  $\mathbf{f}^{\mathcal{O}}$  and  $\mathbf{g}^{\mathcal{O}}$  constant in time (which accounts for a number of interesting cases), the different functions  $\mathbf{u}_t^d$  corresponding to different time instances  $t \in I$  are all equivalent after a time translation. Thus, if  $\mathbf{u}_t^d$  is properly computed for a particular value of  $t$ , e.g.  $t = T$ , the general functions  $\mathbf{u}_t^d$  for  $t \neq T$  are easily recovered as a direct post-process of  $\mathbf{u}_T^d$ ,

$$\mathbf{u}_t^d(\tau) = \mathbf{u}_T^d(\tau + T - t). \quad (40)$$



This fundamental result, proved in paper C, is the crucial observation that allows assessing the error in the timeline-dependent quantity with an affordable cost.

The adjoint approximations  $\tilde{\mathbf{u}}_t^d$  used in the error estimate (39) are computed applying the time shift (40) to the adjoint approximation  $\tilde{\mathbf{u}}_T^d$  associated with the final time  $T$ ,

$$\tilde{\mathbf{u}}_t^d(\tau) := \tilde{\mathbf{u}}_T^d(\tau + T - t). \quad (41)$$

Thus, only one adjoint approximation  $\tilde{\mathbf{u}}_T^d$  has to be computed and the others are simply recovered with a time shift. The approximation  $\tilde{\mathbf{u}}_T^d$  is computed with the modal based-approach described in section 3.2. This choice is specially convenient for assessing the error in the timeline-dependent quantity of interest because it enormously simplifies the implementation of the time shift (41).

The performance of the error estimate for timeline-dependent quantities is studied in the paper C in three numerical examples. The results show that the error estimate is a good approximation of the error in the timeline-dependent quantity of interest. The quality of the error estimate depends on the number of vibration modes used to solve the auxiliary adjoint problems.

**Remark 7** (Illustrative example). *This example illustrates the performance of the error estimate  $\tilde{s}^e(t)$ . The computational domain is the three dimensional structure plotted in figure 14 which is clamped at the supports and it is loaded with the time-dependent traction*

$$\mathbf{g}(t) = \begin{cases} -g(t)\mathbf{e}_1 & \text{on } \Gamma_g, \\ \mathbf{0} & \text{elsewhere,} \end{cases}$$

where function  $g(t)$  is defined in figure 14 and the values  $g_{\max} = 1 \cdot 10^3$  Pa and  $t_g = 1 \cdot 10^{-3}$  s are considered. The set  $\Gamma_g$  is the boundary where the load is applied, see figure 14. The structure is initially at rest ( $\mathbf{u}_0 = \mathbf{v}_0 = \mathbf{0}$ ) and the body force is zero ( $\mathbf{f} = \mathbf{0}$ ). The material properties are Young's modulus  $E = 2 \cdot 10^{10}$  Pa, Poisson's ratio  $\nu = 0.2$ , density  $\rho = 2.4 \cdot 10^3$  kg/m<sup>3</sup> and viscosity  $a_1 = a_2 = 0$ . The final time is  $T = 0.02$  s .

*This example focuses in the timeline-dependent quantity of interest*

$$s(t) := \frac{1}{\text{meas}(\Gamma_g)} (\mathbf{e}_1, \mathbf{u}(t))_{\Gamma_g},$$

which is the average of the  $x$ -component of the displacement in the boundary  $\Gamma_g$  at every time  $t \in I$ .

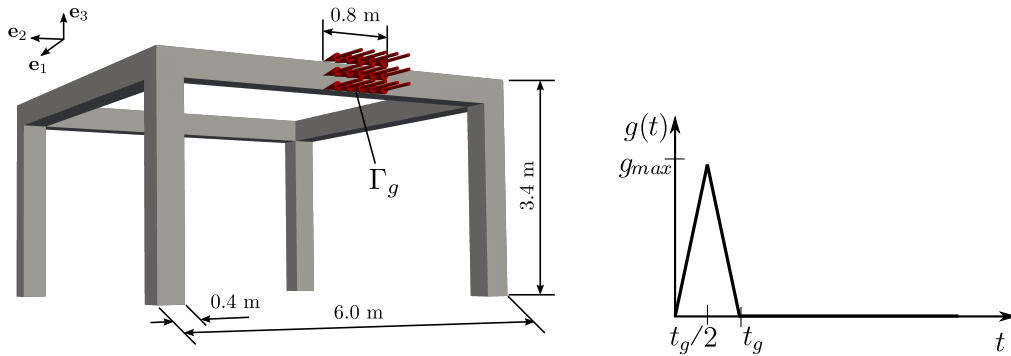


Figure 14: Problem geometry (left) and time description of the external load (right).

The problem is discretized with trilinear hexahedra in space and with the Newmark method in time with parameters  $\beta = 1/4$  and  $\gamma = 1/2$ . The approximated quantity of interest  $\tilde{s}(t) = L^O(\tilde{\mathbf{u}}; t)$  is computed from the approximate solution  $\tilde{\mathbf{u}}$  obtained with the coarse finite element mesh plotted in figure 15 and with  $N = 400$  time steps. The reference quantity of interest  $s(t) = L^O(\mathbf{u}; t)$  is obtained by assuming that the exact solution  $\mathbf{u}$  is fairly replaced by an overkill solution obtained using the reference mesh in figure 15 and  $N = 1600$  time steps. The error in the quantity of interest is evaluated using the reference solution, namely  $s^e(t) = s(t) - \tilde{s}(t)$ . Finally, the error estimate  $\tilde{s}^e(t)$  is computed using up to  $M = 60$  vibration modes for approximating the adjoints.

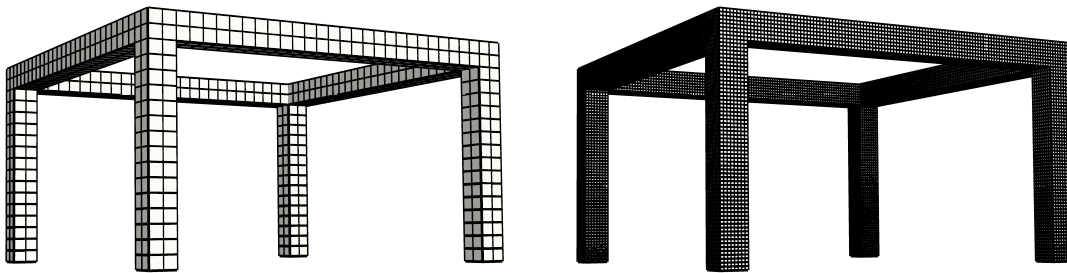


Figure 15: Coarse (left) and reference (right) meshes used in this example with 334 and 22016 elements respectively.

Figure 16 shows the computed and reference timeline-dependent quantities,  $\tilde{s}(t)$  and  $s(t)$ , along with the assessed and reference errors,  $\tilde{s}^e(t)$  and  $s^e(t)$ . Note that the quality of the error estimate  $\tilde{s}^e(t)$  increases with the number of vibration modes. For

$M = 60$  modes, the error estimate  $\tilde{s}^e(t)$  and the reference error  $s^e(t)$  are in very good agreement.

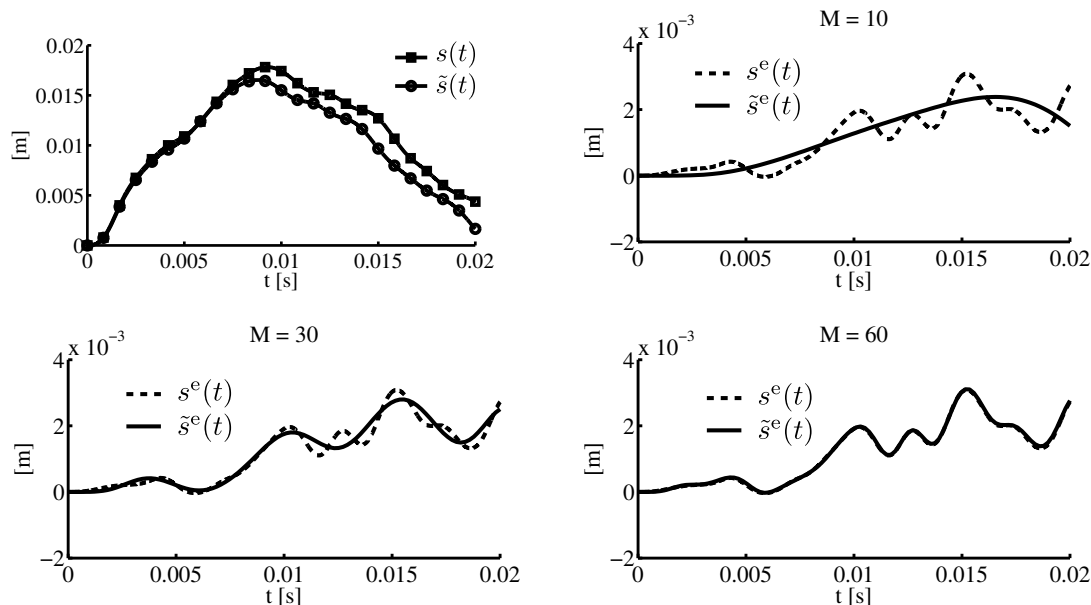


Figure 16: Approximated quantity of interest  $\tilde{s}(t)$  and reference quantity  $s(t)$  (top, left). Reference and assessed errors,  $s^e(t)$  and  $\tilde{s}^e(t)$ , for three different number of vibration modes for approximating the adjoints,  $M = 10$  (top, right),  $M = 30$  (bottom, left) and  $M = 60$  (bottom, right).

## 4 Closure

The main contributions as well as the open research lines are summarized in the following subsections.

### 4.1 Summary

- **Goal-oriented error bounds**

A new technique providing goal-oriented error bounds in the framework of linear structural dynamics is proposed. The novel error bounds are derived in two alternative and equivalent ways: 1) using symmetrized error equations, or 2) using an auxiliary error associated with the viscous stress. The first approach resembles to the one considered for the convection-diffusion-reaction equation,

and the second approach is analogous to the one considered in steady-state linear elasticity.

The proposed methodology yields estimates with better quality than the already available approaches. The bound gap of the novel approach is 50% sharper. The techniques providing error bounds in structural dynamics (including the proposed methodology) require that the formulation has a finite amount of damping. Consequently, the computed bounds are very pessimistic for materials with a small amount of damping. Further research is needed to explore alternative pertinent bounds for nearly elastic problems.

- **Modal-based goal-oriented error assessment**

An efficient goal-oriented error estimate for structural transient dynamics is introduced. The proposed methodology uses modal analysis to compute the adjoint problem instead of direct time-integration methods. The modal-based approach is particularly well suited for some quantities of interest and allows effectively computing and storing the adjoint solution. This is because the adjoint solution is computed and stored for each vibration mode instead of for each time step. Moreover, the modal-based description of the adjoint approximation facilitates the post-processing techniques applied to enhance its space accuracy. The post-processing is performed just once for every relevant mode instead of for each time step. The resulting estimate is well-suited for time-dependent problems because its cost per time step is very low.

The quality of the modal-based error estimate depends on the number of computed vibration modes. The required number of modes strongly depends on the definition of the quantity of interest. Consequently, some practical quantities of interest are proposed requiring only few low-frequency modes.

- **Modal-based goal-oriented adaptivity**

The proposed adaptive strategy aims at computing an optimal space-time discretization such that the computed solution has an error in the quantity of interest below a user-defined tolerance. The major novelty with respect previous approaches is that the local error information driving the adaptive process is computed using the modal-based error estimate.

The numerical examples show that the proposed technique provides adapted solutions fulfilling the user-defined error tolerance. That is, both the assessed and computed errors are below the user-defined error value. Moreover, the discretizations obtained with the proposed adaptive strategy are more efficient than the ones obtained with an uniform refinement of all mesh elements and time steps. The adapted discretizations give more accurate results than the non-adapted ones for the same number of space-time elements.

- **Error assessment for timeline-dependent quantities of interest**

A new type of quantities of interest for error assessment in structural transient dynamics are proposed. These quantities (referred as timeline-dependent quantities of interest) are scalar time-dependent outputs of the transient solution which are better suited to time-dependent problems than the standard scalar ones.

Assessing the error in the timeline-dependent quantity of interest requires approximating infinite standard adjoint problems. However, all these problems are similar and they can be recovered from a common parent problem (associated with the a scalar quantity of interest) by means of a simple translation of the time variable. The time shift is very efficiently performed if the adjoint solution is approximated with modal analysis, because the time-dependence of the adjoint solution is known analytically.

The proposed error estimate for timeline-dependent quantities provides accurate approximations of the error accounting for both the space and time discretization errors. The quality of the error estimate depends on the number of vibration modes used to solve the auxiliary adjoint problems.

## 4.2 Open research lines

- **Space-time adaptivity for explicit time-integration schemes**

The proposed space-time adaptive technique restricts to numerical approximations obtained with space-time variational methods as the time-continuous Galerkin method. Therefore, the adaptive technique cannot directly deal with explicit time-integration schemes using a lumped mass matrix. Explicit schemes are widely used in structural dynamics and, therefore, the corresponding extension of the proposed space-time adaptive technique is worth considering. The

main challenge of this extension is how to separate the effect of lumping the mass matrix from other error sources.

Note that another difficulty arises in mesh adaptivity for explicit schemes. At first sight, the use of local mesh refinement in combination of explicit schemes compromises the efficiency of the overall approximation process. Note that, in this context, the time step length is determined by the smallest element size of the computational mesh. Consequently, a local mesh refinement leads to a fine time step length for the whole computational mesh. However, this difficulty can be overcome using time-step partitioning techniques allowing to use different time step lengths in different regions of the domain, see for instance the work by Casadei and Halleux (2009).

- **Modal-based bounds of the error in the quantity of interest**

The modal-based description of the adjoint solution is used here to compute approximations of the error in the quantity of interest. However, modal analysis can be considered also to compute goal-oriented error bounds. One of the main advantages of this approach is that the modal description of the adjoint solution allows to efficiently compute the adjoint D-admissible stress. This is because the standard stress equilibration techniques are performed for few relevant modes instead for each time step. Thus, the cost per time step is reduced.

- **Error bounds in the timeline-quantity of interest.**

Bounds of the error in the timeline-dependent quantity are still to be explored. The error bounds presented in section 3.1 for standard quantities of interest can be extended to deal with timeline-dependent quantities. Using the time translation presented in section 3.4 and using a modal description for the adjoint solution, the error bounds in the timeline-dependent quantity of interest can be efficiently computed.

- **Enhanced goal-oriented error bounds**

The quality of the proposed error bounds is not enough for practical applications, specially in nearly elastic cases. The proposed bounds can be enhanced following the rationale presented by Parés (2005) designed for a generic non-symmetric problem. This approach introduces a continuous approximation of the error which is used to compute a correction factor improving the quality

of the error bounds. Consequently, the particularization of this methodology might lead to improved error bounds in structural transient dynamics.

- **Modal-based error assessment and timeline-dependent quantities of interest for other linear time-dependent problems**

The proposed modal-based error estimate as well as the proposed timeline-dependent quantities of interest can be extended to deal with other linear time-dependent problems (e.g. parabolic problems as the time-dependent heat equation).

- **Approximating the adjoint solution with other reduced order models**

Modal analysis can be seen as a particular reduced order model. More sophisticated reduced order models can be considered to describe the solution of the adjoint problem, for instance, the Proper Generalized Decomposition (PGD) introduced by Chinesta et al. (2011). This approach can be applied to deal with quantities of interest with a parametric definition. That is, the quantity of interest is defined introducing some free parameters, e.g., the position or shape of the zone of interest, the time instant of interest, etc. Note that the proposed timeline-dependent quantity of interest is indeed a particular parametric quantity where the selected parameter is the time instant of interest.

The PGD approach might allow to efficiently pre-compute and store the solution of the adjoint problem associated with the parametric quantity of interest for any value of the selected parameters in a given domain.





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Paper A

# Error assessment in structural transient dynamics

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# Error assessment in structural transient dynamics \*

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## Abstract

This paper presents in a unified framework the most representative state-of-the-art techniques on a posteriori error assessment for second order hyperbolic problems, i.e., structural transient dynamics. For the sake of presentation, the error estimates are grouped in four types: recovery-based estimates, the dual weighted residual method, the constitutive relation error method and error estimates for timeline-dependent quantities of interest. All these methodologies give a comprehensive overview on the available error assessment techniques in structural dynamics, both for energy-like and goal-oriented estimates.

**Keywords:** elastodynamics, transient dynamics, goal-oriented error assessment, adjoint problem, quantity of interest, recovery estimates, dual weighted residuals, constitutive relation error, timeline-dependent quantity of interest.

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## 1 Introduction

Discretization errors are intrinsic to any Finite Element (FE) solution. Consequently, the tools assessing and controlling the error or, conversely, the accuracy of the numerical approximation have deserved the attention of the FE community. These tools are especially

important if sensitive decisions are taken on the basis of the numerical results. Many *a posteriori* error estimators have been developed with application to different problem types. The application of these techniques to second-order hyperbolic problems (e.g. structural dynamics or elastodynamics) is particularly relevant because, as compared to the standard elliptic problems, the discretization errors are generated and propagated less intuitively or predictively.

The pioneering works on FE error assessment date back to the late 70's and provide estimates of the energy norm of the error in steady-state (elliptic) problems (e.g. linear elasticity or thermal problems), see [1, 2, 3]. Goal-oriented estimates aim at assessing the error of functional outputs of the solution, that is at measuring the error in some *Quantity of Interest* (QoI). They were introduced much later [4, 5, 6, 7]. In the context of elliptic problems, error estimates are currently pretty well established, both for energy (also denoted as global) and goal-oriented (often referred as local, because the QoI are localized in a particular zone), see [8, 9, 10, 11, 12, 13, 14] as state-of-the-art reviews and books.

The techniques developed for elliptic problems have been extended to other problem types. For instance, quasi-steady-state non-linear problems are addressed in references [15, 16, 17, 18], estimates for advection-diffusion-reaction equation are discussed in [19], similar approaches for the Stokes problem are presented in [20], and extension to parabolic time-dependent problems is introduced in [21, 22, 23]. Moreover, the same type of tools for coupled problems have been recently discussed in [24, 25, 26, 27, 28].

In the present paper, attention is devoted to the techniques allowing to assess the error in structural transient dynamics. In this context, different estimates provide also indicators driving mesh adaptive procedures, either using energy-like measures [29, 30, 31, 32, 33, 34] or QoI [35, 36, 37, 38]. Estimates providing error bounds are also available both for energy-like error measures [39, 40, 41] and goal-oriented ones [42, 43, 44, 45, 46, 47, 31].

The strategies assessing the error in structural transient dynamics are based on extending the standard estimates designed for steady state linear elasticity. This is still an open problem. The difficulties arise when freezing the time dependence into a series of static problems, in particular in the treatment of the inertia terms.

This review paper aims at presenting the state-of-the-art techniques on a posteriori error assessment for second order hyperbolic problems. Four types of error estimates are analyzed, offering an comprehensive overview: 1) recovery-based estimates, 2) dual weighted residual method, 3) constitutive relation error and 4) error assessment for timeline-dependent quantities of interest. The main rationale of each technique is presented following the most representative references. Note that the methodologies presented here are introduced using diverse notations by different authors. Here, the different estimates are described within a unified framework for the sake of an easier reading.

The remainder of the text is organized as follows. Section 2 presents the equations of structural dynamics (both strong and weak versions) and their corresponding approximations. Section 3 introduces the error to be assessed, the error equations and the error representations needed for successive sections. Section 4 is devoted to the recovery-based estimates while section 5 deals with the dual weighted residual method and other ex-

PLICIT residual estimates. Section 6 presents the constitutive relation error method and the corresponding error bounds. Finally, section 7 presents an error estimate for special quantities of interest called timeline-dependent quantities of interest. The article is closed with some concluding remarks.

## 2 Problem statement

### 2.1 Strong equations

A visco-elastic body occupies an open bounded domain  $\Omega \subset \mathbb{R}^d$ ,  $d \leq 3$ , with boundary  $\partial\Omega$ . The boundary is divided in two disjoint parts,  $\Gamma_N$  and  $\Gamma_D$  such that  $\partial\Omega = \overline{\Gamma_N} \cup \overline{\Gamma_D}$  and the considered time interval is  $I := (0, T]$ . Under the assumption of small perturbations, the evolution of displacements  $\mathbf{u}(\mathbf{x}, t)$  and stresses  $\boldsymbol{\sigma}(\mathbf{x}, t)$ ,  $\mathbf{x} \in \Omega$  and  $t \in I$ , is described by the visco-elastodynamic equations,

$$\rho(\ddot{\mathbf{u}} + a_1 \dot{\mathbf{u}}) - \nabla \cdot \boldsymbol{\sigma} = \mathbf{f} \quad \text{in } \Omega \times I, \quad (1a)$$

$$\mathbf{u} = \mathbf{0} \quad \text{on } \Gamma_D \times I, \quad (1b)$$

$$\boldsymbol{\sigma} \cdot \mathbf{n} = \mathbf{g} \quad \text{on } \Gamma_N \times I, \quad (1c)$$

$$\mathbf{u} = \mathbf{u}_0 \quad \text{at } \Omega \times \{0\}, \quad (1d)$$

$$\dot{\mathbf{u}} = \mathbf{v}_0 \quad \text{at } \Omega \times \{0\}, \quad (1e)$$

where an upper dot indicates derivation with respect to time, that is  $(\dot{\bullet}) := \frac{d}{dt}(\bullet)$ , and  $\mathbf{n}$  denotes the outward unit normal to  $\partial\Omega$ . The input data includes the mass density  $\rho = \rho(\mathbf{x}) > 0$ , the first Rayleigh coefficient  $a_1 \geq 0$ , the body force  $\mathbf{f} = \mathbf{f}(\mathbf{x}, t)$  and the traction  $\mathbf{g} = \mathbf{g}(\mathbf{x}, t)$  acting on the Neumann boundary  $\Gamma_N \times I$ . The initial conditions for displacements and velocities are  $\mathbf{u}_0 = \mathbf{u}_0(\mathbf{x})$  and  $\mathbf{v}_0 = \mathbf{v}_0(\mathbf{x})$  respectively. For the sake of simplicity and without any loss of generality, Dirichlet conditions (1b) are taken as homogeneous.

The set of equations (1) is closed with the constitutive law,

$$\boldsymbol{\sigma} := \mathbf{C} : \boldsymbol{\varepsilon}(\mathbf{u} + a_2 \dot{\mathbf{u}}), \quad (2)$$

where the parameter  $a_2 \geq 0$  is the second Rayleigh coefficient, the tensor  $\mathbf{C}$  is the standard 4th-order elastic Hooke tensor. The strains are given by the kinematic relation corresponding to small perturbations, that is  $\boldsymbol{\varepsilon}(\mathbf{w}) := \frac{1}{2}(\nabla \mathbf{w} + \nabla^T \mathbf{w})$ .

### 2.2 Weak and discrete formulations

#### 2.2.1 Newmark-like methods

The definition of the weak form of the problem requires introducing the following functional spaces: the standard Sobolev space associated with static displacement fields

$$\mathcal{V}_0 := \{ \mathbf{w} \in [H^1(\Omega)]^d : \mathbf{w} = \mathbf{0} \text{ on } \Gamma_D \},$$

equipped with the usual functional norm which is denoted by  $\|\cdot\|_{\mathbf{V}_0}$ . The Bochner space  $L^2(I; \mathbf{V}_0)$  associated with  $\mathbf{V}_0$  of square-integrable functions from  $I$  into  $\mathbf{V}_0$  is also introduced

$$L^2(I; \mathbf{V}_0) := \left\{ \mathbf{v} : I \rightarrow \mathbf{V}_0, \text{ such that } \int_0^T \|\mathbf{v}(t)\|_{\mathbf{V}_0}^2 dt < +\infty \right\}.$$

The solution of the problem,  $\mathbf{u}(\mathbf{x}, t)$ , belongs to the space  $\mathcal{W}$  defined as

$$\mathcal{W} := \left\{ \mathbf{w} \in L^2(I; \mathbf{V}_0) \text{ with } \dot{\mathbf{w}} \in L^2(I; [L^2(\Omega)]^d) \text{ and } \ddot{\mathbf{w}} \in L^2(I; \mathbf{V}'_0) \right\},$$

where  $\mathbf{V}'_0$  denotes the dual space of  $\mathbf{V}_0$ . Note that in particular this implies that any  $\mathbf{w} \in \mathcal{W}$  is such that  $\mathbf{w} \in C^0(\bar{I}; [L^2(\Omega)]^d)$  and  $\dot{\mathbf{w}} \in C^0(\bar{I}; \mathbf{V}'_0)$ , see [48]. That is, functions in  $\mathcal{W}$  and their time derivatives are continuous in time.

**Remark 1.** *Function  $\mathbf{u}$  is a transformation between  $\Omega \times I$  and  $\mathbb{R}^d$ , i.e.*

$$\begin{aligned} \mathbf{u} : \Omega \times I &\longrightarrow \mathbb{R}^d \\ (\mathbf{x}, t) &\longmapsto \mathbf{u}(\mathbf{x}, t). \end{aligned}$$

*It can also be seen as a transformation between  $I$  and  $\mathbf{V}_0$ , i.e.*

$$\begin{aligned} \mathbf{u} : I &\longrightarrow \mathbf{V}_0 \\ t &\longmapsto \mathbf{u}(t). \end{aligned}$$

*In the remainder of the paper, both notations are used, for  $\mathbf{u}$  and other functions, to denote the same mathematical objects depending on the context.*

Thus, the weak form (integrated in space) of problem (1) reads: find  $\mathbf{u} \in \mathcal{W}$  verifying the initial conditions  $\mathbf{u}(0) = \mathbf{u}_0$  and  $\dot{\mathbf{u}}(0) = \mathbf{v}_0$  and such that for all  $t \in I$

$$m(\ddot{\mathbf{u}}(t) + a_1 \dot{\mathbf{u}}(t), \mathbf{w}) + a(\mathbf{u}(t) + a_2 \dot{\mathbf{u}}(t), \mathbf{w}) = l(t; \mathbf{w}) \quad \forall \mathbf{w} \in \mathbf{V}_0, \quad (3)$$

where the standard linear and bilinear forms are introduced

$$\begin{aligned} a(\mathbf{v}, \mathbf{w}) &:= \int_{\Omega} \boldsymbol{\varepsilon}(\mathbf{v}) : \boldsymbol{\mathcal{C}} : \boldsymbol{\varepsilon}(\mathbf{w}) \, d\Omega \quad , \quad m(\mathbf{v}, \mathbf{w}) := \int_{\Omega} \rho \mathbf{v} \cdot \mathbf{w} \, d\Omega, \\ l(t; \mathbf{w}) &:= (\mathbf{f}(t), \mathbf{w}) + (\mathbf{g}(t), \mathbf{w})_{\Gamma_N}, \end{aligned}$$

along with the scalar products

$$(\mathbf{v}, \mathbf{w}) := \int_{\Omega} \mathbf{v} \cdot \mathbf{w} \, d\Omega \quad \text{and} \quad (\mathbf{v}, \mathbf{w})_{\Gamma_N} := \int_{\Gamma_N} \mathbf{v} \cdot \mathbf{w} \, d\Gamma.$$

A mesh of characteristic element size  $H$  discretizing the spatial domain  $\Omega$  is introduced together with its associated finite element space  $\mathbf{V}_0^H \subset \mathbf{V}_0$ . The degree of the complete polynomial basis in  $\mathbf{V}_0^H$  is denoted by  $p$ . This allows introducing the spatially-discrete and time-continuous version of equation (3) (semidiscrete problem), namely: find  $\mathbf{u}^H(t) \in \mathbf{V}_0^H$  such that for all  $t \in I$

$$m(\ddot{\mathbf{u}}^H(t) + a_1 \dot{\mathbf{u}}^H(t), \mathbf{w}) + a(\mathbf{u}^H(t) + a_2 \dot{\mathbf{u}}^H(t), \mathbf{w}) = l(t; \mathbf{w}) \quad \forall \mathbf{w} \in \mathbf{V}_0^H, \quad (4)$$

with initial conditions (1d) and (1e). In the case  $\mathbf{u}_0$  and  $\mathbf{v}_0$  are not in  $\mathcal{V}_0^H$ , (1d) and (1e) have to be replaced by  $\mathbf{u}^H(0) = \mathbf{\Pi}^H(\mathbf{u}_0)$  and  $\dot{\mathbf{u}}^H(0) = \mathbf{\Pi}^H(\mathbf{v}_0)$ , being  $\mathbf{\Pi}^H$  the interpolation operator mapping functions from the continuous space  $\mathcal{V}_0$  into the discrete space  $\mathcal{V}_0^H$ .

The Newmark method is a numerical time-marching scheme providing an approximation of the standard system of second order ODEs (4) arising in structural dynamics. A time-grid discretizing the time interval  $I$  is introduced,  $\mathcal{T} := \{t_0, t_1, \dots, t_N\}$ , where  $0 = t_0 < t_1 < \dots < t_N = T$ . The time points in  $\mathcal{T}$  define the time intervals  $I_n := (t_{n-1}, t_n]$ ,  $n = 1, \dots, N$ . The length of the time interval  $I_n$  is denoted by  $\Delta t_n := t_n - t_{n-1}$ , for  $n = 1, \dots, N$  and the characteristic time step for the time grid is

$$\Delta t := \max_{1 \leq n \leq N} (\Delta t_n).$$

The Newmark solution consists in displacements, velocities and accelerations at each time  $t_n$ ,  $\mathbf{u}_n^{H,\Delta t} \approx \mathbf{u}^H(t_n)$ ,  $\mathbf{v}_n^{H,\Delta t} \approx \dot{\mathbf{u}}^H(t_n)$  and  $\mathbf{a}_n^{H,\Delta t} \approx \ddot{\mathbf{u}}^H(t_n)$ , respectively, for  $n = 1, \dots, N$ , such that equation (4) is fulfilled at each time  $t_n \in \mathcal{T}$ , that is

$$m(\mathbf{a}_n^{H,\Delta t} + a_1 \mathbf{v}_n^{H,\Delta t}, \mathbf{w}) + a(\mathbf{u}_n^{H,\Delta t} + a_2 \mathbf{v}_n^{H,\Delta t}, \mathbf{w}) = l_n(\mathbf{w}) \quad \forall \mathbf{w} \in \mathcal{V}_0^H. \quad (5)$$

where  $l_n(\mathbf{w}) := l(t_n; \mathbf{w})$ .

At each time interval, it is assumed that  $\mathbf{u}_{n-1}^{H,\Delta t}, \mathbf{v}_{n-1}^{H,\Delta t}, \mathbf{a}_{n-1}^{H,\Delta t}$  are known and that the following discrete integral expressions hold

$$\begin{aligned} \mathbf{u}_n^{H,\Delta t} &= \mathbf{u}_{n-1}^{H,\Delta t} + \Delta t_n \mathbf{v}_{n-1}^{H,\Delta t} + \frac{1}{2} \Delta t_n^2 \left[ (1 - 2\beta) \mathbf{a}_{n-1}^{H,\Delta t} + 2\beta \mathbf{a}_n^{H,\Delta t} \right], \\ \mathbf{v}_n^{H,\Delta t} &= \mathbf{v}_{n-1}^{H,\Delta t} + \Delta t_n \left[ (1 - \gamma) \mathbf{a}_{n-1}^{H,\Delta t} + \gamma \mathbf{a}_n^{H,\Delta t} \right]. \end{aligned}$$

Thus, the only remaining unknown in equation (5) is  $\mathbf{a}_n^{H,\Delta t}$ , which is obtained solving a linear system of algebraic equations. Similarly, at time  $t_0$ , the displacements and velocities are determined by the initial conditions and the acceleration  $\mathbf{a}_0^{H,\Delta t}$  is computed by considering that

$$m(\mathbf{a}_0^{H,\Delta t} + a_1 \mathbf{\Pi}^H(\mathbf{v}_0), \mathbf{w}) + a(\mathbf{\Pi}^H(\mathbf{u}_0) + a_2 \mathbf{\Pi}^H(\mathbf{v}_0), \mathbf{w}) = l_0(\mathbf{w}) \quad \forall \mathbf{w} \in \mathcal{V}_0^H.$$

The scalars  $\beta$  and  $\gamma$  are the parameters of the Newmark method taking values in  $[0, 1]$ . For  $\gamma = 1/2$  the method is second order accurate and there is no numerical damping, whereas for  $\gamma > 1/2$  numerical damping is introduced. Moreover, the method is conditionally stable for  $\beta \geq \gamma/2 \geq 1/4$ . See [49] for specific details.

In the framework of using finite difference based time marching schemes, it is quite common that error estimation strategies require obtaining a numerical approximation of problem (1) more regular than the direct numerical solution, with stronger continuity requirements. This post-processed version of the numerical solution is denoted hereafter as  $\tilde{\mathbf{u}}$ , see for instance section 6. Note that the Newmark method does not directly provide a numerical approximation  $\tilde{\mathbf{u}} \in \mathcal{W}$ , since it is not even defined in the whole time interval  $I$  (it is only given at times  $t_n$  of the time grid).

The first step in order to recover an smooth numerical approximation is to extend the Newmark approximation into the whole time domain using a simple piecewise linear interpolation:

$$\mathbf{u}^{H,\Delta t}(\mathbf{x}, t) := \sum_{n=0}^N \mathbf{u}_n^{H,\Delta t}(\mathbf{x}) \theta_n(t), \quad (6a)$$

$$\mathbf{v}^{H,\Delta t}(\mathbf{x}, t) := \sum_{n=0}^N \mathbf{v}_n^{H,\Delta t}(\mathbf{x}) \theta_n(t), \quad (6b)$$

$$\mathbf{a}^{H,\Delta t}(\mathbf{x}, t) := \sum_{n=0}^N \mathbf{a}_n^{H,\Delta t}(\mathbf{x}) \theta_n(t), \quad (6c)$$

where the functions  $\theta_n(t)$ , for  $n = 0, \dots, N$ , are the one-dimensional piecewise linear shape functions related with the time partition  $\mathcal{T}$ . Note that, however, one cannot take  $\tilde{\mathbf{u}} = \mathbf{u}^{H,\Delta t}(\mathbf{x}, t)$  since this approximation does not meet the regularity requirements of the functional space  $\mathcal{W}$ ,  $\mathbf{u}^{H,\Delta t}(\mathbf{x}, t) \notin \mathcal{W}$ , because its time derivative is not continuous. Following [9], an admissible approximation  $\tilde{\mathbf{u}} \in \mathcal{W}$  is easily recovered from the Newmark solution using the information provided by the numerical accelerations, namely

$$\dot{\tilde{\mathbf{u}}}(\mathbf{x}, t) := \int_0^t \mathbf{a}^{H,\Delta t}(\mathbf{x}, \tau) d\tau + \mathbf{v}_0(\mathbf{x}), \quad (7a)$$

$$\tilde{\mathbf{u}}(\mathbf{x}, t) := \int_0^t \dot{\tilde{\mathbf{u}}}(\mathbf{x}, \tau) d\tau + \mathbf{u}_0(\mathbf{x}). \quad (7b)$$

Note that the recovered function  $\tilde{\mathbf{u}}$  belongs to the following discrete space

$$\mathcal{W}^{H,\Delta t} := \{ \mathbf{w} \in C^1(\bar{I}; \mathcal{V}_0^H) \text{ with } \mathbf{w}|_{I_n} \in \mathbb{P}^q(I_n; \mathcal{V}_0^H), n = 1, \dots, N \},$$

where  $\mathbb{P}^q(\cdot)$  represent the space of polynomials of order  $q$  in time (where for the particular expression (7b), the polynomial order is  $q = 3$ ). Note that by construction the approximation  $\tilde{\mathbf{u}}$  exactly verifies the initial conditions and that the admissible acceleration coincides with the Newmark solution,  $\ddot{\tilde{\mathbf{u}}} = \mathbf{a}^{H,\Delta t}$ . Note that the displacements  $\mathbf{u}^{H,\Delta t}$  and  $\tilde{\mathbf{u}}$  do not coincide but they converge to the same function as  $\Delta t$  tends to zero.

### 2.2.2 Space-time variational formulations

The main objective of a posteriori error estimation techniques is to evaluate the error in some specific scalar measure (energy-type norms or quantities of interest). The error is related with the non-verification of the equation to be solved, that is with the residual. In order to properly define the residual associated with some numerical approximation, a space-time variational form of the problem is required. Note that the variational format is employed to derive the error estimate, not necessarily to solve the problem. For instance, the Newmark method is not using any time variational form. However, there are some a posteriori error estimation techniques using the full variational formulation both for the problem approximation and for the error assessment strategy, see [35, 50, 29, 30].

Among the possible space-time variational formulations available for transient elastodynamics, four options are considered in the remainder of the paper. They correspond to the choices made by the authors that designed the error estimation strategies presented here. The first option is a Single Field (SF) Galerkin method based on the approach introduced by Hughes and Hulbert [51, 52]. The other three options follow a Double Field (DF) formulation. The second option, based on [35], is a time-Continuous Galerkin approach using the *mass* product  $m(\cdot, \cdot)$  to enforce the displacement-velocity consistency, and it will be referred as CGM. The third option, denoted by CGA, very similar to the previous one, differs in the fact that displacement-velocity consistency is enforced using the bilinear form  $a(\cdot, \cdot)$ , see [50]. The fourth case is a double field discontinuous Galerkin method introduced by Johnson [53] and it is denoted by DG.

The SF approach uses the following weak form of problem (1) (the SF reference is omitted in the notation): find  $\mathbf{u} \in \mathcal{W}$  such that

$$B(\mathbf{u}, \mathbf{w}) = L(\mathbf{w}) \quad \forall \mathbf{w} \in \mathcal{W}, \quad (8)$$

where

$$B(\mathbf{v}, \mathbf{w}) := \int_I m(\ddot{\mathbf{v}} + a_1 \dot{\mathbf{v}}, \dot{\mathbf{w}}) dt + \int_I a(\mathbf{v} + a_2 \dot{\mathbf{v}}, \dot{\mathbf{w}}) dt + m(\dot{\mathbf{v}}(0^+), \dot{\mathbf{w}}(0^+)) + a(\mathbf{v}(0^+), \mathbf{w}(0^+)),$$

and

$$L(\mathbf{w}) := \int_I l(t; \dot{\mathbf{w}}(t)) dt + m(\mathbf{v}_0, \dot{\mathbf{w}}(0^+)) + a(\mathbf{u}_0, \mathbf{w}(0^+)).$$

The value  $B(\mathbf{u}, \mathbf{u})$  is the total energy associated with the displacement  $\mathbf{u}$ , which plays a role in obtaining error bounds, see section 6. This formulation is used to derive error estimation strategies but does not provide a practical methodology to compute the numerical approximation of the problem. This is because the weak equation (8) leads to a fully coupled space-time problem with a prohibitive computational cost.

An usual alternative in transient elastodynamics is using Double field (or mixed) formulations, which introduce the velocity  $\dot{\mathbf{u}}$  as new unknown. Thus, the unknown is the double field function  $\mathbf{U} := [\mathbf{u}_u, \mathbf{u}_v] := [\mathbf{u}, \dot{\mathbf{u}}]$ . The main advantage of mixed formulations is that they allow alleviating the continuity requirements on the solution. Instead of  $\mathbf{u} \in \mathcal{W}$ , the solution is  $\mathbf{U} \in \mathcal{W}_0 \times \mathcal{W}_0$  where

$$\mathcal{W}_0 := \{\mathbf{w} \in L^2(I; \mathcal{V}_0) \text{ with } \dot{\mathbf{w}} \in L^2(I; \mathcal{V}'_0)\}.$$

Note that, in particular, this implies that  $\mathbf{u}_u$  and  $\mathbf{u}_v \in C^0(\bar{I}; [L^2(\Omega)]^d)$  but their derivatives are not necessarily continuous.

The trial space for the double field time-continuous and time-discontinuous Galerkin formulations is defined as

$$\widehat{\mathcal{W}} := \{\mathbf{w} \in L^2(I; \mathcal{V}_0) \text{ with } \mathbf{w}|_{I_n} \in H^1(I_n; \mathcal{V}_0), n = 1, \dots, N\}.$$

Note that functions in  $\widehat{\mathcal{W}}$  may be time-discontinuous. This property is necessary to decouple the solution in successive time intervals  $I_n$ .



With these notations, the double field time continuous Galerkin weak form of problem (1) presented in [35] reads: find  $\mathbf{U} \in \mathcal{W}_0 \times \mathcal{W}_0$  such that for all

$$B_{\text{CGM}}(\mathbf{U}, \mathbf{W}) = L_{\text{CGM}}(\mathbf{W}) \quad \forall \mathbf{W} \in \widehat{\mathcal{W}} \times \widehat{\mathcal{W}}, \quad (9)$$

where  $B_{\text{CGM}}(\cdot, \cdot)$  and  $L_{\text{CGM}}(\cdot)$  are defined as

$$\begin{aligned} B_{\text{CGM}}(\mathbf{U}, \mathbf{W}) := & \int_I m(\dot{\mathbf{u}}_v + a_1 \mathbf{u}_v, \mathbf{w}_v) dt + \int_I a(\mathbf{u}_u + a_2 \mathbf{u}_v, \mathbf{w}_v) dt + m(\mathbf{u}_v(0^+), \mathbf{w}_v(0^+)) \\ & + \int_I m(\dot{\mathbf{u}}_u - \mathbf{u}_v, \mathbf{w}_u) dt + m(\mathbf{u}_u(0^+), \mathbf{w}_u(0^+)), \end{aligned} \quad (10a)$$

$$L_{\text{CGM}}(\mathbf{W}) := \int_I l(t; \mathbf{w}_v) dt + m(\mathbf{u}_0, \mathbf{w}_u(0^+)) + m(\mathbf{v}_0, \mathbf{w}_v(0^+)), \quad (10b)$$

and the general notation  $\mathbf{W} := [\mathbf{w}_u, \mathbf{w}_v]$  is used. Note that the constrain  $\dot{\mathbf{v}}_u = \mathbf{v}_v$  is weakly enforced using the mass product defined by  $m(\cdot, \cdot)$  and this is the reason of using  $\mathbf{M}$  in the notation for this approach.

A fully discrete solution is obtained replacing the infinite dimensional spaces involved in the weak form (9) by the discrete spaces

$$\mathcal{W}_0^{H, \Delta t} := \{\mathbf{v} \in C^0(\bar{I}; \mathcal{V}_0^H) \text{ with } \mathbf{v}|_{I_n} \in \mathbb{P}^q(I_n; \mathcal{V}_0^H), n = 1, \dots, N\}, \quad (11a)$$

$$\widehat{\mathcal{W}}^{H, \Delta t} := \{\mathbf{v} \in L^2(I; \mathcal{V}_0^H) \text{ with } \mathbf{v}|_{I_n} \in \mathbb{P}^{q-1}(I_n; \mathcal{V}_0^H), n = 1, \dots, N\}. \quad (11b)$$

Functions in  $\mathcal{W}_0^{H, \Delta t}$  are continuous, piecewise polynomial both in space and time, whereas functions in  $\widehat{\mathcal{W}}^{H, \Delta t}$  are also piecewise polynomials in space and time, continuous in space but not necessarily continuous in time. It is worth noting that the polynomials for the time discretization in  $\mathcal{W}_0^{H, \Delta t}$  are one degree higher than the ones in  $\widehat{\mathcal{W}}^{H, \Delta t}$ . However, properly accounting for the initial conditions, the dimensions of  $\mathcal{W}_0^{H, \Delta t}$  and  $\widehat{\mathcal{W}}^{H, \Delta t}$  coincide due to the continuity requirements of  $\mathcal{W}_0^{H, \Delta t}$ .

The fully discrete equation reads: find  $\widetilde{\mathbf{U}} := [\widetilde{\mathbf{u}}_u, \widetilde{\mathbf{u}}_v] \in \mathcal{W}_0^{H, \Delta t} \times \mathcal{W}_0^{H, \Delta t}$  such that

$$B_{\text{CGM}}(\widetilde{\mathbf{U}}, \mathbf{W}) = L_{\text{CGM}}(\mathbf{W}) \quad \forall \mathbf{W} \in \widehat{\mathcal{W}}^{H, \Delta t} \times \widehat{\mathcal{W}}^{H, \Delta t}. \quad (12)$$

As mentioned before, although problem (12) is integrated over the whole space-time domain  $\Omega \times I$ , the discontinuities of the test functions allow decoupling the problem into  $N$  problems posed over the time slabs  $\Omega \times I_n$ ,  $n = 1, \dots, N$ . To be more precise,  $\widetilde{\mathbf{U}}$  is computed recursively starting from  $I_1$  and going forward in time (from  $n = 1$  to  $N$ ). In each time slab,  $\widetilde{\mathbf{U}}|_{I_n} \in \mathbb{P}^q(I_n; \mathcal{V}_0^H) \times \mathbb{P}^q(I_n; \mathcal{V}_0^H)$  is the solution of (12) where the function  $\mathbf{W}$  is restricted to  $I_n$  (with a zero value outside the time slab). The unknown  $\widetilde{\mathbf{U}}|_{I_n}$  accounts for the *initial* conditions given by the solution at the end point of the previous time-slab,  $\widetilde{\mathbf{U}}|_{I_n}(t_n^+) = \widetilde{\mathbf{U}}|_{I_{n-1}}(t_{n-1}^-) = [\widetilde{\mathbf{u}}_u(t_{n-1}^-), \widetilde{\mathbf{u}}_v(t_{n-1}^-)]$  (or  $[\mathbf{u}_0, \mathbf{v}_0]$  for the first slab). In general, this method requires solving for each time step  $2q$  coupled spatial problems in  $\mathcal{V}_0^H$ . Recall that the Newmark method requires a single problem in  $\mathcal{V}_0^H$  at each time step. However, for  $q = 1$  the block system of algebraic linear equations of

double size can be pre-processed to make it equivalent to the Newmark method with parameters  $\beta = 1/4$  and  $\gamma = 1/2$ , see [35] for a detailed proof.

Eriksson *et al.* [50] consider an alternative energy consistent weak form analogous to (9), where the velocity-displacement compatibility and initial conditions for the displacements are enforced using the energy product  $a(\cdot, \cdot)$ . The weak formulation reads: find  $\mathbf{U} \in \mathcal{W}_0 \times \mathcal{W}_0$  such that

$$B_{\text{CGA}}(\mathbf{U}, \mathbf{W}) = L_{\text{CGA}}(\mathbf{W}) \quad \forall \mathbf{W} \in \widehat{\mathcal{W}} \times \widehat{\mathcal{W}}. \quad (13)$$

where

$$\begin{aligned} B_{\text{CGA}}(\mathbf{U}, \mathbf{W}) &:= \int_I m(\dot{\mathbf{u}}_v + a_1 \mathbf{u}_v, \mathbf{w}_v) dt + \int_I a(\mathbf{u}_u + a_2 \mathbf{u}_v, \mathbf{w}_v) dt + m(\mathbf{u}_v(0^+), \mathbf{w}_v(0^+)) \\ &\quad + \int_I a(\dot{\mathbf{u}}_u - \mathbf{u}_v, \mathbf{w}_u) dt + a(\mathbf{u}_u(0^+), \mathbf{w}_u(0^+)), \\ L_{\text{CGA}}(\mathbf{W}) &:= \int_I l(t; \mathbf{w}_v) dt + a(\mathbf{u}_0, \mathbf{w}_u(0^+)) + m(\mathbf{v}_0, \mathbf{w}_v(0^+)). \end{aligned}$$

It is easily seen that the bilinear form  $B_{\text{CGA}}(\cdot, \cdot)$  is energy consistent, namely  $B_{\text{CGA}}(\mathbf{U}, \mathbf{U}) = B(\mathbf{u}, \mathbf{u})$ . Note that the previous CGM variational formulation does not fulfill this property, that is  $B_{\text{CGM}}(\mathbf{U}, \mathbf{U}) \neq B(\mathbf{u}, \mathbf{u})$ . This is due to the fact that for CGM both the initial conditions for the displacements  $\mathbf{u}_u(0^+) = \mathbf{u}_0$  and the compatibility condition between velocities and displacements  $\mathbf{u}_v = \dot{\mathbf{u}}_u$  are enforced in weak form using the mass product  $m(\cdot, \cdot)$ .

The discrete version of problem (13) reads: find  $\tilde{\mathbf{U}} \in \mathcal{W}_0^{H, \Delta t} \times \mathcal{W}_0^{H, \Delta t}$  such that

$$B_{\text{CGA}}(\tilde{\mathbf{U}}, \mathbf{W}) = L_{\text{CGA}}(\mathbf{W}) \quad \forall \mathbf{W} \in \widehat{\mathcal{W}}^{H, \Delta t} \times \widehat{\mathcal{W}}^{H, \Delta t}. \quad (14)$$

Finally, the double field discontinuous Galerkin method introduced by Johnson [53] is presented. This approach is a variant of the weak problem (13) in which both the trial and test functions are allowed to be discontinuous at time points in  $\mathcal{T}$ .

The continuity of the solution is weakly imposed by adding extra terms to the variational formulation, penalizing the time jumps of the solution at  $\mathcal{T}$ . The time discontinuous Galerkin weak form reads: find  $\mathbf{U} \in \widehat{\mathcal{W}} \times \widehat{\mathcal{W}}$  such that

$$B_{\text{DG}}(\mathbf{U}, \mathbf{W}) = L_{\text{CGA}}(\mathbf{W}) \quad \forall \mathbf{W} \in \widehat{\mathcal{W}} \times \widehat{\mathcal{W}}, \quad (15)$$

where

$$\begin{aligned} B_{\text{DG}}(\mathbf{U}, \mathbf{W}) &:= \sum_{n=1}^N \int_{I_n} (m(\dot{\mathbf{u}}_v + a_1 \mathbf{u}_v, \mathbf{w}_v) + a(\mathbf{u}_u + a_2 \mathbf{u}_v, \mathbf{w}_v)) dt + m(\mathbf{u}_v(0^+), \mathbf{w}_v(0^+)) \\ &\quad + \sum_{n=1}^N \int_{I_n} a(\dot{\mathbf{u}}_u - \mathbf{u}_v, \mathbf{w}_u) dt + a(\mathbf{u}_u(0^+), \mathbf{w}_u(0^+)) \\ &\quad + \sum_{n=1}^{N-1} m(\mathbf{u}_v(t_n^+) - \mathbf{u}_v(t_n^-), \mathbf{w}_v(t_n^+)) + \sum_{n=1}^{N-1} a(\mathbf{u}_v(t_n^+) - \mathbf{u}_u(t_n^-), \mathbf{w}_u(t_n^+)). \end{aligned}$$

Note that, in order to obtain an energy consistent bilinear form, the jumps of the velocities and displacements at time  $t_n$ ,  $\mathbf{u}_v(t_n^+) - \mathbf{u}_v(t_n^-)$  and  $\mathbf{u}_u(t_n^+) - \mathbf{u}_u(t_n^-)$  respectively, are introduced differently in the formulation. The mass bilinear form  $m(\cdot, \cdot)$  is used for velocities whereas the energy product  $a(\cdot, \cdot)$  is used for displacements, in such a way that  $B_{\text{DG}}(\mathbf{U}, \mathbf{U}) = B(\mathbf{u}, \mathbf{u})$ .

The discrete version of problem (15) is obtained replacing the space  $\widehat{\mathcal{W}}$  by the discrete space  $\widehat{\mathcal{W}}^{H,\Delta t}$  defined in equation (11b), namely: find  $\widehat{\mathbf{U}} := [\widehat{\mathbf{u}}_u, \widehat{\mathbf{u}}_v] \in \widehat{\mathcal{W}}^{H,\Delta t} \times \widehat{\mathcal{W}}^{H,\Delta t}$  such that

$$B_{\text{DG}}(\widehat{\mathbf{U}}, \mathbf{W}) = L_{\text{CGA}}(\mathbf{W}) \quad \forall \mathbf{W} \in \widehat{\mathcal{W}}^{H,\Delta t} \times \widehat{\mathcal{W}}^{H,\Delta t}. \quad (16)$$

The discrete problem (16) leads to  $N$  uncoupled local problems posed over the time slabs  $\Omega \times I_n$ ,  $n = 1, \dots, N$ . As in the previous double field formulations, the solution is computed recursively starting from  $I_1$  and going forward in time. This approach requires solving, in each time slab,  $2(q+1)$  coupled spatial problems in  $\mathcal{V}_0^H$ . Thus, for  $q = 1$  the dimension of the linear system to be solved in each time slab is four times larger than the system to be solved with the Newmark method. An efficient resolution strategy for this problem is presented in [29].

The error estimation techniques described in the forthcoming sections are presented using the previous variational formulations. Specifically, the recovery estimates described in section 4 consider the double field discontinuous Galerkin formulation (15), the explicit estimates and dual weighted residual technique described in section 5 are based on the double field time-continuous formulations (9) and (13) and finally, the implicit error estimates presented in section 6 require deriving error representations based on the standard single-field time-continuous Galerkin method (8).

### 3 Error measures and error representation

#### 3.1 Errors and error equations

The error associated with a single field numerical approximation  $\tilde{\mathbf{u}} \approx \mathbf{u}$ , for instance the one introduced in (7), is defined as

$$\mathbf{e} := \mathbf{u} - \tilde{\mathbf{u}} \in \mathcal{W}. \quad (17)$$

Function  $\mathbf{e}$  fulfills the following residual equation: find  $\mathbf{e} \in \mathcal{W}$

$$B(\mathbf{e}, \mathbf{w}) = L(\mathbf{w}) - B(\tilde{\mathbf{u}}, \mathbf{w}) =: R(\mathbf{w}). \quad (18)$$

Equation (18) is derived replacing the exact solution  $\mathbf{u}$  by  $\tilde{\mathbf{u}} + \mathbf{e}$  into (8) and using the linearity of forms  $B(\cdot, \cdot)$  and  $L(\cdot)$ . Note that, the residual  $R(\cdot)$  is well defined only if the numerical approximation  $\tilde{\mathbf{u}}$  is regular enough, that is  $\tilde{\mathbf{u}} \in \mathcal{W}$ . Thus, error techniques making use of the residual equation (18), in particular those presented in section 6, require that  $\tilde{\mathbf{u}} \in \mathcal{W}$ .

In the case of using a double field formulation as (12) or (14), the numerical solution has the form  $\tilde{\mathbf{U}} = [\tilde{\mathbf{u}}_u, \tilde{\mathbf{u}}_v] \in \mathcal{W}_0^{H,\Delta t} \times \mathcal{W}_0^{H,\Delta t}$  and the error is defined by

$$\mathbf{E} := [\mathbf{e}_u, \mathbf{e}_v] := [\mathbf{u} - \tilde{\mathbf{u}}_u, \dot{\mathbf{u}} - \tilde{\mathbf{u}}_v] \in \mathcal{W}_0 \times \mathcal{W}_0,$$

where  $\mathbf{e}_u$  and  $\mathbf{e}_v$  are the errors in displacements and velocities respectively. Here, two different residual equations for the double field error  $\mathbf{E}$  are derived replacing the exact solution  $\mathbf{U}$  by  $\tilde{\mathbf{U}} + \mathbf{E}$  either into equation (9) or (13). That is, the error  $\mathbf{E} \in \mathcal{W}_0 \times \mathcal{W}_0$  is the solution of both

$$B_{\text{CGM}}(\mathbf{E}, \mathbf{W}) = L_{\text{CGM}}(\mathbf{W}) - B_{\text{CGM}}(\tilde{\mathbf{U}}, \mathbf{W}) =: R_{\text{CGM}}(\mathbf{W}) \quad \forall \mathbf{W} \in \mathcal{W}_0 \times \mathcal{W}_0. \quad (19)$$

and

$$B_{\text{CGA}}(\mathbf{E}, \mathbf{W}) = L_{\text{CGA}}(\mathbf{W}) - B_{\text{CGA}}(\tilde{\mathbf{U}}, \mathbf{W}) =: R_{\text{CGA}}(\mathbf{W}) \quad \forall \mathbf{W} \in \mathcal{W}_0 \times \mathcal{W}_0. \quad (20)$$

Note that the previous two equations have the same solution, which is precisely  $\mathbf{E}$ . In practice, the criterion for selecting either equation (19) or (20) depends on whether the Galerkin orthogonality property holds or not. This is because in the error estimation procedures presented in section 5, Galerkin orthogonality is required to properly split the time and space error contributions.

Thus, if the numerical approximation  $\tilde{\mathbf{U}}$  is the solution of the discrete problem (12), the error estimation strategy utilizes equation (19) since, in this case, the following Galerkin orthogonality property holds,

$$R_{\text{CGM}}(\mathbf{W}) = 0 \quad \text{for all } \mathbf{W} \in \widehat{\mathcal{W}}^{H,\Delta t} \times \widehat{\mathcal{W}}^{H,\Delta t}. \quad (21)$$

Analogously, if the numerical approximation  $\tilde{\mathbf{U}}$  is solution of the discrete problem (14), then the error estimation strategy takes equation (20) because

$$R_{\text{CGA}}(\mathbf{W}) = 0 \quad \text{for all } \mathbf{W} \in \widehat{\mathcal{W}}^{H,\Delta t} \times \widehat{\mathcal{W}}^{H,\Delta t}.$$

If the numerical approximation is computed using other techniques, like the Newmark method, both residual equations could be used for error estimation, but the error estimation technique could not rely on Galerkin orthogonality.

The double field formulation (15) could also be used to derive a residual equation for the double field error  $\mathbf{E}$ . However, in the remainder of the paper this formulation is only used for recovery type estimates (which do not utilize a residual equation).

### 3.2 Energy measures

As previously mentioned, the bilinear form  $B(\cdot, \cdot)$  induces an energy measure in elastodynamics. For the sake of providing a physical interpretation, the energy norm  $B(\mathbf{u}, \mathbf{u})$  reads as follows:

$$B(\mathbf{u}, \mathbf{u}) = \|\mathbf{u}\|^2 + \frac{1}{2}\|\dot{\mathbf{u}}(T)\|_m^2 + \frac{1}{2}\|\mathbf{u}(T)\|_a^2 + \frac{1}{2}\|\mathbf{v}_0\|_m^2 + \frac{1}{2}\|\mathbf{u}_0\|_a^2, \quad (22)$$

where  $\|\mathbf{v}\|_m^2 := m(\mathbf{v}, \mathbf{v})$  and  $\|\mathbf{v}\|_a^2 := a(\mathbf{v}, \mathbf{v})$  are the squared norms induced by the bilinear forms  $m(\cdot, \cdot)$  and  $a(\cdot, \cdot)$ , respectively, and the space-time norm  $\|\cdot\|$  is defined as

$$\|\mathbf{v}\| := \left( \int_I a_1 \|\dot{\mathbf{v}}\|_m^2 dt + \int_I a_2 \|\dot{\mathbf{v}}\|_a^2 dt \right)^{1/2}. \quad (23)$$

The terms  $\frac{1}{2}\|\dot{\mathbf{u}}(T)\|_m^2$  and  $\frac{1}{2}\|\mathbf{u}(T)\|_a^2$  are the kinetic and elastic energy of  $\mathbf{u}$  at time  $t = T$ , while the term  $\|\mathbf{u}\|^2$  stands for the dissipated energy between times  $t = 0$  and  $t = T$  due to the presence of damping in the equations, introduced by  $a_1$  and  $a_2$ .

The different terms in (22) are used in the following to measure the error. For instance, the recovery estimates presented in section 4 make use of the squared norm  $\|\cdot\|_m^2 + \|\cdot\|_a^2$ . Note that this is the only relevant energy measure of the error in the case of elastodynamics (for  $a_1 = a_2 = 0$  the dissipated error is zero,  $\|\mathbf{e}\| = 0$ ). On the contrary, dissipation is crucial to derive error bounds, as it is shown in section 6. Actually, the techniques computing upper bounds yield estimates  $\eta_{\text{ener}}$  such that  $\|\mathbf{e}\| \leq \eta_{\text{ener}}$ .

However, there are norms different than those appearing in (22) which are also used in the literature. As an example, reference [54] measures the error with an  $L^2$  norm of the displacements at the final simulation time  $T$ .

### 3.3 Quantities of interest and adjoint problem

Information provided by global error estimates (based on global norms) is not sufficient to make engineering decisions. Alternatively, the end-user often prefers measuring the error using some specific Quantity of Interest (QoI), which are particular functional outputs of the solution.

The quantity of interest is defined by a functional  $L^\mathcal{O} : \mathcal{W} \rightarrow \mathbb{R}$  which extracts a single representative scalar value of the whole space-time solution. The value  $L^\mathcal{O}(\mathbf{u})$  is the quantity of interest, which is approximated by  $L^\mathcal{O}(\tilde{\mathbf{u}})$  given a numerical approximation of the solution  $\tilde{\mathbf{u}} \approx \mathbf{u}$ . Goal-oriented error estimation strategies aim at assessing the quality of  $L^\mathcal{O}(\tilde{\mathbf{u}})$ , that is, the difference between the exact quantity of interest  $L^\mathcal{O}(\mathbf{u})$  and the approximated one  $L^\mathcal{O}(\tilde{\mathbf{u}})$ ,  $L^\mathcal{O}(\mathbf{u}) - L^\mathcal{O}(\tilde{\mathbf{u}})$ . In the remainder of the paper it is assumed that the functional  $L^\mathcal{O}$  is linear. Thus,  $L^\mathcal{O}(\mathbf{u}) - L^\mathcal{O}(\tilde{\mathbf{u}})$  coincides with  $L^\mathcal{O}(\mathbf{e})$ . However, non-linear functionals can also be handled using the same strategies after a simple linearization, see [35, 47] for details.

The estimation of value  $L^\mathcal{O}(\mathbf{e})$  requires introducing an auxiliary problem associated with functional  $L^\mathcal{O}(\cdot)$ , usually denoted by *adjoint* or *dual* problem [31, 47, 42, 43, 44, 45, 46]. The variational form of the adjoint problem consists in finding  $\mathbf{u}^d \in \mathcal{W}$  such that

$$B(\mathbf{w}, \mathbf{u}^d) = L^\mathcal{O}(\mathbf{w}) \quad \forall \mathbf{w} \in \mathcal{W}. \quad (24)$$

The adjoint solution  $\mathbf{u}^d$  characterizes the quantity of interest defined by  $L^\mathcal{O}(\cdot)$ . Note that if  $\mathbf{u}^d$  is available, the computable quantity  $L(\mathbf{u}^d)$  is equal to the quantity of interest  $L^\mathcal{O}(\mathbf{u})$ . In that sense,  $\mathbf{u}^d$  can be seen as the Riesz representation of functional  $L^\mathcal{O}(\cdot)$ .

In practice,  $L^\mathcal{O}(\cdot)$  is selected with the same structure as the functional  $L(\cdot)$ , namely

$$L^\mathcal{O}(\mathbf{w}) := \int_0^T (\mathbf{f}^\mathcal{O}(t), \dot{\mathbf{w}}(t)) \, dt + \int_0^T (\mathbf{g}^\mathcal{O}(t), \dot{\mathbf{w}}(t))_{\Gamma_N} \, dt + m(\mathbf{v}^\mathcal{O}, \dot{\mathbf{w}}(T)) + a(\mathbf{u}^\mathcal{O}, \mathbf{w}(T)), \quad (25)$$

where  $\mathbf{f}^\mathcal{O}$ ,  $\mathbf{g}^\mathcal{O}$ ,  $\mathbf{v}^\mathcal{O}$  and  $\mathbf{u}^\mathcal{O}$  are the data characterizing the quantity of interest. The functions  $\mathbf{f}^\mathcal{O}$  and  $\mathbf{g}^\mathcal{O}$  extract global or localized averages of velocities in  $\Omega$  and  $\Gamma_N$ , respectively, over the whole time interval  $[0, T]$ . The fields  $\mathbf{v}^\mathcal{O}$  and  $\mathbf{u}^\mathcal{O}$  play the role of weighting functions to compute averages of velocities and strains at the final simulation time  $T$ .

In this case, the associated strong form of the adjoint problem is

$$\rho(\ddot{\mathbf{u}}^d - a_1 \dot{\mathbf{u}}^d) - \nabla \cdot \boldsymbol{\sigma}^d = -\mathbf{f}^\mathcal{O} \quad \text{in } \Omega \times I, \quad (26a)$$

$$\mathbf{u}^d = \mathbf{0} \quad \text{on } \Gamma_D \times I, \quad (26b)$$

$$\boldsymbol{\sigma}^d \cdot \mathbf{n} = -\mathbf{g}^\mathcal{O} \quad \text{on } \Gamma_N \times I, \quad (26c)$$

$$\mathbf{u}^d = \mathbf{u}^\mathcal{O} \quad \text{at } \Omega \times \{T\}, \quad (26d)$$

$$\dot{\mathbf{u}}^d = \mathbf{v}^\mathcal{O} \quad \text{at } \Omega \times \{T\}, \quad (26e)$$

with the constitutive law

$$\boldsymbol{\sigma}^d(\mathbf{u}^d) := \mathcal{C} : \boldsymbol{\varepsilon}(\mathbf{u}^d - a_2 \dot{\mathbf{u}}^d). \quad (27)$$

Note that the terms affected by  $a_1$  and  $a_2$  have opposite sign that the ones in the original problem (1). Consequently, the adjoint problem has exactly the same structure as the original (1) if integrated backwards in time starting from the *final conditions* (26d) and (26e).

Having selected the format of the quantity of interest given in (25) yields the adjoint problem (24) analogous to the original one (1). Thus, the same computer code available for solving the original problem (1) can be reused to solve the adjoint problem (26).

**Remark 2.** Note that the functional  $L^\mathcal{O}(\cdot)$  as defined in (25) does not directly allow to compute averages of the displacements of  $\mathbf{u}$  over the time interval  $I = (0, T]$ , namely

$$\int_I (\boldsymbol{\lambda}(t), \mathbf{u}(t)) \, dt. \quad (28)$$

However, it is easy to see that it is possible to express this quantity as

$$\int_I (\boldsymbol{\lambda}(t), \mathbf{u}(t)) \, dt = \int_0^T (\mathbf{f}^\mathcal{O}(t), \dot{\mathbf{u}}(t)) \, dt - (\mathbf{f}^\mathcal{O}(0), \mathbf{u}_0),$$

for

$$\mathbf{f}^\mathcal{O}(t) = \int_t^T \boldsymbol{\lambda}(\xi) \, d\xi. \quad (29)$$

Since the term  $(\mathbf{f}^\mathcal{O}(0), \mathbf{u}_0)$  is constant, assessing the error in the quantity of interest (28) is equivalent to assess  $L^\mathcal{O}(\mathbf{e})$  where the data characterizing  $L^\mathcal{O}(\cdot)$  are  $\mathbf{g}^\mathcal{O} = \mathbf{v}^\mathcal{O} = \mathbf{u}^\mathcal{O} = \mathbf{0}$  and  $\mathbf{f}^\mathcal{O}$  defined in equation (29).

Similarly, the average of displacements at the final time of the computation

$$(\boldsymbol{\lambda}^\circ, \mathbf{u}(T)) + (\boldsymbol{\lambda}_N^\circ, \mathbf{u}(T))_{\Gamma_N}, \quad (30)$$

where the data  $\boldsymbol{\lambda}^\circ$  and  $\boldsymbol{\lambda}_N^\circ$  are weighting functions allowing to localize the average of displacements in some sub-domains in  $\Omega$  and  $\Gamma_N$  respectively, is neither directly included in (25). In this case, an auxiliary problem is introduced: find  $\mathbf{u}^\circ \in \mathcal{V}_0$  solution of the static problem

$$a(\mathbf{u}^\circ, \mathbf{w}) = (\boldsymbol{\lambda}^\circ, \mathbf{w}) + (\boldsymbol{\lambda}_N^\circ, \mathbf{w})_{\Gamma_N} \quad \forall \mathbf{w} \in \mathcal{V}_0. \quad (31)$$

Note that here  $\mathbf{u}^\circ$  is not given as part of the data  $\boldsymbol{\lambda}^\circ$  and  $\boldsymbol{\lambda}_N^\circ$  characterizing (30). The function  $\mathbf{u}^\circ$  has to be computed as the solution of (31). Taking  $\mathbf{w} = \mathbf{e}(T)$  in (31) it is easily seen that assessing the error in the quantity of interest (30) is equivalent to assess  $L^\circ(\mathbf{e})$  where in this case the data characterizing  $L^\circ(\cdot)$  are  $\mathbf{f}^\circ = \mathbf{g}^\circ = \mathbf{v}^\circ = \mathbf{0}$  and  $\mathbf{u}^\circ$  defined in equation (31).

**Remark 3** (Illustrative example). *The following example illustrates the adjoint problem given in (26) for a one dimensional example. The spatial computational domain is  $\Omega = (0, 1)$  m, the boundaries are  $\Gamma_N = \{0 \text{ m}\}$  and  $\Gamma_D = \{1 \text{ m}\}$ , and the time interval is  $I = (0, 2]$  s. The material properties are  $E = 1 \text{ Pa}$ ,  $\nu = 0$ ,  $\rho = 1, \text{ kg/m}^3$  and  $a_1 = a_2 = 0 \text{ s}$ . Two different adjoint problems are illustrated in this remark, associated with the quantities of interest*

$$L_1^\circ(w) = \int_I \int_\Omega \alpha(t)\beta(x)\dot{w}(x,t) \, dx \, dt \quad \text{and} \quad L_2^\circ(w) = \int_I \int_\Omega \alpha(t)\beta(x)w(x,t) \, dx \, dt,$$

where  $\alpha(t)$  and  $\beta(x)$  are the time dependent functions defined in figure 1. Note that

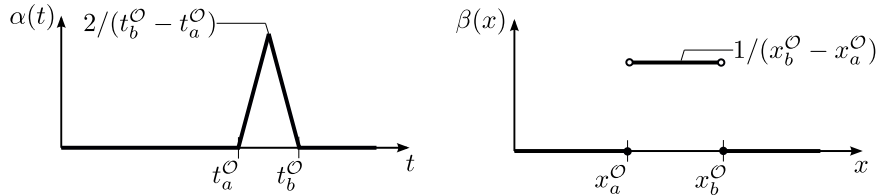


Figure 1: Definition of  $\alpha(t)$  and  $\beta(x)$ , (left) and (right) respectively.

$L_1^\circ(\cdot)$  corresponds to take  $\mathbf{g}^\circ = \mathbf{v}^\circ = \mathbf{u}^\circ = \mathbf{0}$  and  $\mathbf{f}^\circ = \alpha(t)\beta(x)$  in equation (25) and provides a weighted average of velocities in the space-time region  $S^\circ = (x_a^\circ, x_b^\circ) \times (t_a^\circ, t_b^\circ)$ . On the other hand,  $L_2^\circ(\cdot)$  corresponds to take  $\boldsymbol{\lambda} = \alpha(t)\beta(x)$  in (28) and provides a weighted average of displacement in  $S^\circ$ . In this example, the region  $S^\circ$  is characterized by  $x_a^\circ = 0.2 \text{ m}$ ,  $x_b^\circ = 0.3 \text{ m}$ ,  $t_a^\circ = 1.8 \text{ s}$  and  $t_b^\circ = 1.9 \text{ s}$ .

The adjoint problems associated with quantities  $L_1^\circ$  and  $L_2^\circ$  are plotted in figure 2. Note that, the adjoint solutions are indeed the region of influence of each quantity of interest. That is, any perturbation taking place where the adjoint solution is zero has no influence in the quantity of interest. Note also that, the influence regions are different for quantities  $L_1^\circ$  and  $L_2^\circ$  even though they provide information of the solution  $\mathbf{u}$  in the same space-time region  $S^\circ$ .

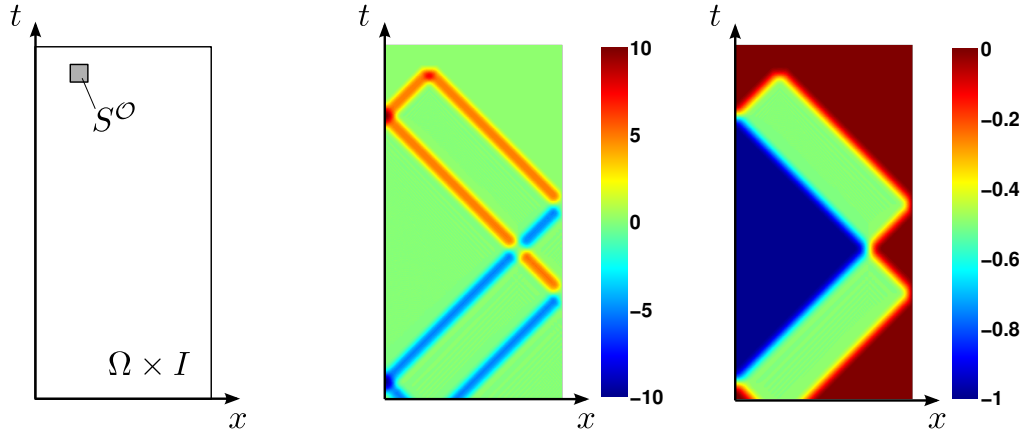


Figure 2: Illustration of the adjoint problem for two quantities of interest  $L_1^\mathcal{O}$  and  $L_2^\mathcal{O}$  (average of velocities and displacements in the region  $S^\mathcal{O}$ , respectively). Definition of the space-time domain  $\Omega \times I$  and region of interest  $S^\mathcal{O}$  (left). Adjoint velocities [m/s] for quantity  $L_1^\mathcal{O}$  (center). Adjoint velocities [m/s] for quantity  $L_2^\mathcal{O}$  (right).

The definition of the adjoint problem given in (24) depends on the weak form selected for the direct problem. Thus, the adjoint problems associated with the double field formulations CGM and CGA presented in (9) and (13) are introduced in the following. Note that the regularity restrictions for these problems are weaker than those of the previous adjoint problem. The adjoint problem associated with the formulation denoted as DG is not presented because, as previously said, this formulation is only used in the context of recovery type estimates for energy error measures.

The quantity of interest of a double field function  $\mathbf{W} = [\mathbf{w}_u, \mathbf{w}_v] \in \mathcal{W}_0 \times \mathcal{W}_0$  is defined as

$$L_D^\mathcal{O}(\mathbf{W}) := L_u^\mathcal{O}(\mathbf{w}_u) + L_v^\mathcal{O}(\mathbf{w}_v), \quad (32)$$

where  $L_v^\mathcal{O} : \mathcal{W}_0 \rightarrow \mathbb{R}$  and  $L_u^\mathcal{O} : \mathcal{W}_0 \rightarrow \mathbb{R}$  are linear functionals extracting quantities of interest from velocities and displacements respectively. The variational form of the adjoint problem associated with a double field quantity  $L_D^\mathcal{O}(\cdot)$  is introduced for both formulations CGM and CGA given in section 2.2.2. Specifically, if the variational formulation CGM presented in (9) is considered, then, the adjoint problem reads: find  $\mathbf{U}^d := [\mathbf{u}_u^d, \mathbf{u}_v^d] \in \mathcal{W}_0 \times \mathcal{W}_0$  such that

$$B_{\text{CGM}}(\mathbf{W}, \mathbf{U}^d) = L_D^\mathcal{O}(\mathbf{W}) \quad \forall \mathbf{W} \in \mathcal{W}_0 \times \mathcal{W}_0, \quad (33)$$

while considering the variational formulation CGA of (13) yields: find  $\mathbf{U}^d \in \mathcal{W}_0 \times \mathcal{W}_0$  such that

$$B_{\text{CGA}}(\mathbf{W}, \mathbf{U}^d) = L_D^\mathcal{O}(\mathbf{W}) \quad \forall \mathbf{W} \in \mathcal{W}_0 \times \mathcal{W}_0. \quad (34)$$

As discussed for the single field quantities of interest, in many practical applications it is important that the adjoint variational problem admits a strong form representation like the one given in (26). This introduces restrictions to the form of the linear functionals describing the quantities of interest as given in (32).



Equation (33) leads to the strong equations (26) for functionals  $L_u^\mathcal{O}(\cdot)$  and  $L_v^\mathcal{O}(\cdot)$  defined as

$$L_u^\mathcal{O}(\mathbf{w}_u) := m(\mathbf{u}^\mathcal{O}, \mathbf{w}_u(T)) \text{ and} \quad (35a)$$

$$L_v^\mathcal{O}(\mathbf{w}_v) := \int_0^T (\mathbf{f}^\mathcal{O}(t), \mathbf{w}_v(t)) dt + \int_0^T (\mathbf{g}^\mathcal{O}(t), \mathbf{w}_v(t))_{\Gamma_N} dt + m(\mathbf{v}^\mathcal{O}, \mathbf{w}_v(T)), \quad (35b)$$

while equation (33) leads to the strong problem (26) for

$$L_u^\mathcal{O}(\mathbf{w}_u) := a(\mathbf{u}^\mathcal{O}, \mathbf{w}_u(T)) \text{ and} \quad (36a)$$

$$L_v^\mathcal{O}(\mathbf{w}_v) := \int_0^T (\mathbf{f}^\mathcal{O}(t), \mathbf{w}_v(t)) dt + \int_0^T (\mathbf{g}^\mathcal{O}(t), \mathbf{w}_v(t))_{\Gamma_N} dt + m(\mathbf{v}^\mathcal{O}, \mathbf{w}_v(T)). \quad (36b)$$

It is worth noting that the quantity of interest associated to the energy consistent double field formulation CGA is the equivalent to the single field quantity of interest defined in (25). This is because the definition of  $L_D^\mathcal{O}$  in (36) is such that  $L_D^\mathcal{O}([\mathbf{w}, \dot{\mathbf{w}}]) = L^\mathcal{O}(\mathbf{w})$ .

### 3.4 Error representation with adjoint problem

The adjoint problem allows rewriting the error in the quantity of interest in terms of residuals, combining the original and adjoint problems. Thus, the error assessment for the quantity of interest is performed using standard error estimation techniques designed for global error measures. Different error representations for  $L^\mathcal{O}(\mathbf{e})$ , or similarly for  $L_D^\mathcal{O}(\mathbf{E})$  in the case of a double field formulation, are used for the error assessment strategies presented here.

Techniques aiming at furnishing bounds for the error in the quantity of interest, like the ones presented in section 6, consider an error representation based on the adjoint weak residual,  $R^d(\cdot)$ , associated with the numerical approximation of the adjoint problem  $\tilde{\mathbf{u}}^d \approx \mathbf{u}^d$ . This error representation reads

$$L^\mathcal{O}(\mathbf{e}) = R^d(\mathbf{e}) + B(\mathbf{e}, \tilde{\mathbf{u}}^d) = R^d(\mathbf{e}) + R(\tilde{\mathbf{u}}^d), \quad (37)$$

where

$$R^d(\mathbf{w}) := L^\mathcal{O}(\mathbf{w}) - B(\mathbf{w}, \tilde{\mathbf{u}}^d). \quad (38)$$

Equation (37) is derived taking  $\mathbf{w} = \mathbf{e}$  into (38) along with the definition of the primal residual. In equation (37), the error in the quantity of interest is expressed as the non-computable term  $R^d(\mathbf{e})$  plus the computable term  $R(\tilde{\mathbf{u}}^d)$ . Thus, bounds for  $L^\mathcal{O}(\mathbf{e})$  are obtained finding bounds for  $R^d(\mathbf{e})$ . Note that,  $R(\tilde{\mathbf{u}}^d)$  is a computable quantity, not necessarily equal to zero. Therefore, numerical approximations  $\tilde{\mathbf{u}}$  and  $\tilde{\mathbf{u}}^d$  are not assumed to fulfill any Galerkin orthogonality property. That allows using many different computational methodologies to obtain  $\tilde{\mathbf{u}}$  and  $\tilde{\mathbf{u}}^d$ . The only requirement is that the numerical solutions are regular enough, that is  $\tilde{\mathbf{u}}, \tilde{\mathbf{u}}^d \in \mathbf{W}$  and that the bounds for  $R^d(\mathbf{e})$  are available.

On the other hand, the error estimates presented in section 5 utilize a different error representation. In this case, the error in the quantity of interest is expressed using the

primal residual and the exact adjoint solution. The following development is analogous for the CGM and the CGA formulations. For the sake of simplicity, the presentation is done for CGM but equivalent expressions stand replacing CGM by CGA. One of the simpler versions of the error representation reads

$$L_D^{\mathcal{O}}(\mathbf{E}) = R_{\text{CGM}}(\mathbf{U}^d). \quad (39)$$

The previous equation is a direct consequence of the definition of both the residuals and the adjoint solution.

In the error estimation setup, replacing  $\mathbf{U}^d$  by an approximated value  $\tilde{\mathbf{U}}^d$ , the error representation (39) gives an accurate approximation of the error in the quantity of interest. However, the local error contributions provided by the local restrictions of this residual expression are often too pessimistic due to the cancellation effect of the contributions of opposite sign with large absolute values, see [55]. Thus, equation (39) is not used directly to compute error maps for space-time mesh adaptivity. The error representation (39) is modified adding and subtracting an arbitrary function  $\mathbf{W}^{H,\Delta t} \in \mathcal{W}_0^{H,\Delta t} \times \mathcal{W}_0^{H,\Delta t}$  in the argument of the residual,

$$L_D^{\mathcal{O}}(\mathbf{E}) = R_{\text{CGM}}(\mathbf{U}^d - \mathbf{W}^{H,\Delta t}) + R_{\text{CGM}}(\mathbf{W}^{H,\Delta t}). \quad (40)$$

Then, using Galerkin orthogonality of the residuals it follows

$$L_D^{\mathcal{O}}(\mathbf{E}) = R_{\text{CGM}}(\mathbf{U}^d - \mathbf{W}^{H,\Delta t}). \quad (41)$$

Globally, the error representation (41) is identical to (39). However, their local restrictions are different and, if  $\mathbf{W}^{H,\Delta t}$  is properly selected, (41) provides a map of local error contributions better suited for adaptive purposes. In practice, the function  $\mathbf{W}^{H,\Delta t}$  is taken as the projection onto the test space  $\mathcal{W}_0^{H,\Delta t} \times \mathcal{W}_0^{H,\Delta t}$  of the computed adjoint approximation  $\tilde{\mathbf{U}}^d$ , see section 5 for details.

Note that the estimated error distribution given by representation (41) is valid only if Galerkin orthogonality holds. In practice it means that, if equation (41) is used, then  $\tilde{\mathbf{U}}$  must be the solution of the discrete problem (12).

## 4 Recovery estimates

In the following, recovery type error estimates in the framework of elastodynamics, see [29, 30, 31], are applied to the DG formulations. That is, they assess the error with respect to the solution  $\hat{\mathbf{U}}$  of the discrete problem (16). Recovery type error estimates provide error indicators for the space and time components of the error, namely  $\eta_n^s$  and  $\eta_n^t$ , associated with a point  $t_n$  of the time discretization  $\mathcal{T}$ . These indicators are input data for an adaptive procedure, allowing to select the mesh size and time step and to design adapted discretizations.

## 4.1 Space-time error splitting

For a given time  $t_n \in \mathcal{T}$ , the error associated with the DG approximation  $\widehat{\mathbf{U}} = [\widehat{\mathbf{u}}_u, \widehat{\mathbf{u}}_v]$  is defined as

$$\mathbf{E}(t_n) := \mathbf{U}(t_n) - \widehat{\mathbf{U}}(t_n^-) = [\mathbf{u}(t_n) - \widehat{\mathbf{u}}_u(t_n^-), \dot{\mathbf{u}}(t_n) - \widehat{\mathbf{u}}_v(t_n^-)].$$

Note that the error  $\mathbf{E}(t_n)$  measures both the space and time discretization errors at time  $t_n$ . In order to separate the contribution of the space and time errors, the error  $\mathbf{E}(t_n)$  is decomposed into

$$\mathbf{E}(t_n) = \mathbf{E}^s(t_n) + \mathbf{E}^t(t_n),$$

where  $\mathbf{E}^s(t_n)$  and  $\mathbf{E}^t(t_n)$  are defined as

$$\mathbf{E}^s(t_n) := \mathbf{U}(t_n) - \mathbf{U}^H(t_n) \quad \text{and} \quad \mathbf{E}^t(t_n) := \mathbf{U}^H(t_n) - \widehat{\mathbf{U}}(t_n^-). \quad (42)$$

Note that  $\mathbf{E}^s(t_n)$  and  $\mathbf{E}^t(t_n)$  are defined introducing function  $\mathbf{U}^H = [\mathbf{u}^H, \dot{\mathbf{u}}^H]$ , being  $\mathbf{u}^H$  the exact (in time) solution of the semidiscrete problem (4), which is unknown. Function  $\mathbf{U}^H$  tends to the exact solution  $\mathbf{U}$  as  $H$  tends to zero. Consequently, also  $\mathbf{E}^s(t_n)$  tends to zero with  $H$  and therefore it is referred as the space discretization error. Similarly, the term  $\mathbf{E}^t(t_n)$  tends to zero as the time step, used in the discretization of equation (4), tends to zero. Thus,  $\mathbf{E}^t(t_n)$  is associated with the error produced by the time discretization.

In order to derive local error indicators, the space and time discretization errors are measured using the kinetic and elastic energy. Note that the error measure used here corresponds to the total energy if the viscosity vanishes ( $a_1 = a_2 = 0$ ):

$$\|\mathbf{E}^s(t_n)\|_{m+a}^2 := \|\mathbf{e}_v^s(t_n)\|_m^2 + \|\mathbf{e}_u^s(t_n)\|_a^2 \quad \text{and} \quad \|\mathbf{E}^t(t_n)\|_{m+a}^2 := \|\mathbf{e}_v^t(t_n)\|_m^2 + \|\mathbf{e}_u^t(t_n)\|_a^2.$$

The goal of recovery error estimates is to furnish error indicators  $\eta_n^s$  and  $\eta_n^t$  such that

$$\eta_n^s \approx \|\mathbf{E}^s(t_n)\|_{m+a}^2 \quad \text{and} \quad \eta_n^t \approx \|\mathbf{E}^t(t_n)\|_{m+a}^2.$$

The time error indicators  $\eta_n^t$ ,  $n = 1, \dots, N$  are directly used to define the desired size of the time step at each time  $t_n$ . On the other hand, the space error indicator  $\eta_n^s$  is decomposed into element contributions

$$\eta_n^s = \sum_{k=1}^{N_{\text{el}}} \eta_{n,k}^s, \quad (43)$$

where  $\eta_{n,k}^s$  is an estimate of the contribution of element  $\Omega_k$  to the norm  $\|\mathbf{E}^s(t_n)\|_{m+a}^2$ . The space error indicators  $\eta_{n,k}^s$  allow defining a desired element size in the zone of element  $\Omega_k$  at time  $t_n$ . Thus, combining the information provided by  $\eta_n^t$  and  $\eta_{n,k}^s$ , the adapted space and time discretizations are designed to meet the precision required.

The remainder of this section is devoted to the actual computation of the error indicators  $\eta_{k,n}^s$  and  $\eta_n^t$ .

## 4.2 Assessing time discretization errors

The local error indicators  $\eta_n^t \approx \|\mathbf{E}^t(t_n)\|_{m+a}^2$  are computed in references [29, 30] using the time-discontinuities of the DG approximation  $\widehat{\mathbf{U}}$ , namely

$$\eta_n^t := \|\widehat{\mathbf{U}}(t_n^+) - \widehat{\mathbf{U}}(t_n^-)\|_{m+a} = \|\hat{\mathbf{u}}_v(t_n^+) - \hat{\mathbf{u}}_v(t_n^-)\|_m^2 + \|\hat{\mathbf{u}}_u(t_n^+) - \hat{\mathbf{u}}_u(t_n^-)\|_a^2.$$

This definition is suggested by the super-convergent properties of the time-DG formulations, stating that the solution at  $t_n^-$  is much more accurate than the solution at  $t_n^+$ .

References [56, 57, 31] introduce alternative indicators  $\eta_n^t$  if the numerical approximation is a time-continuous function  $\widetilde{\mathbf{U}}$  instead of the DG approximation  $\widehat{\mathbf{U}}$ . In that case the indicators cannot be computed using the time jumps and, therefore, the following alternative definition is introduced

$$\eta_n^t := \|\mathbf{U}^*(t_n) - \widetilde{\mathbf{U}}(t_n)\|_{m+a}^2 = \|\mathbf{u}_v^*(t_n) - \tilde{\mathbf{u}}_v(t_n^-)\|_m^2 + \|\mathbf{u}_u^*(t_n) - \tilde{\mathbf{u}}_u(t_n^-)\|_a^2.$$

where  $\mathbf{U}^*(t_n)$  is an enhanced (in time) function computed using the numerical approximation  $\widetilde{\mathbf{U}}$  at previous time steps, see reference [31] for specific details. The post-processed solution  $\mathbf{U}^*(t_n)$  replaces the exact (in time) approximation  $\mathbf{U}^H(t_n)$  in order to obtain an error estimate.

## 4.3 Assessing space discretization errors

The natural post-process of the numerical solution for the recovery type error estimators, is performed for stress fields rather than for the displacements, see [3]. In this context, energy is measured in terms of stresses using the complementary energy norm  $\|\cdot\|_{\bar{a}}$  defined as

$$\|\boldsymbol{\sigma}\|_{\bar{a}}^2 := \bar{a}(\boldsymbol{\sigma}, \boldsymbol{\sigma}) \quad \text{where} \quad \bar{a}(\boldsymbol{\sigma}, \boldsymbol{\tau}) := \int_{\Omega} \boldsymbol{\sigma} : \mathbf{C}^{-1} : \boldsymbol{\tau} \, d\Omega.$$

Thus, value  $\|\mathbf{E}^s(t_n)\|_{m+a}^2$  is rewritten using  $\|\cdot\|_{\bar{a}}$  as

$$\|\mathbf{E}^s(t_n)\|_{m+a}^2 = \|\dot{\mathbf{u}}(t_n) - \dot{\mathbf{u}}^H(t_n)\|_m^2 + \|\boldsymbol{\sigma}(\mathbf{u}(t_n)) - \boldsymbol{\sigma}(\mathbf{u}^H(t_n))\|_{\bar{a}}^2. \quad (44)$$

In the framework of space error indicators, the semi-discrete (exact in time) functions  $\dot{\mathbf{u}}^H(t_n)$  and  $\boldsymbol{\sigma}(\mathbf{u}^H(t_n))$  are assumed to fairly approximate  $\hat{\mathbf{u}}_v(t_n)$  and  $\boldsymbol{\sigma}(\hat{\mathbf{u}}_u(t_n))$ . This is equivalent to assume that the time integration error of the numerical approximation  $\widehat{\mathbf{U}}(t_n) = [\hat{\mathbf{u}}_u(t_n), \hat{\mathbf{u}}_v(t_n)]$  is small. This requires reducing the time discretization error before assessing the space discretization error. Thus, the adaptive strategy aims at reaching the prescribed time accuracy at each time  $t_n$  before starting with the space adaptive procedure.

A computable value for  $\|\mathbf{E}^s(t_n)\|_{m+a}^2$  using equation (44) is obtained recovering approximations of the exact velocities  $\mathbf{v}_n^* \approx \dot{\mathbf{u}}(t_n)$  and stresses  $\boldsymbol{\sigma}_n^* \approx \boldsymbol{\sigma}(\mathbf{u}(t_n))$  with some post-process of  $\widehat{\mathbf{U}}(t_n)$ . Introducing these approximations, the space error contributions  $\eta_n^s \approx \|\mathbf{E}^s(t_n)\|_{m+a}^2$ ,  $n = 1, \dots, N$ , are computed as

$$\eta_n^s := \|\mathbf{v}_n^* - \hat{\mathbf{u}}_v(t_n^-)\|_m^2 + \|\boldsymbol{\sigma}_n^* - \boldsymbol{\sigma}(\hat{\mathbf{u}}_u(t_n^-))\|_{\bar{a}}^2. \quad (45)$$

The error indicator  $\eta_n^s$  corresponding to time  $t_n$  is decomposed into elementary contributions  $\eta_{n,k}^s$  associated with element  $\Omega_k$  of the mesh. The elementary contributions  $\eta_{n,k}^s$  are computed restricting the integrals in equation (45) to the elements  $\Omega_k$ . The numerical methodology providing  $\mathbf{v}_n^*$  and  $\boldsymbol{\sigma}_n^*$  is detailed below.

The post-processed stress  $\boldsymbol{\sigma}_n^*$  is usually computed using standard stress recovery techniques originally presented for static problems, see [3, 58, 59]. These techniques allow recovering an enhanced continuous stress field  $\boldsymbol{\sigma}_n^*$ , which is obtained projecting each component of the space-discontinuous stress field  $\boldsymbol{\sigma}(\hat{\mathbf{u}}_u(t_n^-))$  into the continuous finite element functional space used to approximate displacements and velocities, see figure 3. That is, the enhanced stress  $\boldsymbol{\sigma}_n^*$  is recovered in the space

$$\mathcal{S}^H := \{\boldsymbol{\tau} \in [C^0(\bar{\Omega})]^{d(d+1)/2} : \boldsymbol{\tau}|_{\Omega_k} \in [\mathbb{P}^p(\Omega_k)]^{d(d+1)/2}, k = 1, \dots, N_{\text{el}}\},$$

where  $\mathbb{P}^p(\Omega_k)$  is the space of polynomials of degree  $p$  on the element  $\Omega_k$ . In particular, the stress field  $\boldsymbol{\sigma}_n^* \in \mathcal{S}^H$  is sought as a linear combination of nodal shape functions, namely

$$\boldsymbol{\sigma}_n^* = \sum_{j=1}^{N_{\text{nod}}} \boldsymbol{\sigma}_n^*(\mathbf{x}_j) N_j(\mathbf{x}),$$

where  $N_j(\mathbf{x})$  is the finite element shape function of degree  $p$  associated to the node  $\mathbf{x}_j$ , being  $N_{\text{nod}}$  the total number of nodes. The output of the recovery procedure are the nodal values of the recovered stress field  $\boldsymbol{\sigma}_n^*(\mathbf{x}_j), j = 1, \dots, N_{\text{nod}}$ , describing the stress  $\boldsymbol{\sigma}_n^* \in \mathcal{S}^H$ .

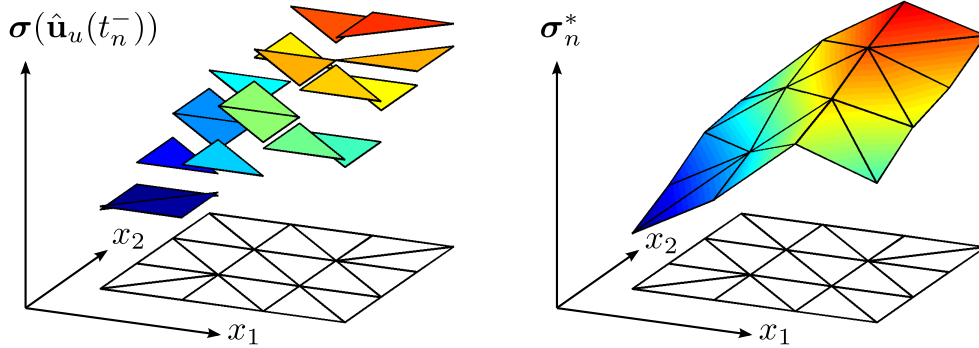


Figure 3: The the discontinuous stress field  $\boldsymbol{\sigma}(\hat{\mathbf{u}}_u(t_n^-))$  (left) is projected into the continuous space generated by the shape functions furnishing the stress field  $\boldsymbol{\sigma}_n^*$  (right) for all time steps  $n = 1, \dots, N$  (illustrated for  $p = 1$ ).

Several strategies are studied in the literature for the stress recovery. Two of the most common techniques are summarized hereafter. First, the pioneering reference by Zienkiewicz and Zhu [3] proposes to recover the stress field  $\boldsymbol{\sigma}_n^*$  using a global  $L^2$ -projection. Specifically, the recovered stress  $\boldsymbol{\sigma}_n^*$  is found solving the global discrete problem: find  $\boldsymbol{\sigma}_n^* \in \mathcal{S}^H$  such that

$$(\boldsymbol{\sigma}_n^*, \boldsymbol{\tau}) = (\boldsymbol{\sigma}(\hat{\mathbf{u}}_u(t_n^-)), \boldsymbol{\tau}) \quad \forall \boldsymbol{\tau} \in \mathcal{S}^H, \quad (46)$$

where  $(\cdot, \cdot)$  denotes the  $L^2$ -product in  $\mathcal{S}^H$ . Problem (46) is equivalent to a linear system of equations with a global mass matrix, for each component of the stress tensor, and at each time step  $t_n$ . If the space mesh changes, then the mass matrix has to be reassembled and *inverted* at each time step. Hence, this technique is not very well suited in dynamic problems. In that case, stress recovery techniques involving only local problems are preferred, as the well known SPR technique also introduced by Zienkiewicz and Zhu [58, 59].

The SPR technique allows recovering the nodal values of the recovered stress field  $\sigma_n^*(\mathbf{x}_j)$  solving local small problems associated to the mesh nodes (nodal patches). The values  $\sigma_n^*(\mathbf{x}_j)$  are found using the information provided by the set of Gauss points  $\mathcal{X}^{\omega_j}$  inside the patch  $\omega_j := \text{supp}(N_j) \subset \Omega$  surrounding the  $j$ -th node, see figure 4. A local stress field

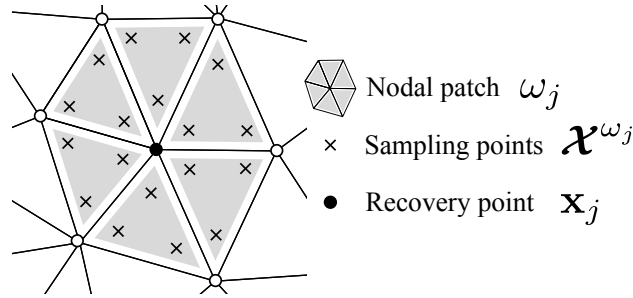


Figure 4: Definition of nodal patches and sampling points for stress recovery.

of polynomial degree  $\tilde{p}$ ,  $\sigma_n^{\omega_j} \in [\mathbb{P}^{\tilde{p}}(\omega_j)]^{d(d+1)/2}$ , is defined on the patch  $\omega_j$ . The recovered stresses  $\sigma_n^{\omega_j}$  are computed as the least squares fitting of the values of the stress  $\sigma(\hat{\mathbf{u}}_u(t_n^-))$  at the sampling points  $\mathcal{X}^{\omega_j}$ . Specifically, the stress  $\sigma_n^{\omega_j}$  is defined as

$$\sigma_n^{\omega_j} := \arg \min_{\mathbf{w} \in [\mathbb{P}^{\tilde{p}}(\omega_j)]^{d(d+1)/2}} \sum_{\mathbf{x} \in \mathcal{X}^{\omega_j}} (\mathbf{w}(\mathbf{x}) - \sigma(\hat{\mathbf{u}}_u(t_n^-))(\mathbf{x}))^2. \quad (47)$$

The recovered value  $\sigma_n^*(\mathbf{x}_j)$  is then taken to be the value of the local stress field  $\sigma_n^{\omega_j}$  at point  $\mathbf{x}_j$ , that is  $\sigma_n^*(\mathbf{x}_j) := \sigma_n^{\omega_j}(\mathbf{x}_j)$ . In practice, for linear elements,  $\tilde{p}$  is taken equal to 2. In any case,  $\tilde{p}$  must be such that the dimension of  $\mathbb{P}^{\tilde{p}}(\omega_j)$  is lower than the cardinal of  $\mathcal{X}^{\omega_j}$  to guarantee that the least squares projection is well posed.

On the other hand, the technique furnishing the enhanced velocities  $\mathbf{v}_n^*$  consists, basically, in increasing the interpolation degree of the computed velocity  $\hat{\mathbf{u}}_v(t_n^-)$ , see figure 5. This type of post-process is used in other contexts, for instance, in assessing the error  $L^2$ -norm in static problems [60, 61] or, in building enhanced vibration modes and eigenfrequencies [62]. In the following, the post-process strategy introduced in [62] is presented.

Let  $\Omega_k^{\text{patch}}$  denote the patch of elements around  $\Omega_k$ , consisting of all the elements sharing at least one node with  $\Omega_k$ , and let  $\mathcal{X}_k$  and  $\mathcal{X}_k^{\text{patch}}$  denote the set of nodes of element  $\Omega_k$  and patch  $\Omega_k^{\text{patch}}$  respectively, see figure 6.

The post-processed velocity field  $\mathbf{v}_n^*$  is found using the information provided by the restriction of  $\hat{\mathbf{u}}_v(t_n^-)$  to  $\Omega_k^{\text{patch}}$ . Specifically, a velocity field  $\mathbf{v}_n^{\mathcal{X}_k} \in [\mathbb{P}^{p+1}(\Omega_k^{\text{patch}})]^d$  is found

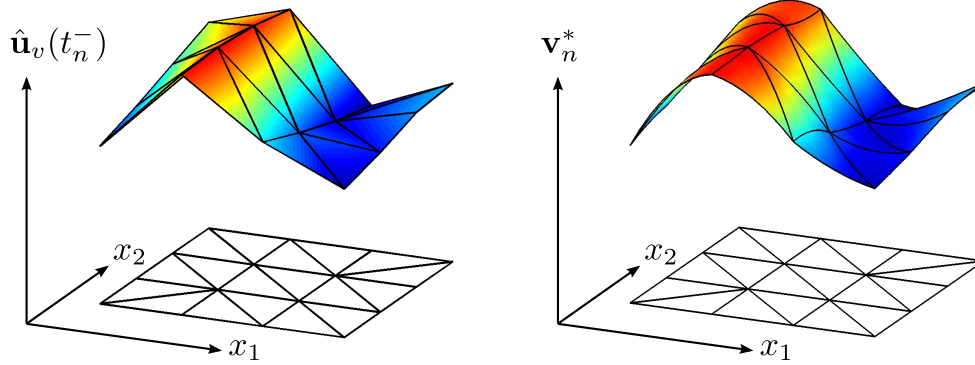


Figure 5: The recovery techniques for the velocities consist in increasing the interpolation order of  $\hat{\mathbf{u}}_v(t_n^-)$  (left) furnishing the recovered velocities  $\mathbf{v}_n^*$  (right).

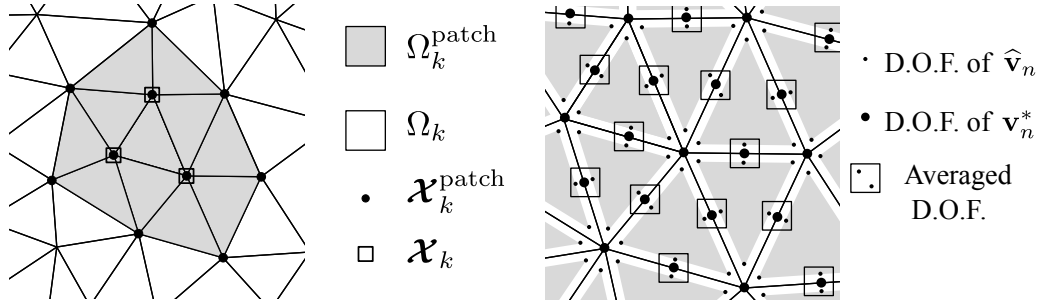


Figure 6: Definition of element patches (left) and illustration of the averaging of discontinuous function  $\hat{\mathbf{v}}_n$  into the continuous function  $\mathbf{v}_n^*$  (right).

such that it fits the values of  $\hat{\mathbf{u}}_v(t_n^-)$  at  $\mathcal{X}_k^{\text{patch}}$  in a least squares sense and coincides with  $\hat{\mathbf{u}}_v(t_n^-)$  at  $\mathcal{X}_k$ . That is,

$$\mathbf{v}_n^{\mathcal{X}_k} = \arg \min_{\mathbf{w} \in [\mathbb{P}^{p+1}(\Omega_k^{\text{patch}})]^d} \sum_{\mathbf{x} \in \mathcal{X}_k^{\text{patch}}} (\mathbf{w}(\mathbf{x}) - \hat{\mathbf{u}}_v(\mathbf{x}, t_n^-))^2 \quad (48)$$

constrained to  $\mathbf{w}(\mathbf{x}) = \hat{\mathbf{u}}_v(\mathbf{x}, t_n^-)$  for  $\mathbf{x} \in \mathcal{X}_k$ .

Problem (48) results in  $d$  (one for each component of the velocity, all with the same *mass* matrix) linear system of equations of size equal to the dimension of  $\mathbb{P}^{p+1}$  for each element of the computational mesh.

In order to obtain the post-processed velocity field  $\mathbf{v}_n^*$ , first the contributions of the restriction of the local recovered functions  $\mathbf{v}_n^{\mathcal{X}_k}$  to the corresponding element  $\Omega_k$  are summed,  $\hat{\mathbf{v}}_n := \sum_k \mathbf{v}_n^{\mathcal{X}_k}|_{\Omega_k}$ . Note that  $\hat{\mathbf{v}}_n$  is discontinuous because, for two neighboring elements  $\Omega_k$  and  $\Omega_{k'}$  with a common side  $\Gamma_{kk'} := \bar{\Omega}_k \cap \bar{\Omega}_{k'}$ , functions  $\mathbf{v}_n^{\mathcal{X}_k}|_{\Omega_k}$  and  $\mathbf{v}_n^{\mathcal{X}_{k'}}|_{\Omega_{k'}}$  coincide at the endpoints of  $\Gamma_{kk'}$  but, in general, not in the other points of  $\Gamma_{kk'}$ . In order to build up a continuous approximation  $\mathbf{v}_n^*$ , the local contributions are averaged on the

element sides. Typically  $\mathbf{v}_n^{\mathcal{X}^k}|_{\Omega_k}$  is represented with the nodal values of a finite element of degree  $p + 1$ . Therefore, computing  $\mathbf{v}_n^*$  is simply performed by just averaging the values of the degrees of freedom associated with the element edges (not vertices), as illustrated in figure 6.

## 5 Dual weighted residual & explicit residual estimates

### 5.1 Dual weighted residual method

This section presents the so-called Dual Weighted Residual (DWR) technique providing estimates for the error in the quantity of interest. This technique is introduced by Rannacher and Stuttmeier in the context of steady state linear elasticity problems [63], but it is also applied to linear elastodynamics [35, 31, 36, 14, 37, 38]. In particular, the dual weighted residual technique is presented here in the context of elastodynamics following [35].

The dual weighted residual methodology provides a scalar estimate  $\eta^{\text{dwr}}$  for the error in the quantity of interest

$$\eta^{\text{dwr}} \approx L_{\text{D}}^{\mathcal{O}}(\mathbf{E}),$$

where, here, the error  $\mathbf{E}$  is defined with respect the solution  $\tilde{\mathbf{U}}$  of the discrete double-field problem (12). Following [35] the developments are restricted to linear time descriptions, that is  $q = 1$ .

The error estimate  $\eta^{\text{dwr}}$  is obtained replacing the exact adjoint solution  $\mathbf{U}^{\text{d}}$  by an appropriate approximation  $\tilde{\mathbf{U}}^{\text{d}}$  in the error representation (39), namely

$$\eta^{\text{dwr}} := R_{\text{CGM}}(\tilde{\mathbf{U}}^{\text{d}}). \tag{49}$$

The scalar estimate  $\eta^{\text{dwr}}$  provides a single scalar quantity, which may be used in the framework of an adaptive procedure as a stopping criterion. That is, to check whether the computed numerical approximation has reached the desired accuracy. Additionally, the dual weighted residual method provides local error indicators (typically element by element) to drive goal-oriented adaptive procedures. This information is used to improve the space-time discretization in order to reduce the error  $\eta^{\text{dwr}}$ .

Deriving the local error indicators requires rewriting the error representation (41) in such a way that the contributions of the space and time discretization errors are separated. This allows to decide whether the space or the time discretizations (or both) have to be refined. First, in order to separate the space and time errors in the error representation (41), the projection  $\mathbf{\Pi}^{\Delta t} \mathbf{\Pi}^H \mathbf{U}^{\text{d}}$  of  $\mathbf{U}^{\text{d}}$  in the space  $\widehat{\mathcal{W}}^{H, \Delta t} \times \widehat{\mathcal{W}}^{H, \Delta t}$  is introduced, where  $\mathbf{\Pi}^{\Delta t}$  is a projection of a time dependent function into the space of time piecewise constant functions and  $\mathbf{\Pi}^H$  is the classical nodal interpolation projecting space-dependent functions into  $\mathcal{V}_0^H$ . In practice,  $\mathbf{\Pi}^{\Delta t}$  is defined for a time-dependent



function  $\mathbf{w}$  taking the average of  $\mathbf{w}$  inside each time interval  $I_n$

$$\Pi^{\Delta t} \mathbf{w}|_{I_n} := \frac{1}{\text{meas}(I_n)} \int_{I_n} \mathbf{w} \, dt.$$

Note that the error in the projection  $\mathbf{w} - \Pi^{\Delta t} \mathbf{w}$  is orthogonal to piecewise constant functions in time

$$\int_{I_n} (\mathbf{w} - \Pi^{\Delta t} \mathbf{w}) \cdot \mathbf{v} \, dt = 0 \quad \forall \mathbf{v} \in [\mathbb{P}^0(I_n)]^d. \quad (50)$$

Thus, taking  $\mathbf{W}^{H,\Delta t} = \Pi^{\Delta t} \Pi^H \mathbf{U}^d$  in (41) yields

$$L_D^{\mathcal{O}}(\mathbf{E}) = R_{\text{CGM}}(\mathbf{U}^d - \Pi^{\Delta t} \Pi^H \mathbf{U}^d). \quad (51)$$

**Remark 4.** Figure 7 illustrates the projection operators  $\Pi^H$  and  $\Pi^{\Delta t}$  using the adjoint velocity  $\mathbf{u}_v^d$  shown in figure 2 associated to the quantity of interest  $L_1^{\mathcal{O}}(\cdot)$ . The exact adjoint velocity  $\mathbf{u}_v^d \in \mathcal{W}$  is continuous both in space and time. Function  $\Pi^{\Delta t} \mathbf{u}_v^d \in \widehat{\mathcal{W}}^{\Delta t}$ , with

$$\widehat{\mathcal{W}}^{\Delta t} := \{\mathbf{v} \in [L^2(I; \mathcal{V}_0)]^d : \mathbf{v}|_{I_n} \in [\mathbb{P}^{q-1}(I_n; \mathcal{V}_0)]^d, n = 1, \dots, N\},$$

is piecewise polynomial (constant for  $q = 1$ ) in time. However, the spacial description of functions in  $\widehat{\mathcal{W}}^{\Delta t}$  is infinite dimensional. Finally, the fully discrete projection  $\Pi^{\Delta t} \Pi^H \mathbf{u}_v^d \in \widehat{\mathcal{W}}^{H,\Delta t}$  is continuous and piecewise polynomial in space, see figure 7.

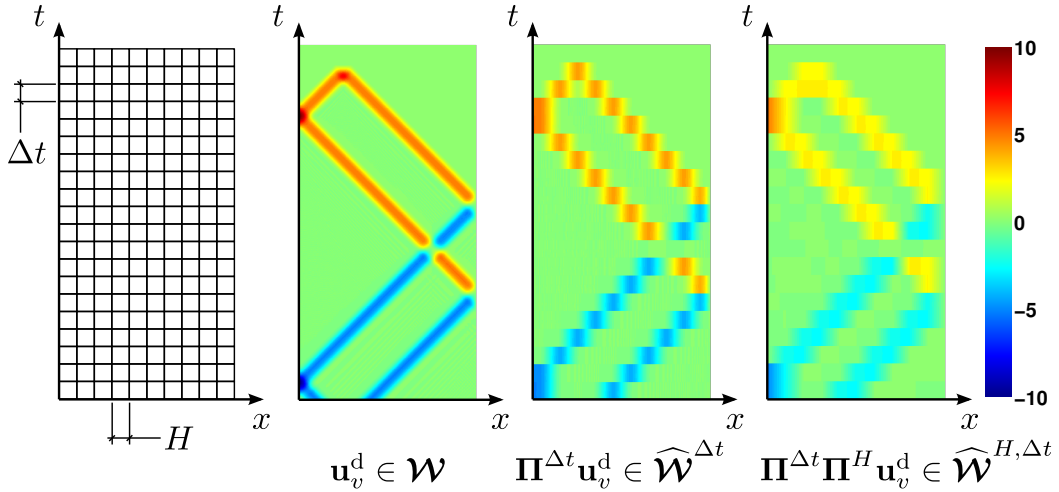


Figure 7: Space-time discretization defining the spaces  $\widehat{\mathcal{W}}^{\Delta t}$  and  $\widehat{\mathcal{W}}^{H,\Delta t}$  (outer left). Adjoint velocity field  $\mathbf{u}_v^d$  associated to the quantity of interest  $L_1^{\mathcal{O}}(\cdot)$  (see figure 2) and its projections  $\Pi^{\Delta t} \mathbf{u}_v^d \in \widehat{\mathcal{W}}^{\Delta t}$  and  $\Pi^{\Delta t} \Pi^H \mathbf{u}_v^d \in \widehat{\mathcal{W}}^{H,\Delta t}$  (right).

The space and time errors are separated adding and subtracting  $\Pi^{\Delta t} \mathbf{U}^d$  in equation (51)

$$L_D^{\mathcal{O}}(\mathbf{E}) = R_{\text{CGM}}(\mathbf{U}^d - \Pi^{\Delta t} \mathbf{U}^d) + R_{\text{CGM}}(\Pi^{\Delta t} (\mathbf{U}^d - \Pi^H \mathbf{U}^d)). \quad (52)$$

The terms  $R_{\text{CGM}}(\mathbf{U}^d - \mathbf{\Pi}^{\Delta t} \mathbf{U}^d)$  and  $R_{\text{CGM}}(\mathbf{\Pi}^{\Delta t}(\mathbf{U}^d - \mathbf{\Pi}^H \mathbf{U}^d))$  are associated with the time and space discretization errors respectively. Indeed,  $R_{\text{CGM}}(\mathbf{U}^d - \mathbf{\Pi}^{\Delta t} \mathbf{U}^d)$  tends to zero as the time discretization is refined whereas  $R_{\text{CGM}}(\mathbf{\Pi}^{\Delta t}(\mathbf{U}^d - \mathbf{\Pi}^H \mathbf{U}^d))$  tends to zero as the space discretization is refined.

Once the space and time errors are separated, the next step to obtain the local error contributions is splitting the integrals in  $R_{\text{CGM}}(\cdot)$  using the space-time cells  $\Omega_k \times I_n$  associated with the elements  $\Omega_k$  and time intervals  $I_n$ . That is

$$\begin{aligned} R_{\text{CGM}}(\mathbf{W}) &= \sum_{n=1}^N \sum_{k=1}^{N_{\text{el}}} \int_{I_n} [(\mathbf{f}, \mathbf{w}_v)_{\Omega_k} + (\mathbf{g}, \mathbf{w})_{\Gamma_N \cap \partial \Omega_k} - m(\dot{\tilde{\mathbf{u}}}_v, \mathbf{w}_v)_{\Omega_k} - a(\tilde{\mathbf{u}}_u, \mathbf{w}_v)] dt \\ &\quad + \sum_{n=1}^N \sum_{k=1}^{N_{\text{el}}} \int_{I_n} (\mathbf{r}_u, \mathbf{w}_u)_{\Omega_k} dt + \sum_{k=1}^{N_{\text{el}}} [(\mathbf{r}_u^0, \mathbf{w}_u(0))_{\Omega_k} + (\mathbf{r}_v^0, \mathbf{w}_v(0))_{\Omega_k}], \end{aligned} \quad (53)$$

where

$$\mathbf{r}_u := \rho(\dot{\tilde{\mathbf{u}}}_u - \tilde{\mathbf{u}}_v), \quad \mathbf{r}_u^0 := \rho(\mathbf{u}_0 - \tilde{\mathbf{u}}_u(0)), \quad \mathbf{r}_v^0 := \rho(\mathbf{v}_0 - \tilde{\mathbf{u}}_v(0)).$$

The residual  $R_{\text{CGM}}(\cdot)$  is written in equation (53) for the particular case of pure elasticity ( $a_1 = a_2 = 0$ ) following reference [35].

An alternative format of the residual is derived integrating by parts the term  $a(\tilde{\mathbf{u}}_u, \mathbf{w}_v)$  in (53). Thus, the strong residuals associated with the interior of the elements and the element boundaries (edges or faces) are introduced

$$\mathbf{r}_v^{\text{el}} := \mathbf{f} - \rho \dot{\tilde{\mathbf{u}}}_v + \nabla \cdot \boldsymbol{\sigma}(\tilde{\mathbf{u}}_u), \quad \text{in } \Omega_k, k = 1, \dots, N_{\text{el}}, \quad \text{and} \quad \mathbf{r}_v^{\text{fa}} := \begin{cases} -\frac{1}{2} \llbracket \boldsymbol{\sigma}(\tilde{\mathbf{u}}_u) \cdot \mathbf{n} \rrbracket & \text{on } \Gamma^{\text{int}}, \\ \mathbf{g} - \boldsymbol{\sigma}(\tilde{\mathbf{u}}_u) \cdot \mathbf{n} & \text{on } \Gamma_N, \end{cases}$$

being  $\Gamma^{\text{int}}$  the set of interelement boundaries (mesh edges or faces). The jump  $\llbracket \boldsymbol{\sigma} \cdot \mathbf{n} \rrbracket$  is defined on a generic element interface  $\Gamma_l = \partial \Omega_k \cap \partial \Omega_{k'} \in \Gamma^{\text{int}}$  as  $\llbracket \boldsymbol{\sigma} \cdot \mathbf{n} \rrbracket := \boldsymbol{\sigma}|_{\Omega_k} \cdot \mathbf{n}_k + \boldsymbol{\sigma}|_{\Omega_{k'}} \cdot \mathbf{n}_{k'}$  where  $\mathbf{n}_k$  and  $\mathbf{n}_{k'}$  are the outward unit normals to  $\partial \Omega_k$  and  $\partial \Omega_{k'}$  respectively. Equation (53) is therefore rewritten as

$$\begin{aligned} R_{\text{CGM}}(\mathbf{W}) &= \sum_{n=1}^N \sum_{k=1}^{N_{\text{el}}} \int_{I_n} [(\mathbf{r}_u, \mathbf{w}_u)_{\Omega_k} + (\mathbf{r}_v^{\text{el}}, \mathbf{w}_v)_{\Omega_k} + (\mathbf{r}_v^{\text{fa}}, \mathbf{w}_v)_{\partial \Omega_k}] \\ &\quad + \sum_{k=1}^{N_{\text{el}}} [(\mathbf{r}_u^0, \mathbf{w}_u(0))_{\Omega_k} + (\mathbf{r}_v^0, \mathbf{w}_v(0))_{\Omega_k}], \end{aligned} \quad (54)$$

where functions  $\mathbf{r}_u$ ,  $\mathbf{r}_v^{\text{el}}$ ,  $\mathbf{r}_v^{\text{fa}}$ ,  $\mathbf{r}_u^0$  and  $\mathbf{r}_v^0$  are the computable strong residuals contributing to  $R_{\text{CGM}}(\cdot)$ . This new format of the residual is interesting because, when restricted to elements and time slabs, it provides better space-time local indicators than the original one (53).

The local (element by element) error contributions associated with the space and time discretization errors are obtained using the residual decomposition (54) in the error

representation (52):

$$\begin{aligned}
 L_D^{\mathcal{O}}(\mathbf{E}) &= \sum_{k=1}^{N_{\text{el}}} [(\mathbf{r}_u^0, \mathbf{u}_u^{\text{d}}(0) - \mathbf{\Pi}^{\Delta t} \mathbf{\Pi}^H \mathbf{u}_u^{\text{d}}(0))_{\Omega_k} + (\mathbf{r}_v^0, \mathbf{u}_v^{\text{d}}(0) - \mathbf{\Pi}^{\Delta t} \mathbf{\Pi}^H \mathbf{u}_v^{\text{d}}(0))] \\
 &+ \sum_{n=1}^N \sum_{k=1}^{N_{\text{el}}} \int_{I_n} [(\mathbf{r}_u - \mathbf{\Pi}^{\Delta t} \mathbf{r}_u, \mathbf{u}_u^{\text{d}} - \mathbf{\Pi}^{\Delta t} \mathbf{u}_u^{\text{d}})_{\Omega_k} + (\mathbf{r}_u, \mathbf{\Pi}^{\Delta t} \mathbf{u}_u^{\text{d}} - \mathbf{\Pi}^{\Delta t} \mathbf{\Pi}^H \mathbf{u}_u^{\text{d}})_{\Omega_k}] \\
 &+ \sum_{n=1}^N \sum_{k=1}^{N_{\text{el}}} \int_{I_n} [(\mathbf{r}_v^{\text{el}} - \mathbf{\Pi}^{\Delta t} \mathbf{r}_v^{\text{el}}, \mathbf{u}_v^{\text{d}} - \mathbf{\Pi}^{\Delta t} \mathbf{u}_v^{\text{d}})_{\Omega_k} + (\mathbf{r}_v^{\text{el}}, \mathbf{\Pi}^{\Delta t} \mathbf{u}_v^{\text{d}} - \mathbf{\Pi}^{\Delta t} \mathbf{\Pi}^H \mathbf{u}_v^{\text{d}})_{\Omega_k}] \\
 &+ \sum_{n=1}^N \sum_{k=1}^{N_{\text{el}}} \int_{I_n} [(\mathbf{r}_v^{\text{fa}} - \mathbf{\Pi}^{\Delta t} \mathbf{r}_v^{\text{fa}}, \mathbf{u}_v^{\text{d}} - \mathbf{\Pi}^{\Delta t} \mathbf{u}_v^{\text{d}})_{\partial\Omega_k} + (\mathbf{r}_v^{\text{fa}}, \mathbf{\Pi}^{\Delta t} \mathbf{u}_v^{\text{d}} - \mathbf{\Pi}^{\Delta t} \mathbf{\Pi}^H \mathbf{u}_v^{\text{d}})_{\partial\Omega_k}], \tag{55}
 \end{aligned}$$

where functions  $\mathbf{\Pi}^{\Delta t} \mathbf{r}_u$ ,  $\mathbf{\Pi}^{\Delta t} \mathbf{r}_v^{\text{el}}$  and  $\mathbf{\Pi}^{\Delta t} \mathbf{r}_v^{\text{fa}}$  are introduced into equation (55) using the orthogonality property (50).

Equation (55) leads to local error contributions which might have opposite signs from element to element. In practice, the remeshing criteria, which translate the local error contributions into a desired element size, require that the input local indicators are positive. The positive error contributions are obtained using the Cauchy–Schwartz inequality in equation (55), namely

$$|L_D^{\mathcal{O}}(\mathbf{E})| \leq \sum_{k=1}^{N_{\text{el}}} \eta_k^{\text{dwr,i}} + \sum_{n=0}^N \sum_{k=1}^{N_{\text{el}}} [\eta_{n,k}^{\text{dwr,s}} + \eta_{n,k}^{\text{dwr,t}}], \tag{56}$$

with

$$\eta_k^{\text{dwr,i}} := \|\mathbf{r}_u^0\|_{\Omega_k} \|\mathbf{u}_u^{\text{d}}(0) - \mathbf{\Pi}^{\Delta t} \mathbf{\Pi}^H \mathbf{u}_u^{\text{d}}(0)\|_{\Omega_k} + \|\mathbf{r}_v^0\|_{\Omega_k} \|\mathbf{u}_v^{\text{d}}(0) - \mathbf{\Pi}^{\Delta t} \mathbf{\Pi}^H \mathbf{u}_v^{\text{d}}(0)\|_{\Omega_k}, \tag{57a}$$

$$\eta_{n,k}^{\text{dwr,t}} := \|\mathbf{r}_u - \mathbf{\Pi}^{\Delta t} \mathbf{r}_u\|_{\Omega_k \times I_n} \|\mathbf{u}_u^{\text{d}} - \mathbf{\Pi}^{\Delta t} \mathbf{u}_u^{\text{d}}\|_{\Omega_k \times I_n} + \|\mathbf{r}_v^{\text{el}} - \mathbf{\Pi}^{\Delta t} \mathbf{r}_v^{\text{el}}\|_{\Omega_k \times I_n} \|\mathbf{u}_v^{\text{d}} - \mathbf{\Pi}^{\Delta t} \mathbf{u}_v^{\text{d}}\|_{\Omega_k \times I_n} \tag{57b}$$

$$+ \|\mathbf{r}_v^{\text{fa}} - \mathbf{\Pi}^{\Delta t} \mathbf{r}_v^{\text{fa}}\|_{\partial\Omega_k \times I_n} \|\mathbf{u}_v^{\text{d}} - \mathbf{\Pi}^{\Delta t} \mathbf{u}_v^{\text{d}}\|_{\partial\Omega_k \times I_n}, \tag{57c}$$

$$\eta_{n,k}^{\text{dwr,s}} := \|\mathbf{r}_u\|_{\Omega_k} \|\mathbf{\Pi}^{\Delta t} \mathbf{u}_u^{\text{d}} - \mathbf{\Pi}^{\Delta t} \mathbf{\Pi}^H \mathbf{u}_u^{\text{d}}\|_{\Omega_k \times I_n} + \|\mathbf{r}_v^{\text{el}}\|_{\Omega_k \times I_n} \|\mathbf{\Pi}^{\Delta t} \mathbf{u}_v^{\text{d}} - \mathbf{\Pi}^{\Delta t} \mathbf{\Pi}^H \mathbf{u}_v^{\text{d}}\|_{\Omega_k \times I_n} \tag{57d}$$

$$+ \|\mathbf{r}_v^{\text{fa}}\|_{\partial\Omega_k \times I_n} \|\mathbf{\Pi}^{\Delta t} \mathbf{u}_v^{\text{d}} - \mathbf{\Pi}^{\Delta t} \mathbf{\Pi}^H \mathbf{u}_v^{\text{d}}\|_{\partial\Omega_k \times I_n}, \tag{57e}$$

where the notation  $\|\cdot\|_{\Omega_k}$ ,  $\|\cdot\|_{\Omega_k \times I_n}$  and  $\|\cdot\|_{\partial\Omega_k \times I_n}$  is used to denote the  $L^2$  norms in  $\Omega_k$ ,  $\Omega_k \times I_n$  and  $\partial\Omega_k \times I_n$  respectively. Note that 1)  $\eta_k^{\text{dwr,i}}$  is the contribution of element  $\Omega_k$  to the interpolation error due to the initial conditions, 2)  $\eta_{n,k}^{\text{dwr,t}}$  is the contribution of the space-time slab  $\Omega_k \times I_n$  to the time discretization error and 3)  $\eta_{n,k}^{\text{dwr,s}}$  is the contribution of the space-time slab  $\Omega_k \times I_n$  to the space discretization error.

The inequality in equation (56) guarantees that the error in the quantity of interest is controlled if the local errors  $\eta_k^{\text{dwr,i}}$ ,  $\eta_{n,k}^{\text{dwr,s}}$ ,  $\eta_{n,k}^{\text{dwr,t}}$  are small enough. Thus, the local error indicators  $\eta_k^{\text{dwr,i}}$ ,  $\eta_{n,k}^{\text{dwr,s}}$ ,  $\eta_{n,k}^{\text{dwr,t}}$  are useful to drive goal-oriented adaptive procedures aiming

at efficiently controlling  $|L^\mathcal{O}(\mathbf{e})|$ . However, these local error indicators cannot be used to obtain a reliable assessment of  $L^\mathcal{O}(\mathbf{e})$ , which is better estimated directly with equation (49).

Note that the local error contributions in equation (57) are not fully computable. They involve the norms of the strong residuals which are computable, but they also involve the unknown *exact* solution of the adjoint problem,  $\mathbf{U}^d$ . Computable local error indicators from equation (57) are obtained following two alternative approaches.

On the one hand, the exact adjoint solution  $\mathbf{U}^d$  is replaced in (57) by a suitable approximation  $\tilde{\mathbf{U}}^d$  as previously done in equation (49). The approximation  $\tilde{\mathbf{U}}^d$  must belong to a richer space than  $\widehat{\mathcal{W}}^{H,\Delta t} \times \widehat{\mathcal{W}}^{H,\Delta t}$  in order to preclude Galerkin cancellation. Function  $\tilde{\mathbf{U}}^d$  is computed in [64, 35] as post-process of the numerical adjoint approximation using recovery techniques. Alternatively, references [35, 47] compute  $\tilde{\mathbf{U}}^d$  solving a global problem in a richer space obtained with  $H$ - or  $p$ -refinement. This second approach might be computationally unaffordable in three-dimensional demanding problems.

On the other hand, computable local indicators are obtained introducing a priori error estimates for the adjoint interpolation errors  $\mathbf{U}^d - \mathbf{\Pi}^{\Delta t} \mathbf{\Pi}^H \mathbf{U}^d$  and  $\mathbf{U}^d - \mathbf{\Pi}^{\Delta t} \mathbf{U}^d$  appearing in equation (57). This allows to write the adjoint interpolation error in terms of higher order derivatives (both in space and time) of  $\mathbf{U}^d$ . Then, the unknown high order derivatives of  $\mathbf{U}^d$  are replaced by a post-process of the computed adjoint approximation, see [31, 35] for details. The use of *a priori* interpolation estimates introduces unknown constants in the final expression of the estimate. However, the local information given by the computable part of the estimate is used to perform space-time mesh adaptivity.

## 5.2 An $L^2$ -norm explicit estimate

This section briefly summarizes reference [50] deriving explicit estimates for the  $L^2$ -norm of the final displacement error,  $\|\mathbf{e}_u(T)\| = (\mathbf{e}_u(T), \mathbf{e}_u(T))^{1/2}$ . An analogous rationale holds for assessing the total energy of the error,  $\|\mathbf{e}_v(T)\|_m^2 + \|\mathbf{e}_u(T)\|_a^2$ , see references [34, 53].

Here, the  $L^2$ -norm of the error is seen as a particular quantity of interest to be estimated using the DWR approach presented in section 5.1. The non-linear character of this quantity of interest induces a corresponding functional output  $L_D^\mathcal{O}(\cdot)$  involving the unknown error  $\mathbf{e}_u(T)$ . This functional output induces an adjoint problem that plays a role in the derivation of the estimate. However, it is worth mentioning that the estimate is explicit and it does not require solving any adjoint problem.

The presentation in reference [50] considers the wave equation as model problem (with a scalar unknown). The equations of structural dynamics (1) are a general framework (with vectorial unknown) for a second order hyperbolic problem, that are seen as a generalization of the wave equation. Thus, the concepts introduced in [50] are presented here in the framework of problem (1) for the sake of a unified exposition.

Following [50], the Dirichlet boundary conditions are defined on the whole boundary,  $\Gamma_D = \partial\Omega$ , the density is taken  $\rho = 1$  and the initial conditions are assumed to be exactly represented by the numerical approximation, that is  $\mathbf{u}_0 - \tilde{\mathbf{u}}_u(0) = \mathbf{0}$  and  $\mathbf{v}_0 - \tilde{\mathbf{u}}_v(0) = \mathbf{0}$ .

The global  $L^2$ -norm of the final displacement error,  $\|\mathbf{e}_u(T)\|$ , is assessed introducing the auxiliary quantity of interest

$$L_D^{\mathcal{O}}(\mathbf{W}) := (\mathbf{e}_u(T), \mathbf{w}_u(T)). \quad (58)$$

Note that, with this definition, the error in the quantity of interest is indeed the  $L^2$ -norm of the final displacement error,

$$L_D^{\mathcal{O}}(\mathbf{E}) = (\mathbf{e}_u(T), \mathbf{e}_u(T)) = \|\mathbf{e}_u(T)\|^2.$$

Assuming that  $\mathbf{U}^d$  is the solution of the adjoint problem (34) associated with the quantity of interest (58), the value  $\|\mathbf{e}_u(T)\|$  can be expressed using the error representation (41). That is

$$\|\mathbf{e}_u(T)\|^2 = L_D^{\mathcal{O}}(\mathbf{E}) = R_{\text{CGA}}(\mathbf{U}^d - \mathbf{\Pi}^{\Delta t} \mathbf{\Pi}^H \mathbf{U}^d), \quad (59)$$

where  $\mathbf{W}^{H, \Delta t}$  is replaced in (41) by the projection of the adjoint solution  $\mathbf{\Pi}^{\Delta t} \mathbf{\Pi}^H \mathbf{U}^d$ . The error representation (59) involves the residual  $R_{\text{CGA}}(\cdot)$  instead of  $R_{\text{CGM}}(\cdot)$  because the numerical approximation  $\tilde{\mathbf{U}}$  is computed in reference [50] with the discrete CGA problem (14), and consequently, the orthogonality property holds only for  $R_{\text{CGA}}(\cdot)$ .

The space and time errors are separated adding and subtracting the projection  $\mathbf{\Pi}^H \mathbf{U}^d$  into equation (59),

$$L_D^{\mathcal{O}}(\mathbf{E}) = R_{\text{CGA}}(\mathbf{U}^d - \mathbf{\Pi}^H \mathbf{U}^d) + R_{\text{CGA}}(\mathbf{\Pi}^H (\mathbf{U}^d - \mathbf{\Pi}^{\Delta t} \mathbf{U}^d)), \quad (60)$$

where the terms  $R_{\text{CGA}}(\mathbf{U}^d - \mathbf{\Pi}^H \mathbf{U}^d)$  and  $R_{\text{CGA}}(\mathbf{\Pi}^H (\mathbf{U}^d - \mathbf{\Pi}^{\Delta t} \mathbf{U}^d))$  are related with the space and time discretization errors respectively. The space projection operator  $\mathbf{\Pi}^H$  is defined here as the  $L^2$ -projection in the space  $\mathcal{V}_0^H$  instead of the usual nodal interpolation, see reference [50]. This technicality is required to ensure some orthogonality properties. That is,  $\mathbf{\Pi}^H \mathbf{w}$  is defined for a generic function  $\mathbf{w} \in \mathcal{V}_0$  as the solution of the problem: find  $\mathbf{\Pi}^H \mathbf{w} \in \mathcal{V}_0^H$  such that

$$(\mathbf{\Pi}^H \mathbf{w}, \mathbf{v}) = (\mathbf{w}, \mathbf{v}), \quad \forall \mathbf{v} \in \mathcal{V}_0^H.$$

The derivation of the explicit error estimate is split into three conceptual steps: 1) equation (60) is rewritten using the orthogonality properties of the operators  $\mathbf{\Pi}^H$  and  $\mathbf{\Pi}^{\Delta t}$  and integrating by parts in time, 2) the adjoint interpolation errors are expressed in terms of high order derivatives of the adjoint solution using *a priori* error estimates and 3) the resulting high order derivatives are bounded using a stability property of the adjoint solution. These steps are detailed below.

First, using the orthogonality properties of the operators  $\mathbf{\Pi}^H$  and  $\mathbf{\Pi}^{\Delta t}$ , equation (60) is rewritten as

$$\begin{aligned} L_D^{\mathcal{O}}(\mathbf{E}) &= \int_I (\mathbf{f} - \mathbf{\Pi}^H \mathbf{f}, \dot{\mathbf{u}}^d - \mathbf{\Pi}^H \dot{\mathbf{u}}^d) dt + \int_I (\mathbf{f} - \mathbf{\Pi}^{\Delta t} \mathbf{f}, \mathbf{\Pi}^H (\dot{\mathbf{u}}^d - \mathbf{\Pi}^{\Delta t} \dot{\mathbf{u}}^d)) dt \\ &\quad - \int_I a(\tilde{\mathbf{u}}_u, \dot{\mathbf{u}}^d - \mathbf{\Pi}^H \dot{\mathbf{u}}^d) dt - \int_I a(\tilde{\mathbf{u}}_u - \mathbf{\Pi}^{\Delta t} \tilde{\mathbf{u}}_u, \mathbf{\Pi}^H (\dot{\mathbf{u}}^d - \mathbf{\Pi}^{\Delta t} \dot{\mathbf{u}}^d)) dt \\ &\quad - \int_I a(\dot{\tilde{\mathbf{u}}}_u - \tilde{\mathbf{u}}_v, \mathbf{u}^d - \mathbf{\Pi}^H \mathbf{u}^d) dt + \int_I a(\tilde{\mathbf{u}}_v - \mathbf{\Pi}^{\Delta t} \tilde{\mathbf{u}}_v, \mathbf{\Pi}^H (\mathbf{u}^d - \mathbf{\Pi}^{\Delta t} \mathbf{u}^d)) dt. \end{aligned} \quad (61)$$

Then, integrating by parts the time integrals in the term

$$\int_I a(\tilde{\mathbf{u}}_u, \dot{\mathbf{u}}^d - \Pi^H \dot{\mathbf{u}}^d) dt,$$

equation (61) yields

$$\begin{aligned} L_D^{\mathcal{O}}(\mathbf{E}) &= \int_I (\mathbf{f} - \Pi^H \mathbf{f}, \dot{\mathbf{u}}^d - \Pi^H \dot{\mathbf{u}}^d) dt + \int_I (\mathbf{f} - \Pi^{\Delta t} \mathbf{f}, \Pi^H (\dot{\mathbf{u}}^d - \Pi^{\Delta t} \dot{\mathbf{u}}^d)) dt \\ &\quad - \int_I a(\tilde{\mathbf{u}}_u - \Pi^{\Delta t} \tilde{\mathbf{u}}_u, \Pi^H (\dot{\mathbf{u}}^d - \Pi^{\Delta t} \dot{\mathbf{u}}^d)) dt \\ &\quad + \int_I a(\tilde{\mathbf{u}}_v, \mathbf{u}^d - \Pi^H \mathbf{u}^d) dt + \int_I a(\tilde{\mathbf{u}}_v - \Pi^{\Delta t} \tilde{\mathbf{u}}_v, \Pi^H (\mathbf{u}^d - \Pi^{\Delta t} \mathbf{u}^d)) dt \\ &\quad - a(\tilde{\mathbf{u}}_u(T), \mathbf{u}^d(T) - \Pi^H \mathbf{u}^d(T)) + a(\tilde{\mathbf{u}}_u(0), \mathbf{u}^d(0) - \Pi^H \mathbf{u}^d(0)). \end{aligned} \quad (62)$$

Second, previous equation is rewritten using a priori error estimates for the interpolation errors  $\mathbf{u}^d - \Pi^H \mathbf{u}^d$  and  $\mathbf{u}^d - \Pi^{\Delta t} \mathbf{u}^d$ , see reference [50] for details:

$$\begin{aligned} |L_D^{\mathcal{O}}(\mathbf{E})| &\leq C_1 \left( \max_{t \in I} \|\dot{\mathbf{u}}^d(t)\| \right) \left( \int_I \Delta t \|\mathbf{f} - \Pi^{\Delta t} \mathbf{f}\| dt + \int_I \Delta t \|\Delta_H (\tilde{\mathbf{u}}_u - \Pi^{\Delta t} \tilde{\mathbf{u}}_u)\| dt \right) \\ &\quad + C_2 \left( \max_{t \in I} \|\dot{\mathbf{u}}^d(t)\|_a \right) \left( \int_I \|H(\mathbf{f} - \Pi^H \mathbf{f})\| dt + \int_I \Delta t \|\tilde{\mathbf{u}}_v - \Pi^{\Delta t} \tilde{\mathbf{u}}_v\|_a dt \right) \\ &\quad + C_3 \left( \max_{t \in I} \|\nabla \cdot \boldsymbol{\sigma}^d(\mathbf{u}^d(t))\| \right) \left( \int_I \|H^2 R_2(\tilde{\mathbf{u}}_v)\| dt + \|H^2 R_2(\tilde{\mathbf{u}}_v(T))\| + \|H^2 R_2(\tilde{\mathbf{u}}_v(0))\| \right) \end{aligned} \quad (63)$$

where the discrete laplacian operator  $\Delta_H$  is defined for a generic function  $\mathbf{w} \in \mathcal{V}_0$  as : find  $\Delta_H \mathbf{w} \in \mathcal{V}_0^H$  such that

$$(\Delta_H \mathbf{w}, \mathbf{v}) = a(\mathbf{w}, \mathbf{v}), \quad \forall \mathbf{w} \in \mathcal{V}_0^H,$$

and the operator  $R_2(\cdot)$  is defined for a generic function  $\mathbf{w} \in \mathcal{V}_0^H$  as

$$R_2(\mathbf{w})|_{\Omega_k} := \frac{1}{2H_k} \max_{\mathbf{x} \in \partial\Omega_k} \left| \llbracket \mathbf{s}^E(\mathbf{w}(\mathbf{x})) \cdot \mathbf{n}(\mathbf{x}) \rrbracket \right|.$$

Third, the factors in (63) involving the adjoint solution are bounded in terms of the error  $\|\mathbf{e}_u(T)\|$  using the following stability property of the adjoint solution  $\mathbf{u}^d$  associated with the quantity of interest (58).

**Theorem 1.** *The solution  $\mathbf{u}^d$  of the adjoint problem (34) for quantity defined in (58), which strong form is*

$$\rho \ddot{\mathbf{u}}^d - \nabla \cdot \boldsymbol{\sigma}^d(\mathbf{u}^d) = \mathbf{0} \quad \text{in } \Omega \times I, \quad (64a)$$

$$\mathbf{u}^d = \mathbf{0} \quad \text{on } \Gamma_D \times I, \quad (64b)$$

$$\boldsymbol{\sigma}^d \cdot \mathbf{n} = \mathbf{0} \quad \text{on } \Gamma_N \times I, \quad (64c)$$

$$-\nabla \cdot \boldsymbol{\sigma}^d(\mathbf{u}^d) = \mathbf{e}_u \quad \text{at } \Omega \times \{T\}, \quad (64d)$$

$$\dot{\mathbf{u}}^d = \mathbf{0} \quad \text{at } \Omega \times \{T\}, \quad (64e)$$

fulfills

$$\max_{t \in I} (\|\ddot{\mathbf{u}}^d(t)\|^2 + \|\dot{\mathbf{u}}^d(t)\|_a^2 + \|\nabla \cdot \boldsymbol{\sigma}(\mathbf{u}^d(t))\|^2) \leq C \|\mathbf{e}_u(T)\|^2. \quad (65)$$

The proof can be found in [50, Lemma 17.3].

Using the stability property (65) and recalling that  $L_D^{\mathcal{O}}(\mathbf{E}) = \|\mathbf{e}_u(T)\|^2$ , equation (63) yields

$$\begin{aligned} \|\mathbf{e}_u(T)\| \leq C & \left( \int_I \Delta t \|\mathbf{f} - \boldsymbol{\Pi}^{\Delta t} \mathbf{f}\| \, dt + \int_I \Delta t \|\boldsymbol{\Delta}_H(\tilde{\mathbf{u}}_u - \boldsymbol{\Pi}^{\Delta t} \tilde{\mathbf{u}}_u)\| \, dt \right. \\ & + \int_I \Delta t \|\tilde{\mathbf{u}}_v - \boldsymbol{\Pi}^{\Delta t} \tilde{\mathbf{u}}_v\|_a \, dt + \int_I \|H(\mathbf{f} - \boldsymbol{\Pi}^H \mathbf{f})\| \, dt \\ & \left. + \int_I \|H^2 R_2(\tilde{\mathbf{u}}_v)\| \, dt + \|H^2 R_2(\tilde{\mathbf{u}}_u(T))\| + \|H^2 R_2(\tilde{\mathbf{u}}_u(0))\| \right). \quad (66) \end{aligned}$$

Constant  $C$  in the previous expression is unknown. Nevertheless the computable part of the error estimate is split into space-time local contributions providing information on the relative magnitude of the error generated at each region of the computational domain. This information can be used for adaptive purposes. The three first terms in the right hand side of equation (66) are associated with the time discretization error while the four last terms are associated with the space discretization. The local contributions associated with these terms are used to adapt the corresponding space or time discretization.

## 6 Constitutive relation error and implicit estimates

This section aims at computing bounds for the error in the dissipation norm  $\|\mathbf{e}\|$  and in the quantity of interest  $L^{\mathcal{O}}(\mathbf{e})$  using the so-called constitutive relation error estimates [9]. These estimates require an underlying stress equilibration technique based on solving local problems. The residual is playing the role of the loading of the local problems, and therefore the solution is not an explicit post-process of the residual. Thus, these strategies are also denoted as implicit residual type estimates. In many contexts, constitutive relation error estimates and implicit residual type estimates are fully equivalent.

The goal is to compute scalar values  $\eta_{\text{ener}}$ ,  $\eta_L$  and  $\eta_U$  such that

$$\|\mathbf{e}\| \leq \eta_{\text{ener}} \quad \text{and} \quad \eta_L \leq L^{\mathcal{O}}(\mathbf{e}) \leq \eta_U. \quad (67)$$

Deriving the error bounds  $\eta_{\text{ener}}$ ,  $\eta_L$  and  $\eta_U$  using constitutive relation estimates requires that problem (1) contains some damping (i.e. either  $a_1$  or  $a_2$  is different from zero). This means that the bounding properties of the estimate are lost in the limit case of pure elasticity ( $a_1 = a_2 = 0$ ). Non-zero damping allows computing the error bounds (67) following a rationale analogous to the one used in steady-state problems [9].

The technique providing  $\eta_{\text{ener}}$ ,  $\eta_L$  and  $\eta_U$  is presented here following reference [46], where the model problem under consideration corresponds to taking  $a_1 = 0$  and  $a_2 > 0$  in equations (1). Thus, in the remaining of this section, the coefficient  $a_1$  is assumed to be

zero and  $a_2$  is assumed to be strictly positive. The same rationale holds for other damped versions of problem (1). For instance, references [44, 42, 45] introduce damping in the constitutive relation (2) using the linear Maxwell viscous model.

## 6.1 Computable upper bounds for the dissipation norm

The key ingredient to compute error bounds is building a pair  $(\tilde{\boldsymbol{\sigma}}, \tilde{\mathbf{u}})$  of a *dynamically admissible* (D-admissible) stress and a *kinematically admissible* (K-admissible) displacement.

On the one hand, the set of K-admissible displacements is defined as

$$\mathcal{U} := \{\mathbf{w} \in \mathcal{W} : \mathbf{w} = \mathbf{u}_0 \text{ at } \Omega \times \{0\} \text{ and } \dot{\mathbf{w}} = \mathbf{v}_0 \text{ at } \Omega \times \{0\}\}.$$

Functions in  $\mathcal{U}$  are continuous with continuous time derivative and exactly fulfilling both the homogeneous Dirichlet condition (1b) and the initial conditions (1d) and (1e). On the other hand, the space of D-admissible stresses is defined for a given K-admissible displacement  $\tilde{\mathbf{u}} \in \mathcal{U}$  as

$$\mathcal{S}(\tilde{\mathbf{u}}) := \left\{ \boldsymbol{\tau} \in \mathcal{Z} : \int_I (\boldsymbol{\tau}, \boldsymbol{\varepsilon}(\dot{\mathbf{w}})) \, dt = \int_I (l(t; \dot{\mathbf{w}}) - (\rho \ddot{\tilde{\mathbf{u}}}, \dot{\mathbf{w}})) \, dt \quad \forall \mathbf{w} \in \mathcal{W} \right\}, \quad (68)$$

where

$$\mathcal{Z} := \{\boldsymbol{\tau} : [\boldsymbol{\tau}]_{ij} \in L^2(\Omega \times I) \quad i, j \leq d\}, \quad (69)$$

and for  $\boldsymbol{\tau}, \boldsymbol{\varepsilon} \in \mathcal{Z}$ , the standard  $L^2$  product in  $\Omega$  reads

$$(\boldsymbol{\tau}, \boldsymbol{\varepsilon}) := \int_{\Omega} \boldsymbol{\tau} : \boldsymbol{\varepsilon} \, d\Omega.$$

The stress tensors in  $\mathcal{S}(\tilde{\mathbf{u}})$  are in dynamic equilibrium with respect the external loads plus the inertia forces associated with  $\ddot{\tilde{\mathbf{u}}}$ . For that reason, the definition of (and the notation for) the set  $\mathcal{S}(\tilde{\mathbf{u}})$  depends on the K-admissible displacement  $\tilde{\mathbf{u}}$ . A stress tensor  $\tilde{\boldsymbol{\sigma}} \in \mathcal{S}(\tilde{\mathbf{u}})$  is generally discontinuous between mesh elements, while the traction vector  $\tilde{\boldsymbol{\sigma}} \cdot \mathbf{n}$  is continuous across element edges (or faces in 3D). The weak form of the dynamic equilibrium is implicitly stated in the definition of  $\mathcal{S}(\tilde{\mathbf{u}})$  given in (68). The equivalent strong formulation for D-admissibility enforces point wise equilibrium in the interior of the elements and traction continuity across the element interfaces. Thus, for a given finite element mesh, a D-admissible stress  $\tilde{\boldsymbol{\sigma}}$  fulfills

$$\begin{aligned} -\nabla \cdot \tilde{\boldsymbol{\sigma}} &= \mathbf{f} - \rho \ddot{\tilde{\mathbf{u}}} \quad \text{on } \Omega^{\text{int}} \times I, \\ \tilde{\boldsymbol{\sigma}} \cdot \mathbf{n} &= \mathbf{g} \quad \text{on } \Gamma_{\text{N}} \times I, \\ \llbracket \tilde{\boldsymbol{\sigma}} \cdot \mathbf{n} \rrbracket &= \mathbf{0} \quad \text{on } \Gamma^{\text{int}} \times I, \end{aligned}$$

where  $\Omega^{\text{int}}$  is the interior of the elements of the mesh and  $\Gamma^{\text{int}}$  is the set of interelement faces (or edges in 2D).



The admissible pair  $(\tilde{\boldsymbol{\sigma}}, \tilde{\mathbf{u}}) \in \mathcal{S}(\tilde{\mathbf{u}}) \times \mathcal{U}$  defines the following stress error

$$\tilde{\boldsymbol{\sigma}}^e := \tilde{\boldsymbol{\sigma}} - \boldsymbol{\sigma}(\tilde{\mathbf{u}}), \quad (70)$$

which corresponds to the non verification of the constitutive relation (2). The so-called *constitutive relation error* (following the terminology by Ladevèze and co-workers) is then computed as  $\|\tilde{\boldsymbol{\sigma}}^e\|_\sigma$  where  $\|\cdot\|_\sigma$  is the stress version of the space-time norm  $\|\cdot\|$  defined in (23), namely

$$\|\boldsymbol{\tau}\|_\sigma^2 := \frac{1}{a_2} \int_I \|\boldsymbol{\tau}\|_a^2 dt.$$

For the particular case  $a_1 = 0$ , the displacement and stress norms are related by  $\|\mathbf{w}\| = \|a_2 \mathbf{C} : \boldsymbol{\varepsilon}(\dot{\mathbf{w}})\|_\sigma$ .

The constitutive relation error  $\|\tilde{\boldsymbol{\sigma}}^e\|_\sigma$  is computable once the fields  $\tilde{\boldsymbol{\sigma}}$  and  $\tilde{\mathbf{u}}$  are available. Note that,  $\|\tilde{\boldsymbol{\sigma}}^e\|_\sigma = 0$  if and only if  $\tilde{\boldsymbol{\sigma}} = \boldsymbol{\sigma}$  and  $\tilde{\mathbf{u}} = \mathbf{u}$ . Consequently,  $\|\tilde{\boldsymbol{\sigma}}^e\|_\sigma$  is adopted as a pertinent error measure. Moreover, the value  $\|\tilde{\boldsymbol{\sigma}}^e\|_\sigma$  provides information about the unknown error  $\mathbf{e}$ , as shown by the following theorem. For the sake of simplifying, the operators identifying the elastic and viscous contributions of the constitutive law are introduced as  $\mathbf{s}^E(\mathbf{u}) := \mathbf{C} : \boldsymbol{\varepsilon}(\mathbf{u})$  and  $\mathbf{s}^\nu(\mathbf{u}) := a_2 \mathbf{C} : \boldsymbol{\varepsilon}(\dot{\mathbf{u}})$ .

**Theorem 2.** *Given an admissible pair  $(\tilde{\boldsymbol{\sigma}}, \tilde{\mathbf{u}}) \in \mathcal{S}(\tilde{\mathbf{u}}) \times \mathcal{U}$ , the errors  $\mathbf{e}$  and  $\tilde{\boldsymbol{\sigma}}^e$  defined in equations (17) and (70), respectively, fulfill*

$$\|\tilde{\boldsymbol{\sigma}}^e\|_\sigma^2 = \|\dot{\mathbf{e}}(T)\|_m^2 + \|\mathbf{e}(T)\|_a^2 + \|\mathbf{e}\|^2 + \|\boldsymbol{\sigma}^\nu - \tilde{\boldsymbol{\sigma}}^\nu\|_\sigma^2, \quad (71)$$

where  $\boldsymbol{\sigma}^\nu := \mathbf{s}^\nu(\mathbf{u})$  and  $\tilde{\boldsymbol{\sigma}}^\nu := \tilde{\boldsymbol{\sigma}} - \mathbf{s}^E(\tilde{\mathbf{u}})$ .

For the proof, the reader is referred to [46, Theorem 1].

A direct consequence of theorem 2 is the relation  $\|\tilde{\boldsymbol{\sigma}}^e\|_\sigma^2 \geq \|\dot{\mathbf{e}}(T)\|_m^2 + \|\mathbf{e}(T)\|_a^2 + \|\mathbf{e}\|^2$  and, in particular, the following upper bound

$$\eta_{\text{ener}} := \|\tilde{\boldsymbol{\sigma}}^e\|_\sigma \geq \|\mathbf{e}\|. \quad (72)$$

Moreover, expression (72) is particularly important because it is used to bound the quantity of interest.

## 6.2 Error bounds in the quantity of interest

Bounds of the error in the quantity of interest  $L^{\mathcal{O}}(\mathbf{e})$  are obtained combining admissible pairs for both the original and the adjoint problems,  $(\tilde{\mathbf{u}}, \tilde{\boldsymbol{\sigma}})$  and  $(\tilde{\mathbf{u}}^d, \tilde{\boldsymbol{\sigma}}^d)$ . The space of *adjoint kinematically admissible* displacements is defined as

$$\mathcal{U}^d := \{ \mathbf{w} \in \mathcal{W} : \mathbf{w} = \mathbf{u}^{\mathcal{O}} \text{ at } \Omega \times \{T\} \text{ and } \dot{\mathbf{w}} = \mathbf{v}^{\mathcal{O}} \text{ at } \Omega \times \{T\} \}.$$

The space of *adjoint dynamically admissible* stress fields is defined for a given  $\tilde{\mathbf{u}}^d \in \mathcal{U}^d$  as

$$\mathcal{S}^d(\tilde{\mathbf{u}}^d) := \left\{ \boldsymbol{\tau} \in \mathcal{Z} : \int_I (\boldsymbol{\tau}, \boldsymbol{\varepsilon}(\dot{\mathbf{w}})) dt = \int_I (l^{\mathcal{O}}(t; \dot{\mathbf{w}}) - (\rho \ddot{\tilde{\mathbf{u}}^d}, \dot{\mathbf{w}})) dt \quad \forall \mathbf{w} \in \mathcal{W} \right\},$$

where  $l^\mathcal{O}(t; \mathbf{w}) := (\mathbf{f}^\mathcal{O}(t), \mathbf{w}) + (\mathbf{g}^\mathcal{O}(t), \mathbf{w})_{\Gamma_N}$ .

The admissible pair  $(\tilde{\mathbf{u}}^d, \tilde{\boldsymbol{\sigma}}^d) \in \mathcal{U}^d \times \mathcal{S}^d(\tilde{\mathbf{u}}^d)$  provides the error in stresses associated with the non verification of the adjoint constitutive relation (27),

$$\tilde{\boldsymbol{\sigma}}^{d,e} := \tilde{\boldsymbol{\sigma}}^d - \boldsymbol{\sigma}^d(\tilde{\mathbf{u}}^d). \quad (73)$$

The bounds for  $L^\mathcal{O}(\mathbf{e})$  are computed using the constitutive relation errors  $\tilde{\boldsymbol{\sigma}}^e$  and  $\tilde{\boldsymbol{\sigma}}^{d,e}$ , as defined in (70) and (73). Actually,  $\tilde{\boldsymbol{\sigma}}^e$  and  $\tilde{\boldsymbol{\sigma}}^{d,e}$  are used to obtain bounds for  $R^d(\mathbf{e})$ , which is the non-computable part of the error representation (37). There are three different approaches to derive upper bounds on the basis of the constitutive relation errors, which are described in the following.

### 6.2.1 Error bounds based on the Cauchy-Schwarz inequality

References [42, 43, 44, 45] derive computable bounds using the Cauchy-Schwarz inequality. The bounds for  $R^d(\mathbf{e})$  are obtained noting that if  $(\tilde{\mathbf{u}}^d, \tilde{\boldsymbol{\sigma}}^d)$  is an adjoint admissible pair, then its associated error  $\tilde{\boldsymbol{\sigma}}^{d,e}$  verifies

$$\bar{B}^\nu(\tilde{\boldsymbol{\sigma}}^{d,e}, \mathbf{s}^\nu(\mathbf{w})) = R^d(\mathbf{w}) \quad \forall \mathbf{w} \in \mathcal{W}, \quad (74)$$

where  $\bar{B}^\nu(\cdot, \cdot)$  is the bilinear form

$$\bar{B}^\nu(\boldsymbol{\tau}_1, \boldsymbol{\tau}_2) := \frac{1}{a_2} \int_I \bar{a}(\boldsymbol{\tau}_1, \boldsymbol{\tau}_2) dt. \quad (75)$$

Note that the bilinear form  $\bar{B}^\nu(\cdot, \cdot)$  induces the stress energy norm  $\|\cdot\|_\sigma$ , that is  $\|\boldsymbol{\tau}\|_\sigma^2 := \bar{B}^\nu(\boldsymbol{\tau}, \boldsymbol{\tau})$ .

Taking  $\mathbf{w} = \mathbf{e}$ , equation (74) yields

$$R^d(\mathbf{e}) = \bar{B}^\nu(\tilde{\boldsymbol{\sigma}}^{d,e}, \mathbf{s}^\nu(\mathbf{e})).$$

Being  $\bar{B}^\nu$  symmetric-positive-definite, the Cauchy-Schwarz inequality holds and yields

$$|R^d(\mathbf{e})| \leq \|\tilde{\boldsymbol{\sigma}}^{d,e}\|_\sigma \|\mathbf{s}^\nu(\mathbf{e})\|_\sigma = \|\tilde{\boldsymbol{\sigma}}^{d,e}\|_\sigma \|\mathbf{e}\|.$$

The factor involving the unknown error  $\mathbf{e}$  is bounded using the equation (72) leading to the following computable bound for  $|R^d(\mathbf{e})|$ ,

$$|R^d(\mathbf{e})| \leq \|\tilde{\boldsymbol{\sigma}}^{d,e}\|_\sigma \|\tilde{\boldsymbol{\sigma}}^e\|_\sigma.$$

The computable bounds for the error in the quantity of interest are readily obtained using the previous result together with the computable part of the error representation (37). That is,

$$\zeta_L^{\text{C-S}} \leq L^\mathcal{O}(\mathbf{e}) \leq \zeta_U^{\text{C-S}},$$

where

$$\zeta_U^{\text{C-S}} := \|\tilde{\boldsymbol{\sigma}}^{d,e}\|_\sigma \|\tilde{\boldsymbol{\sigma}}^e\|_\sigma + R(\tilde{\mathbf{u}}^d), \quad (76a)$$

$$\zeta_L^{\text{C-S}} := -\|\tilde{\boldsymbol{\sigma}}^{d,e}\|_\sigma \|\tilde{\boldsymbol{\sigma}}^e\|_\sigma + R(\tilde{\mathbf{u}}^d). \quad (76b)$$

The use of the Cauchy-Schwarz inequality is typically inducing a large overestimation of the quantities assessed. This is because the two *vectors*  $\tilde{\boldsymbol{\sigma}}^{d,e}$  and  $\mathbf{s}^\nu(\mathbf{e})$  are, in general, far of being *parallel*. This makes the error bounds given in (76) not sharp, with an unrealistic and impractical bound gap.

### 6.2.2 Bounds using symmetric error equations

Alternative error bounds, based on different algebraic identities, are often used in the literature to derive sharper bounds than the ones obtained with the Cauchy-Schwarz approach. For instance, the parallelogram identity is used in [7, 65, 66] in the context of linear elasticity and in [21, 22] for transient convection-diffusion-reaction equations. In the framework of structural dynamics reference [46] proposes a bounding expression, alternative to the one in equation (76) originally proposed in reference [45].

The derivation of the alternative bounds requires introducing symmetrized equations for the original and adjoint errors. However, it is worth noting that this is only a mathematical artifact and, in practice, the error bounds are computed using only the admissible pairs  $(\tilde{\mathbf{u}}, \tilde{\boldsymbol{\sigma}})$  and  $(\tilde{\mathbf{u}}^d, \tilde{\boldsymbol{\sigma}}^d)$  without solving any auxiliary symmetrized error equations.

The symmetrized error equations read: find  $\mathbf{e}^\nu \in \mathbf{U}_0$  and  $\mathbf{e}^{d,\nu} \in \mathbf{U}_0^d$  such that

$$B^\nu(\mathbf{e}^\nu, \mathbf{w}) = R(\mathbf{w}) \quad \forall \mathbf{w} \in \mathcal{W}, \quad (77a)$$

$$B^\nu(\mathbf{e}^{d,\nu}, \mathbf{w}) = R^d(\mathbf{w}) \quad \forall \mathbf{w} \in \mathcal{W}, \quad (77b)$$

where the spaces  $\mathbf{U}_0$  and  $\mathbf{U}_0^d$  are defined respectively as

$$\mathbf{U}_0 := \{\mathbf{w} \in \mathcal{W} : \mathbf{w} = 0 \text{ at } \Omega \times \{0\} \text{ and } \dot{\mathbf{w}} = 0 \text{ at } \Omega \times \{0\}\},$$

and

$$\mathbf{U}_0^d := \{\mathbf{w} \in \mathcal{W} : \mathbf{w} = 0 \text{ at } \Omega \times \{T\} \text{ and } \dot{\mathbf{w}} = 0 \text{ at } \Omega \times \{T\}\}.$$

Equations (77) resemble the residual equation (18) for the primal error  $\mathbf{e}$ . Note that the difference is that the non symmetric bilinear form  $B(\cdot, \cdot)$  is replaced by the symmetric one  $B^\nu(\cdot, \cdot)$  defined as

$$B^\nu(\mathbf{v}, \mathbf{w}) := a_2 \int_I a(\dot{\mathbf{v}}, \dot{\mathbf{w}}) dt.$$

It is easily shown that for any scalar value  $\kappa \neq 0$ , see [46], the following algebraic identity holds:

$$-\frac{1}{4} \|\kappa \mathbf{e}^\nu - \frac{1}{\kappa} \mathbf{e}^{d,\nu}\|^2 \leq R^d(\mathbf{e}) \leq \frac{1}{4} \|\kappa \mathbf{e}^\nu + \frac{1}{\kappa} \mathbf{e}^{d,\nu}\|^2. \quad (78)$$

Functions  $\kappa \mathbf{e}^\nu \pm \frac{1}{\kappa} \mathbf{e}^{d,\nu}$  are solutions of the infinite dimensional problems (77). Therefore, the error bounds proposed in (78) are not computable. However, introducing the constitutive relation errors of the original and adjoint problem, the computable bounds for  $\|\kappa \mathbf{e}^\nu \pm \frac{1}{\kappa} \mathbf{e}^{d,\nu}\|$  are

$$\|\kappa \mathbf{e}^\nu \pm \frac{1}{\kappa} \mathbf{e}^{d,\nu}\| \leq \|\kappa \tilde{\boldsymbol{\sigma}}^e \pm \frac{1}{\kappa} \tilde{\boldsymbol{\sigma}}^{d,e}\|_\sigma. \quad (79)$$

The final bounds for  $L^{\mathcal{O}}(\mathbf{e})$  are derived substituting expression (79) in equation (78) and adding the correction term  $R(\tilde{\mathbf{u}}^d)$

$$\zeta_U := \frac{1}{4} \|\kappa \tilde{\boldsymbol{\sigma}}^e + \frac{1}{\kappa} \tilde{\boldsymbol{\sigma}}^{d,e}\|_\sigma^2 + R(\tilde{\mathbf{u}}^d), \quad (80a)$$

$$\zeta_L := -\frac{1}{4} \|\kappa \tilde{\boldsymbol{\sigma}}^e - \frac{1}{\kappa} \tilde{\boldsymbol{\sigma}}^{d,e}\|_\sigma^2 + R(\tilde{\mathbf{u}}^d), \quad (80b)$$

where  $\zeta_L$  and  $\zeta_U$  are such that

$$\zeta_L \leq L^{\mathcal{O}}(\mathbf{e}) \leq \zeta_U.$$

The error bounds (80) have a similar (but not identical) structure as the ones obtained using the parallelogram rule in linear elasticity. In both cases, the error in the quantity of interest is expressed in terms of energy measures of linear combinations of the original and adjoint errors. The main difference with respect to the parallelogram approach is that, here, energy-like lower bounds of the error are not used to obtain sharper bounds for the quantity of interest.

Note that the bounds (80) hold for any non-zero scalar parameter  $\kappa$ . In practice, the parameter  $\kappa$  is determined such that it minimizes the bound gap, yielding the optimal value

$$\kappa = \left( \frac{\|\tilde{\boldsymbol{\sigma}}^{\text{d,e}}\|_{\sigma}}{\|\tilde{\boldsymbol{\sigma}}^{\text{e}}\|_{\sigma}} \right)^{1/2}. \quad (81)$$

The error bounds  $\zeta_U, \zeta_L$  proposed in (80) are sharper than  $\zeta_U^{\text{C-S}}, \zeta_L^{\text{C-S}}$  in (76) obtained using the Cauchy-Schwartz inequality. Indeed, introducing the optimal value of  $\kappa$  given by (81) into the bound expression (80) yields

$$\zeta_U = \frac{1}{2} \|\tilde{\boldsymbol{\sigma}}^{\text{d,e}}\|_{\sigma} \|\tilde{\boldsymbol{\sigma}}^{\text{e}}\|_{\sigma} + R(\tilde{\mathbf{u}}^{\text{d}}) + \frac{1}{2} \bar{B}^{\nu}(\tilde{\boldsymbol{\sigma}}^{\text{d,e}}, \tilde{\boldsymbol{\sigma}}^{\text{e}}), \quad (82\text{a})$$

$$\zeta_L = -\frac{1}{2} \|\tilde{\boldsymbol{\sigma}}^{\text{d,e}}\|_{\sigma} \|\tilde{\boldsymbol{\sigma}}^{\text{e}}\|_{\sigma} + R(\tilde{\mathbf{u}}^{\text{d}}) + \frac{1}{2} \bar{B}^{\nu}(\tilde{\boldsymbol{\sigma}}^{\text{d,e}}, \tilde{\boldsymbol{\sigma}}^{\text{e}}). \quad (82\text{b})$$

The bound gap, that is the difference between the upper and lower bound, is therefore  $\zeta_U - \zeta_L = \|\tilde{\boldsymbol{\sigma}}^{\text{d,e}}\|_{\sigma} \|\tilde{\boldsymbol{\sigma}}^{\text{e}}\|_{\sigma}$  whereas the bound gap of the bounds in equation (76) is  $\zeta_U^{\text{C-S}} - \zeta_L^{\text{C-S}} = 2\|\tilde{\boldsymbol{\sigma}}^{\text{d,e}}\|_{\sigma} \|\tilde{\boldsymbol{\sigma}}^{\text{e}}\|_{\sigma}$ . Hence, the bound gap in equation (80) is half of the bound gap corresponding to equation (76), that is  $\zeta_U - \zeta_L = \frac{1}{2}(\zeta_U^{\text{C-S}} - \zeta_L^{\text{C-S}})$ , and provides a sharper error assessment.

### 6.2.3 Equivalent alternative approach

The error bounds (82) are derived here using an alternative presentation, without introducing the symmetrized error equations and following a rationale similar to the one presented in references [67, 68] for steady-state linear elasticity. This alternative approach requires introducing an auxiliary stress field that stands for the error with respect to the averaged viscous stress, namely

$$\boldsymbol{\sigma}_{\text{ave}}^{\text{e},\nu} := \boldsymbol{\sigma}^{\nu} - \frac{1}{2} (\tilde{\boldsymbol{\sigma}}^{\nu} + \mathbf{s}^{\nu}(\tilde{\mathbf{u}})). \quad (83)$$

Note that  $\boldsymbol{\sigma}_{\text{ave}}^{\text{e},\nu}$  is introduced as a mathematical artifact (it is not computable because it involves the exact solution  $\mathbf{u}$ ) allowing to rewrite the residual  $R^{\text{d}}(\mathbf{e})$  as

$$R^{\text{d}}(\mathbf{e}) = \bar{B}^{\nu}(\tilde{\boldsymbol{\sigma}}^{\text{d,e}}, \mathbf{s}^{\nu}(\mathbf{e})) = \bar{B}^{\nu}(\tilde{\boldsymbol{\sigma}}^{\text{d,e}}, \boldsymbol{\sigma}_{\text{ave}}^{\text{e},\nu}) + \frac{1}{2} \bar{B}^{\nu}(\tilde{\boldsymbol{\sigma}}^{\text{d,e}}, \tilde{\boldsymbol{\sigma}}^{\text{e}}). \quad (84)$$

Hence, the bounds for  $R^d(\mathbf{e})$  are obtained bounding the value  $\bar{B}^\nu(\tilde{\boldsymbol{\sigma}}^{d,e}, \boldsymbol{\sigma}_{\text{ave}}^{e,\nu})$  which is the only non-computable term in the right hand side of equation (84). The computable bound for  $\bar{B}^\nu(\tilde{\boldsymbol{\sigma}}^{d,e}, \boldsymbol{\sigma}_{\text{ave}}^{e,\nu})$  is derived by applying the Cauchy-Schwartz inequality

$$|\bar{B}^\nu(\tilde{\boldsymbol{\sigma}}^{d,e}, \tilde{\boldsymbol{\sigma}}_{\text{ave}}^e)| \leq \|\tilde{\boldsymbol{\sigma}}^{d,e}\|_\sigma \|\tilde{\boldsymbol{\sigma}}_{\text{ave}}^{e,\nu}\|_\sigma, \quad (85)$$

and then bounding  $\|\tilde{\boldsymbol{\sigma}}_{\text{ave}}^{e,\nu}\|_\sigma$ . The following theorem proves that the constitutive relation error  $\tilde{\boldsymbol{\sigma}}^e$  provides a bound for  $\|\tilde{\boldsymbol{\sigma}}_{\text{ave}}^{e,\nu}\|_\sigma$ .

**Theorem 3.** *The constitutive relation error  $\tilde{\boldsymbol{\sigma}}^e$  defined in equation (70) leads to the following upper bound of the averaged stress error  $\boldsymbol{\sigma}_{\text{ave}}^{e,\nu}$  defined in equation (83),*

$$\frac{1}{2} \|\tilde{\boldsymbol{\sigma}}^e\|_\sigma \geq \|\boldsymbol{\sigma}_{\text{ave}}^{e,\nu}\|_\sigma. \quad (86)$$

*Proof.* Using the relation  $\boldsymbol{\sigma}_{\text{ave}}^{e,\nu} = \mathbf{s}^\nu(\mathbf{e}) - \frac{1}{2}\tilde{\boldsymbol{\sigma}}^e$ , the value  $\|\boldsymbol{\sigma}_{\text{ave}}^{e,\nu}\|_\sigma^2$  is rewritten as

$$\begin{aligned} \|\boldsymbol{\sigma}_{\text{ave}}^{e,\nu}\|_\sigma^2 &= \frac{1}{4} \|\tilde{\boldsymbol{\sigma}}^e\|_\sigma^2 + \|\mathbf{s}^\nu(\mathbf{e})\|_\sigma^2 - \bar{B}^\nu(\mathbf{s}^\nu(\mathbf{e}), \tilde{\boldsymbol{\sigma}}^e), \\ &= \frac{1}{4} \|\tilde{\boldsymbol{\sigma}}^e\|_\sigma^2 + \bar{B}^\nu(\mathbf{s}^\nu(\mathbf{e}), \mathbf{s}^\nu(\mathbf{e}) - \tilde{\boldsymbol{\sigma}}^e), \\ &= \frac{1}{4} \|\tilde{\boldsymbol{\sigma}}^e\|_\sigma^2 + \bar{B}^\nu(\mathbf{s}^\nu(\mathbf{e}), \boldsymbol{\sigma}^\nu - \tilde{\boldsymbol{\sigma}}^\nu). \end{aligned}$$

The proof is concluded noting that  $\bar{B}^\nu(\mathbf{s}^\nu(\mathbf{e}), \boldsymbol{\sigma}^\nu - \tilde{\boldsymbol{\sigma}}^\nu) \leq 0$ , which directly proves that (86) holds.

The statement  $\bar{B}^\nu(\mathbf{s}^\nu(\mathbf{e}), \boldsymbol{\sigma}^\nu - \tilde{\boldsymbol{\sigma}}^\nu) \leq 0$  is proved noting that, for an admissible pair  $(\tilde{\mathbf{u}}, \tilde{\boldsymbol{\sigma}}) \in \mathcal{U} \times \mathcal{S}(\tilde{\mathbf{u}})$ , the following relation holds

$$0 = \int_I (\rho(\ddot{\mathbf{u}} - \ddot{\tilde{\mathbf{u}}}, \dot{\mathbf{e}}) dt + \int_I (\boldsymbol{\sigma} - \tilde{\boldsymbol{\sigma}}, \boldsymbol{\varepsilon}(\dot{\mathbf{e}})) dt. \quad (87)$$

Then, injecting the expression

$$\boldsymbol{\sigma} - \tilde{\boldsymbol{\sigma}} = \mathbf{s}^E(\mathbf{u} - \tilde{\mathbf{u}}) + \boldsymbol{\sigma}^\nu - \tilde{\boldsymbol{\sigma}}^\nu,$$

into equation (87) one has

$$\begin{aligned} 0 &= \int_I (\rho\ddot{\mathbf{e}}, \dot{\mathbf{e}}) dt + \int_I a(\mathbf{e}, \dot{\mathbf{e}}) dt + \bar{B}^\nu(\mathbf{s}^\nu(\mathbf{e}), \boldsymbol{\sigma}^\nu - \tilde{\boldsymbol{\sigma}}^\nu) \\ &= \frac{1}{2} \int_I \frac{d}{dt} (\rho\dot{\mathbf{e}}, \dot{\mathbf{e}}) dt + \frac{1}{2} \int_I \frac{d}{dt} a(\mathbf{e}, \mathbf{e}) dt + \bar{B}^\nu(\mathbf{s}^\nu(\mathbf{e}), \boldsymbol{\sigma}^\nu - \tilde{\boldsymbol{\sigma}}^\nu) \\ &= \frac{1}{2} [\|\dot{\mathbf{e}}\|_m^2 + \|\mathbf{e}\|_a^2]_{t=0}^{t=T} + \bar{B}^\nu(\mathbf{s}^\nu(\mathbf{e}), \boldsymbol{\sigma}^\nu - \tilde{\boldsymbol{\sigma}}^\nu). \end{aligned}$$

Taking into account that  $\mathbf{e}(0) = \dot{\mathbf{e}}(0) = \mathbf{0}$ , one has

$$\bar{B}^\nu(\mathbf{s}^\nu(\mathbf{e}), \boldsymbol{\sigma}^\nu - \tilde{\boldsymbol{\sigma}}^\nu) = -\frac{1}{2} \|\dot{\mathbf{e}}(T)\|_m^2 - \frac{1}{2} \|\mathbf{e}(T)\|_a^2,$$

which proves that  $\bar{B}^\nu(\mathbf{s}^\nu(\mathbf{e}), \boldsymbol{\sigma}^\nu - \tilde{\boldsymbol{\sigma}}^\nu) \leq 0$ .  $\square$

Using equations (85) and (86), the computable bound for  $|\bar{B}^\nu(\tilde{\boldsymbol{\sigma}}^{\text{d,e}}, \tilde{\boldsymbol{\sigma}}_{\text{ave}}^{\text{e}})|$  is readily obtained as

$$|\bar{B}^\nu(\tilde{\boldsymbol{\sigma}}^{\text{d,e}}, \tilde{\boldsymbol{\sigma}}_{\text{ave}}^{\text{e}})| \leq \frac{1}{2} \|\tilde{\boldsymbol{\sigma}}^{\text{d,e}}\|_\sigma \|\tilde{\boldsymbol{\sigma}}^{\text{e}}\|_\sigma. \quad (88)$$

Using equation (88) together with equations (84) and (37), the result given in equation (82) is derived in an alternative way, without using the symmetrized error equations.

### 6.3 Construction of D-admissible fields

This section describes in detail the computation of a D-admissible stress  $\tilde{\boldsymbol{\sigma}} \in \mathcal{S}(\tilde{\mathbf{u}})$ , given a K-admissible field  $\tilde{\mathbf{u}} \in \mathcal{U}$ . The presentation focuses in the original problem because the same methodology is used also for the adjoint problem.

The stress  $\tilde{\boldsymbol{\sigma}} \in \mathcal{S}(\tilde{\mathbf{u}})$  is characterized by a series of stresses  $\tilde{\boldsymbol{\sigma}}_n$ ,  $n = 0, \dots, N$  at the time points in  $\mathcal{T}$ . Each  $\tilde{\boldsymbol{\sigma}}_n$  is seen as a statically equilibrated stress field for some loading. Thus, the D-admissible stress  $\tilde{\boldsymbol{\sigma}}$  is eventually computed solving a series of static equilibration problems following the standard procedures described in [9, 8, 66].

The following theorem demonstrates how the the D-admissible stress  $\tilde{\boldsymbol{\sigma}} \in \mathcal{S}(\tilde{\mathbf{u}})$  can be computed in terms of the statically equilibrated stresses  $\tilde{\boldsymbol{\sigma}}_n$ ,  $n = 0, \dots, N$ .

**Theorem 4.** *Given the external loads  $\mathbf{f}, \mathbf{g}$  and a K-admissible field  $\tilde{\mathbf{u}} \in \mathcal{U}$ , then a D-admissible stress  $\tilde{\boldsymbol{\sigma}} \in \mathcal{S}(\tilde{\mathbf{u}})$  is straightforwardly defined through piecewise linear interpolation in time*

$$\tilde{\boldsymbol{\sigma}}(\mathbf{x}, t) := \sum_{n=0}^N \tilde{\boldsymbol{\sigma}}_n(\mathbf{x}) \theta_n(t), \quad (89)$$

provided that: 1) the stress fields  $\tilde{\boldsymbol{\sigma}}_n$ ,  $n = 0, \dots, N$  fulfill the static equilibrium condition

$$(\tilde{\boldsymbol{\sigma}}_n, \boldsymbol{\varepsilon}(\mathbf{w})) = l_n(\mathbf{w}) - (\rho \ddot{\mathbf{u}}_n, \mathbf{w}) \quad \forall \mathbf{w} \in \mathcal{V}_0, \quad (90)$$

or equivalently

$$\nabla \cdot \tilde{\boldsymbol{\sigma}}_n = \mathbf{f}_n - \rho \ddot{\mathbf{u}}_n \quad \text{in } \Omega^{\text{int}}, \quad (91a)$$

$$[[\tilde{\boldsymbol{\sigma}}_n \cdot \mathbf{n}]] = \mathbf{0} \quad \text{on } \Gamma^{\text{int}}, \quad (91b)$$

$$\tilde{\boldsymbol{\sigma}}_n \cdot \mathbf{n} = \mathbf{g}_n \quad \text{on } \Gamma_N, \quad (91c)$$

and 2) the external loads  $\mathbf{f}, \mathbf{g}$  and the acceleration  $\ddot{\mathbf{u}}$  are piecewise linear in time, i.e.

$$\mathbf{f}(\mathbf{x}, t) = \sum_{n=0}^N \mathbf{f}_n(\mathbf{x}) \theta_n(t), \quad (92a)$$

$$\mathbf{g}(\mathbf{x}, t) = \sum_{n=0}^N \mathbf{g}_n(\mathbf{x}) \theta_n(t), \quad (92b)$$

$$\ddot{\mathbf{u}}(\mathbf{x}, t) = \sum_{n=0}^N \ddot{\mathbf{u}}_n(\mathbf{x}) \theta_n(t). \quad (92c)$$

Most of the techniques providing D-admissible stresses, see [9, 8, 66], require as input an approximation of the stresses, say  $\boldsymbol{\sigma}_n^{H,\Delta t} \approx \boldsymbol{\sigma}(t_n)$ , fulfilling a discrete form of (90), namely

$$(\rho \ddot{\mathbf{u}}_n, \mathbf{w}) + (\boldsymbol{\sigma}_n^{H,\Delta t}, \boldsymbol{\varepsilon}(\mathbf{w})) = l_n(\mathbf{w}) \quad \forall \mathbf{w} \in \mathcal{V}_0^H, \quad (93)$$

being  $\mathcal{V}_0^H$  the usual functional space associated with the computational mesh. This relation guarantees that the local problems are solvable.

Note that equation (93) holds defining the admissible solution  $\tilde{\mathbf{u}}$  as in (7), that is  $\ddot{\mathbf{u}}_n = \mathbf{a}_n^{H,\Delta t}$ , and taking discrete stress as  $\boldsymbol{\sigma}_n^{H,\Delta t} := \mathcal{C} : \boldsymbol{\varepsilon}(\mathbf{u}_n^{H,\Delta t} + a_2 \mathbf{v}_n^{H,\Delta t})$  (being  $\mathbf{u}_n^{H,\Delta t}$  and  $\mathbf{v}_n^{H,\Delta t}$  the Newmark displacements and velocities).

### 6.3.1 The hybrid fluxes method

The hybrid fluxes method introduced by Ladevèze in [2] is a classical stress equilibration technique. It is also denoted in more recent works by EET (Element Equilibration Technique). This methodology provides stress fields  $\tilde{\boldsymbol{\sigma}}_n$ ,  $n = 0, \dots, N$ , fulfilling equations (90) and (91). The construction of the equilibrated stress field  $\tilde{\boldsymbol{\sigma}}_n$  is based on some approximate stress  $\boldsymbol{\sigma}_n^{H,\Delta t}$  that is taken as the input of the procedure. This section is devoted to present this methodology, stressing the technical details of its application to compute D-admissible stresses.

Some additional notations are needed to introduce the hybrid fluxes method. The location of a generic node of the computational mesh is denoted by  $\mathbf{x}_i$ ,  $i = 1, \dots, N_{\text{no}}$ , being  $N_{\text{no}}$  the total number of nodes. As introduced before, elements in the mesh are denoted by  $\Omega_k \subset \Omega$ ,  $k = 1, \dots, N_{\text{el}}$ , where  $N_{\text{el}}$  is the total number of elements. Element sides (or faces in 3D) are denoted by  $\Gamma_l \subset \bar{\Omega}$ ,  $l = 1, \dots, N_{\text{fa}}$ , being  $N_{\text{fa}}$  their total number (note that  $\Gamma_l$  is either an inter-element boundary, that is  $\Gamma_l = \partial\Omega_k \cap \partial\Omega_{k'}$  for some  $k$  and  $k'$  or a boundary element side, that is  $\Gamma_l = \partial\Omega_k \cap \partial\Omega$  for some  $k$ ). Also, some sets of indices are introduced describing the connectivity of every node  $\mathbf{x}_i$ , element  $\Omega_k$  and face  $\Gamma_l$ . The set  $\mathcal{N}(\Omega_k)$  is the standard connectivity information containing the indices of the nodes of element  $\Omega_k$ . The set  $\mathcal{E}(\mathbf{x}_i)$  contains the indices of the elements to which node  $\mathbf{x}_i$  belongs. The set  $\mathcal{F}(\mathbf{x}_i)$  contains the indices of the sides/faces to which node  $\mathbf{x}_i$  belongs. The set  $\mathcal{F}(\Omega_k)$  contains the indices of the faces of element  $\Omega_k$ . Finally, the set  $\mathcal{N}(\Gamma_l)$  contains the indices of the nodes of face  $\Gamma_l$ . Figure 8 illustrates the definitions of these sets.

The equilibrated stresses  $\tilde{\boldsymbol{\sigma}}_n$  at time  $t_n$ ,  $n = 0, \dots, N$ , is computed solving local equilibrium problems element-by-element. Each local problem consists in finding the restriction of  $\tilde{\boldsymbol{\sigma}}_n$  to element  $\Omega_k$  of the mesh,  $k = 1, \dots, N_{\text{el}}$ , such that

$$\nabla \cdot \tilde{\boldsymbol{\sigma}}_n = \mathbf{f}_n - \rho \ddot{\mathbf{u}}_n \quad \text{in } \Omega_k, \quad (94a)$$

$$\tilde{\boldsymbol{\sigma}}_n \cdot \mathbf{n}_k = \eta_{kl} \mathbf{t}_n \quad \text{on } \partial\Gamma_l \subset \Omega_k. \quad (94b)$$

It is worth noting that the boundary conditions (94b) for the local problem (94) are not known and they require obtaining the inter-element tractions  $\mathbf{t}_n$ , defined on every  $\Gamma_l$ , for  $l = 1, \dots, N_{\text{fa}}$ . The coefficient  $\eta_{kl}$  takes the values 1 or -1, depending on the orientation of the face  $\Gamma_l$  with respect to  $\Omega_k$ . It is assumed that the orientation of  $\Gamma_l$  is given by a

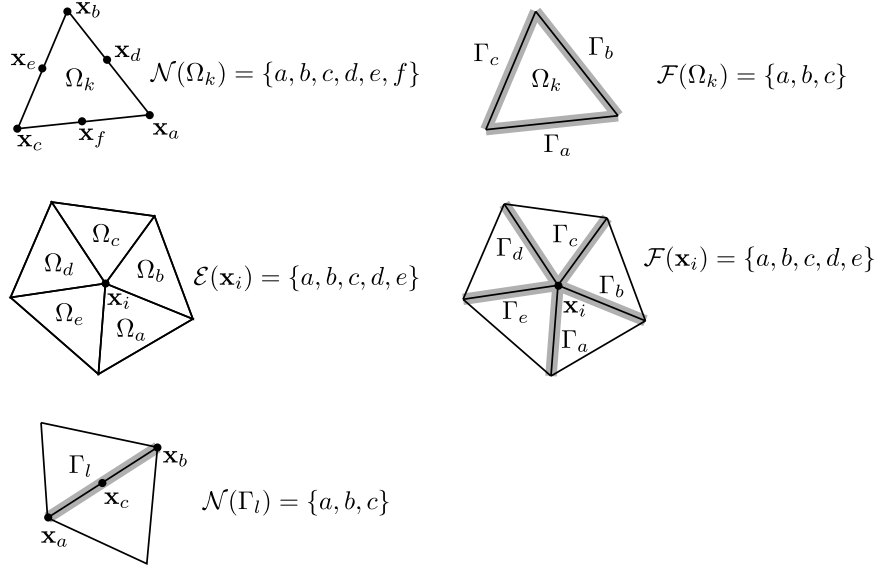


Figure 8: Illustration of sets  $\mathcal{N}(\Omega_k)$ ,  $\mathcal{F}(\Omega_k)$ ,  $\mathcal{E}(\mathbf{x}_i)$ ,  $\mathcal{F}(\mathbf{x}_i)$  and  $\mathcal{N}(\Gamma_l)$ .

normal unit vector  $\tilde{\mathbf{n}}_l$  and then  $\eta_{kl} = \tilde{\mathbf{n}}_l \cdot \mathbf{n}_k$ . Moreover, equation(94) is a pure Neumann problem and therefore is only well posed if the prescribed loads, the body forces and the tractions in the right-hand-sides of (94a) and (94b), are in equilibrium.

Thus, the inter-element tractions  $\mathbf{t}_n$  must be computed previous to solving the local problems (94) and they must fulfill local equilibrium, namely

$$\sum_{l \in \mathcal{F}(\Omega_k)} \int_{\Gamma_l} \eta_{kl} \mathbf{t}_n \cdot \mathbf{w} \, d\Gamma + \int_{\Omega_k} (\mathbf{f}_n - \rho \ddot{\mathbf{u}}_n) \cdot \mathbf{w} \, d\Omega = 0, \quad (95)$$

for all  $\mathbf{w}$  in the space of rigid body motions. This space is defined as (in 3D)  $\text{span}\{(1, 0, 0)^T, (0, 1, 0)^T, (0, 0, 1)^T, (-y, x, 0)^T, (-z, 0, x)^T, (0, -z, y)^T\}$ .

On the faces on the Neumann boundary, that is for  $\Gamma_l \subset \Gamma_N$ , the tractions have to match the actual boundary conditions (91c), that is  $\eta_{kl} \mathbf{t}_n = \mathbf{g}_n$ .

At the first sight, obtaining the inter-element tractions  $\mathbf{t}_n$  fulfilling (95) leads to a global problem and requires solving the unknowns for all the faces  $\Gamma_l$ , for  $l = 1, \dots, N_{\text{fa}}$ , resulting in a large system of linear equations. In practice, this problem is decoupled into local computations thanks to the idea introduced in [2], which is based on enforcing locally (for each element  $\Omega_k$ ) the so-called *prolongation condition*

$$\int_{\Omega_k} (\tilde{\boldsymbol{\sigma}}_n - \boldsymbol{\sigma}_n^{H, \Delta t}) \cdot \nabla \varphi_i \, d\Omega = \mathbf{0} \quad \forall i \in \mathcal{N}(\Omega_k), \quad (96)$$

where  $\varphi_i$  is the shape function associated with node  $\mathbf{x}_i$ . Note that this additional restriction is selecting a particular solution for  $\mathbf{t}_n$  and hence of  $\tilde{\boldsymbol{\sigma}}_n$ . The problem (95) is decoupled into local computations precisely for this particular solution.



Assuming that  $\boldsymbol{\sigma}^{H,\Delta t}$  is such that (93) holds, enforcing the prolongation condition (96) is equivalent to find  $\mathbf{t}_n$  such that

$$\sum_{l \in \mathcal{F}(\Omega_k)} \int_{\Gamma_l} \eta_{kl} \mathbf{t}_n \varphi_i \, d\Gamma = \int_{\Omega_k} (\boldsymbol{\sigma}_n^{H,\Delta t} \cdot \nabla \varphi_i - (\mathbf{f}_n - \rho \ddot{\mathbf{u}}_n) \varphi_i) \, d\Omega, \quad (97)$$

for all mesh elements  $\Omega_k$ ,  $k = 1, \dots, N_{\text{el}}$  and for all nodes  $i \in \mathcal{N}(\Omega_k)$ , pertaining to element  $\Omega_k$ .

If  $\mathbf{t}_n$  fulfills (97) and (93) holds, then the equilibrium condition (95) is satisfied and  $\tilde{\boldsymbol{\sigma}}_n$  can be computed solving the local problems (94). Thus, the tractions  $\mathbf{t}_n$  are obtained such that they fulfill equation (97).

The following definitions are introduced:

$$\mathbf{b}_{il} := \int_{\Gamma_l} \mathbf{t}_n \varphi_i \, d\Gamma \quad \text{and} \quad \mathbf{j}_{ki} := \int_{\Omega_k} (\boldsymbol{\sigma}_n^{H,\Delta t} \cdot \nabla \varphi_i - (\mathbf{f}_n - \rho \ddot{\mathbf{u}}_n) \varphi_i) \, d\Omega. \quad (98)$$

Note that  $\mathbf{b}_{il}$  is nonzero only if  $l \in \mathcal{F}(\mathbf{x}_i)$  or, conversely, if  $i \in \mathcal{N}(\Gamma_l)$ . Thus, equation (97) yields

$$\sum_{l \in (\mathcal{F}(\Omega_k) \cap \mathcal{F}(\mathbf{x}_i))} \eta_{kl} \mathbf{b}_{il} = \mathbf{j}_{ki}, \quad (99)$$

for all mesh elements  $\Omega_k$ ,  $k = 1, \dots, N_{\text{el}}$  and for all nodes  $i \in \mathcal{N}(\Omega_k)$ .

Expression (99) is a linear system of vectorial equations (vectorial, in the sense that  $\mathbf{b}_{il}$  and  $\mathbf{j}_{ki}$  are vectors). The number of vectorial equations is equal to the number of elements  $N_{\text{el}}$  times the number of element nodes (i.e. three for linear triangles). The unknowns are the values  $\mathbf{b}_{il}$ , for  $l = 1, \dots, N_{\text{fa}}$  and  $i \in \mathcal{N}(\Gamma_l)$ , which are the projections of the traction  $\mathbf{t}_n$  in the FE functional space (restricted to the faces). The number of unknowns is  $N_{\text{fa}}$  times the number of edge/face nodes (i.e. two for linear triangles). The number of unknowns is typically larger than the number of equations, and therefore, additional criteria are required to select one of the solutions.

**Remark 5.** *For the sake of illustration, the equations and unknowns accounting is performed for linear 2D triangles in the case of a Dirichlet problem. The number of equations in (99) is  $N_{\text{eq}} = 3N_{\text{el}}$  and the number of unknowns is  $N_{\text{unk}} = 2N_{\text{fa}}$ . The number of mesh faces is expressed in terms of the number of mesh elements as  $N_{\text{fa}} = \frac{3}{2}N_{\text{el}} + \frac{1}{2}N_{\partial\Omega}$ , where  $N_{\partial\Omega}$  is the number of faces on the boundary. Thus, the number of unknowns and equations are such that  $N_{\text{unk}} = N_{\text{eq}} + N_{\partial\Omega} > N_{\text{eq}}$ .*

At first sight, expression (99) leads to a global system of equations, involving the complete computational domain. However, the global system is decoupled into  $N_{\text{no}}$  local systems, associated with each node of the mesh,  $\mathbf{x}_i$ , and involving only the unknowns  $\mathbf{b}_{il}$  for  $l \in \mathcal{F}(\mathbf{x}_i)$ . In other words, the range for  $i$  and  $k$  in the system of equations (99) is rewritten as: for  $i = 1, \dots, N_{\text{no}}$  and then for all  $k \in \mathcal{E}(\mathbf{x}_i)$ . In that sense, for a given value of  $i$ , stating (99) for all  $k \in \mathcal{E}(\mathbf{x}_i)$  leads to a system of equations involving only the unknowns  $\mathbf{b}_{il}$  for  $l \in \mathcal{F}(\mathbf{x}_i)$  which do not participate in any other local system associated

with a different node. For a given  $\mathbf{x}_i$ ,  $i = 1, \dots, N_{\text{no}}$ , the local problem is a reformulation of (99) reading

$$\sum_{l \in (\mathcal{F}(\Omega_k) \cap \mathcal{F}(\mathbf{x}_i))} \eta_{kl} \mathbf{b}_{il} = \mathbf{j}_{ki}, \quad \forall k \in \mathcal{E}(\mathbf{x}_i). \quad (100)$$

All the unknown values  $\mathbf{b}_{il}$ , for  $l = 1, \dots, N_{\text{fa}}$  and  $i \in \mathcal{N}(\Gamma_l)$ , are determined once the local problems are solved for all mesh nodes  $\mathbf{x}_i$ ,  $i = 1, \dots, N_{\text{no}}$ .

The actual resolution of the system (100) depends whether the current node is interior or on the boundary, and (for higher order elements) if the node coincides with a mesh vertex or not. See reference [9] for a detailed discussion of all these cases.

For the sake of simplicity, the presentation in detail of one of these local systems is restricted to the particular case of an interior (not on the boundary) node  $\mathbf{x}_i$ , being also an element vertex. In this case, the number of equations in system (100) is  $\#\mathcal{E}(\mathbf{x}_i)$  (one for each element in  $\mathcal{E}(\mathbf{x}_i)$ ,  $\#$  denotes the cardinal) and the number of unknowns is  $\#\mathcal{F}(\mathbf{x}_i)$  (one for each face in  $\mathcal{F}(\mathbf{x}_i)$ ). Note that the number of elements in  $\mathcal{E}(\mathbf{x}_i)$  coincides with the number of faces in  $\mathcal{F}(\mathbf{x}_i)$  because the node  $\mathbf{x}_i$  is interior. Thus, the local system (100) has the same number of equations and unknowns. The square matrix associated with the local system of equations (100) has entries  $\eta_{kl}$  (thus, equal to  $\pm 1$  or equal to 0 if  $k \notin \mathcal{E}(\Gamma_l)$ ) and does not have full rank. The rank deficiency is readily shown by summing up all the equations of system (100) (summing up in  $k$ ). Note that for a given face  $\Gamma_l$  there are only two adjacent elements, say  $\tilde{k}$  and  $\tilde{k}'$ . Consequently, the resulting equation is

$$\sum_{l \in \mathcal{F}(\mathbf{x}_i)} (\eta_{\tilde{k}l} + \eta_{\tilde{k}'l}) \mathbf{b}_{il} = \sum_{k \in \mathcal{E}(\mathbf{x}_i)} \mathbf{j}_{ki},$$

Note that  $\eta_{\tilde{k}l} + \eta_{\tilde{k}'l} = 0$  and therefore problem (100) is solvable only if the right hand side data fulfills

$$\sum_{k \in \mathcal{E}(\mathbf{x}_i)} \mathbf{j}_{ki} = \mathbf{0}.$$

The previous requirement is fulfilled if equation (93) holds. In fact, this is a version of the Galerkin orthogonality property. Under this assumption, system (100) is compatible but, due to the rank deficiency, it has infinite solutions. A particular solution is found such that it minimizes the functional

$$\Phi_i(\mathbf{b}_{il}) := \frac{1}{2} \sum_{l \in \mathcal{F}(\mathbf{x}_i)} (\mathbf{b}_{il} - \bar{\mathbf{b}}_{il})^2,$$

with

$$\bar{\mathbf{b}}_{il} := \frac{1}{2} \int_{\Gamma_l} (\boldsymbol{\sigma}_n^{H,\Delta t}|_{\Omega_{\tilde{k}}} + \boldsymbol{\sigma}_n^{H,\Delta t}|_{\Omega_{\tilde{k}'}}) \cdot \tilde{\mathbf{n}}_l \varphi_i \, d\Gamma.$$

Once the quantities  $\mathbf{b}_{il}$ , for  $l = 1, \dots, N_{\text{fa}}$  and  $i \in \mathcal{N}(\Gamma_l)$ , are available, the tractions  $\mathbf{t}_n$  are completely determined. In some cases, is it useful to parametrize tractions  $\mathbf{t}_n$  using nodal values instead of quantities  $\mathbf{b}_{il}$ . Specifically, the nodal values parametrizing the

restriction of  $\mathbf{t}_n$  to the face  $\Gamma_l$  are obtained solving a linear system of equations with the *mass* matrix with entries

$$\int_{\Gamma_l} \varphi_i \varphi_j \, d\Gamma, \quad i, j \in \mathcal{N}(\Gamma_l),$$

and the right hand side vector containing the values  $\mathbf{b}_{il}$ ,  $i \in \mathcal{N}(\Gamma_l)$ . For 2D linear elements, the system to be solved at each element side has two unknowns and two equations.

Once the tractions  $\mathbf{t}_n$  are available, the stress field  $\tilde{\boldsymbol{\sigma}}_n$  is obtained solving the local problems (94) in each element  $\Omega_k$ . The local Neumann problems (94) can be solved taking as unknowns either displacements (standard FE approach) or stresses (the so-called dual formulations). The standard displacement-based approach uses a finite element solver locally, selecting a reference mesh (created with  $H$  or  $p$  refinement) discretizing each element. The local approximate solution undervaluates the energy of the exact solution and therefore the global upper bound property is not strictly guaranteed. The resulting estimates are referred as *asymptotic* [69] because the upper bound property holds only asymptotically, as the element size of the reference mesh tends to zero (or the degree of the polynomial tends to infinity). Alternatively, the dual approach (taking stresses as unknowns) provides directly D-admissible piecewise polynomial solutions for  $\tilde{\boldsymbol{\sigma}}_n$ . In this case, the upper bound property is guaranteed and therefore the estimates are denoted as *strict*.

The general procedure to compute the stress  $\tilde{\boldsymbol{\sigma}}_n$  is summarized in algorithm 1.

**Data:**

- Approximate stress field  $\boldsymbol{\sigma}_n^{H, \Delta t}$ ,
- K-admissible displacement  $\tilde{\mathbf{u}}_n$  and
- geometrical information of the finite element mesh (nodes, elements and faces)

**Result:**

- Equilibrated stress  $\tilde{\boldsymbol{\sigma}}_n$

```

// Compute equilibrated interelement tractions
for  $i = 1, \dots, N_{\text{no}}$  (loop in nodes  $\mathbf{x}_i$ ) do
    | compute  $\mathbf{b}_{il}$ ,  $l = 1, \dots, \mathcal{F}(\mathbf{x}_i)$  solving local system (100);
end
(Traction  $\mathbf{t}_n$  at  $\Gamma_l$ ,  $l = 1, \dots, N_{\text{fa}}$  is characterized from the values  $\mathbf{b}_{il}$ )
// Compute equilibrated stress
for  $k = 1, \dots, N_{\text{el}}$  (loop in elements  $\Omega_k$ ) do
    | compute the equilibrated stress  $\tilde{\boldsymbol{\sigma}}_n$  solving the local problems (94);
end
    
```

**Algorithm 1:** Computation of equilibrated stresses  $\tilde{\boldsymbol{\sigma}}_n$  with the hybrid fluxes method.

### 6.3.2 The flux-free method

The flux free method furnishes equilibrated stresses  $\tilde{\boldsymbol{\sigma}}_n$ ,  $n = 0, \dots, N$ , fulfilling equations (90) and (91) without requiring any equilibrated tractions to set the boundary conditions of the local problems. That is, the local Neumann problems do not require enforcing any flux on the boundary. This reduces considerably the implementation complexity of the method.

The equilibrated stresses  $\tilde{\boldsymbol{\sigma}}_n$ ,  $n = 0, \dots, N$ , are generated as a correction of the computed stress  $\boldsymbol{\sigma}_n^{H,\Delta t}$ ,

$$\tilde{\boldsymbol{\sigma}}_n^{\text{ff}} := \mathbf{C} : \boldsymbol{\varepsilon}(\tilde{\mathbf{e}}_n) + \boldsymbol{\sigma}_n^{H,\Delta t}, \quad (101)$$

where  $\tilde{\mathbf{e}}_n$  is an estimate of the error in displacements, computed solving local flux free problems [66]. As for the hybrid fluxes method, the computed stress  $\boldsymbol{\sigma}_n^{H,\Delta t}$  has to fulfill equation (93) to ensure solvability of the local problems.

For the sake of simplicity, the presentation is restricted to linear elements. In this case, all the nodes  $\mathbf{x}_i$ ,  $i = 1, \dots, N_{\text{no}}$ , are also mesh vertices. The main rationale of the flux free method is to define function  $\tilde{\mathbf{e}}_n$  as the addition of local estimates  $\tilde{\mathbf{e}}_n^i$  associated with the mesh vertices, namely

$$\tilde{\mathbf{e}}_n := \sum_{i=1}^{N_{\text{no}}} \tilde{\mathbf{e}}_n^i. \quad (102)$$

Each local estimate  $\tilde{\mathbf{e}}_n^i$  is computed solving a problem defined in the patch  $\omega^i := \text{supp}(\varphi_i)$  centered at node  $\mathbf{x}_i$ . The local problem is solved with a refined finite element mesh in the patch  $\omega^i$ . The characteristic element size of this refined mesh is  $h \ll H$  and the corresponding functional space is denoted by  $\mathcal{V}_{\omega^i}^h$ .

The local estimate  $\tilde{\mathbf{e}}_n^i$  is one solution of the problem: find  $\tilde{\mathbf{e}}_n^i \in \mathcal{V}_{\omega^i}^h$  such that

$$a(\tilde{\mathbf{e}}_n^i, \mathbf{w}) = R_n(\varphi_i(\mathbf{w} - \boldsymbol{\Pi}^H \mathbf{w})) \quad \forall \mathbf{w} \in \mathcal{V}_{\omega^i}^h, \quad (103)$$

where the weak residual  $R_n$  stands for

$$R_n(\mathbf{w}) := l_n(\mathbf{w}) - (\rho \ddot{\mathbf{u}}_n, \mathbf{w}) - (\boldsymbol{\sigma}_n^{H,\Delta t}, \boldsymbol{\varepsilon}(\mathbf{w})). \quad (104)$$

Here, the operator  $\boldsymbol{\Pi}^H : \mathcal{V}_0 \rightarrow \mathcal{V}_0^H$  is the interpolation operator in  $\mathcal{V}_0^H$ . Once  $\tilde{\mathbf{e}}_n^i$  are computed for  $i = 1, \dots, N_{\text{no}}$  solving (103),  $\tilde{\mathbf{e}}_n$  is recovered using (102) and the stress field  $\tilde{\boldsymbol{\sigma}}_n$  follows from (101).

It is worth noting that the flux-free method requires that the residual  $R_n$  fulfills Galerkin orthogonality. It allows introducing the projection  $\boldsymbol{\Pi}^H \mathbf{w}$  into the residual  $R_n(\cdot)$  which guarantees the well-posedness (solvability) of the local problems. Note that, if equation (93) holds, then the residual  $R_n$  introduced in (104) fulfills

$$R_n(\mathbf{w}) = 0 \quad \forall \mathbf{w} \in \mathcal{V}_0^H.$$

The flux-free recovered stresses  $\tilde{\boldsymbol{\sigma}}_n$  are equilibrated in the asymptotic sense, that is fulfilling equilibrium equations (91) but referred to a discrete space associated with the

reference  $h$ -mesh. Thus, the estimate provided by  $\tilde{\boldsymbol{\sigma}}_n$  does not yield a strict upper bound with respect to the exact error, as indicated in theorem 2. Even though, the flux-free estimate furnishes an asymptotic upper bound, that is the bounding properties hold when the element size  $h$  of the reference mesh tends to zero. The flux free method leads to strict bounds if the local problems are solved in stresses with a dual formulation, see [70] for details.

The procedure to compute the stress field  $\tilde{\boldsymbol{\sigma}}_n$  with the flux free approach is detailed in algorithm 2.

**Data:**

- Approximate stress field  $\boldsymbol{\sigma}_n^{H,\Delta t}$ ,
- K-admissible displacement  $\tilde{\mathbf{u}}_n$  and
- geometrical information of the finite element mesh (nodes and elements).

**Result:**

- Equilibrated stress  $\tilde{\boldsymbol{\sigma}}_n$ .

```

// Compute flux-free error estimate
initialize error estimate:  $\tilde{\mathbf{e}}_n = \mathbf{0}$ ;
for  $i = 1, \dots, N_{\text{no}}$  (loop in nodes  $\mathbf{x}_i$ ) do
    | compute the local estimates  $\tilde{\mathbf{e}}_n^i$  solving local systems (103);
    | add the contribution of  $\tilde{\mathbf{e}}_n^i$  to the global flux free estimate:  $\tilde{\mathbf{e}}_n \leftarrow \tilde{\mathbf{e}}_n + \tilde{\mathbf{e}}_n^i$ ;
end
// Compute equilibrated stress
Post-process  $\tilde{\mathbf{e}}_n$  into  $\tilde{\boldsymbol{\sigma}}_n = \mathcal{C} : \boldsymbol{\varepsilon}(\tilde{\mathbf{e}}_n) + \boldsymbol{\sigma}_n^{H,\Delta t}$ ;
    
```

**Algorithm 2:** Computation of the equilibrated stresses  $\tilde{\boldsymbol{\sigma}}_n$  with the flux-free method.

## 7 Error assessment for timeline-dependent quantities of interest

### 7.1 Timeline-dependent quantities of interest

Reference [71] introduces a new type of goal-oriented estimates assessing the error in so-called *timeline-dependent quantities of interest*. These new quantities are scalar time-dependent outputs of the solution instead of single scalar values and are specially well suited to transient problems. Timeline-dependent quantities are associated with a bounded mapping  $L_{\text{TL}}^{\mathcal{O}}(\cdot)$  taking a function  $\mathbf{w}$  in the solution space  $\mathcal{W}$  and returning a time-dependent scalar function, that is

$$L_{\text{TL}}^{\mathcal{O}} : \mathcal{W} \longrightarrow \mathcal{L}^2(I)$$

$$\mathbf{w} \longmapsto L_{\text{TL}}^{\mathcal{O}}(\mathbf{w}).$$

Note that the functional  $L_{\text{TL}}^{\mathcal{O}}(\cdot)$  is a different mathematical object than the functional  $L^{\mathcal{O}}(\cdot)$  associated with the standard quantities of interest because  $L_{\text{TL}}^{\mathcal{O}}(\cdot)$  returns a time-dependent scalar function and  $L^{\mathcal{O}}(\cdot)$  returns a single scalar value, see figure 9.

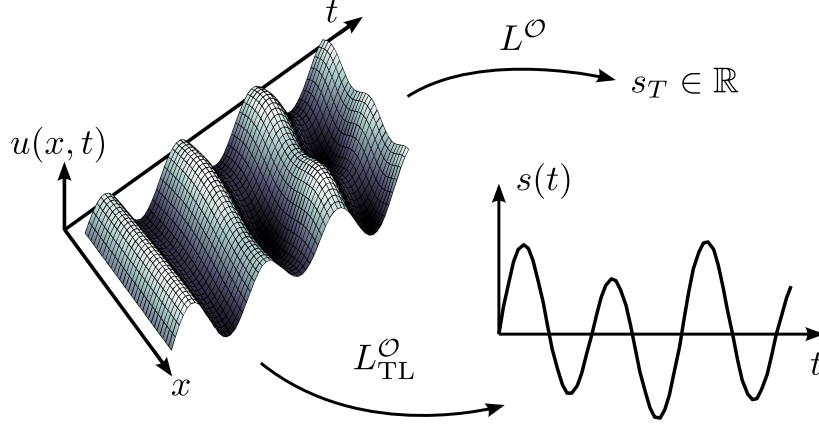


Figure 9: Illustration of scalar and timeline-dependent quantities of interest. The functional  $L^{\mathcal{O}}$  maps the time-space solution  $\mathbf{u}$  into a scalar value  $s_T \in \mathbb{R}$ . The operator  $L_{\text{TL}}^{\mathcal{O}}$  transforms  $\mathbf{u}$  into a time-dependent function  $s(t)$ .

A convenient expression for  $L_{\text{TL}}^{\mathcal{O}}(\cdot)$  is defined as an extension of the functional  $L^{\mathcal{O}}(\cdot)$  defined in (25),

$$[L_{\text{TL}}^{\mathcal{O}}(\mathbf{w})](t) := \int_0^t (\mathbf{f}^{\mathcal{O}}(\tau), \dot{\mathbf{w}}(\tau)) \, d\tau + \int_0^t (\mathbf{g}^{\mathcal{O}}(\tau), \dot{\mathbf{w}}(\tau))_{\Gamma_N} \, d\tau + (\rho \mathbf{v}^{\mathcal{O}}, \dot{\mathbf{w}}(t)) + a(\mathbf{u}^{\mathcal{O}}, \mathbf{w}(t)), \quad (105)$$

where the functions  $\mathbf{f}^{\mathcal{O}}$  and  $\mathbf{g}^{\mathcal{O}}$  define weighted averages of the solution in the interior domain  $\Omega$  or the Neumann boundary  $\Gamma_N$ , respectively, in the time interval  $[0, t]$  for a generic time  $t \in I$ . On the other hand, functions  $\mathbf{v}^{\mathcal{O}}$  and  $\mathbf{u}^{\mathcal{O}}$  define weighted averages of the velocities and displacements, respectively, at a generic time point  $t \in I$ . For the sake of simplicity, the notation  $L_{\text{TL}}^{\mathcal{O}}(\mathbf{w}; t) := [L_{\text{TL}}^{\mathcal{O}}(\mathbf{w})](t)$  is introduced.

The aim of reference [71] is assessing the quality of the computed timeline-dependent quantity,  $\tilde{s}(t) := L_{\text{TL}}^{\mathcal{O}}(\tilde{\mathbf{u}}; t)$ , with respect to the exact quantity of interest,  $s(t) := L_{\text{TL}}^{\mathcal{O}}(\mathbf{u}; t)$ . That is, the goal is to assess the error in the quantity of interest which is now a function of time

$$s^e(t) := s(t) - \tilde{s}(t).$$

## 7.2 Error representation with family of adjoint problems

Assessing the error in the timeline-quantity  $s^e(t)$  requires introducing an error representation similar to the one presented in section 3.4 for the scalar quantity of interest. Thus, an auxiliary problem, analogous to the adjoint problem (24), has to be introduced for the timeline quantity  $L_{\text{TL}}^{\mathcal{O}}(\cdot)$ .

This auxiliary problem is defined noting that, for a given time  $t \in I$ , the value  $s(t) = L_{\text{TL}}^{\mathcal{O}}(\mathbf{u}; t)$  is seen as a scalar quantity of interest taking  $t$  as the *final* time. This scalar quantity of interest is characterized as  $L^{\mathcal{O}}(\cdot) = L_{\text{TL}}^{\mathcal{O}}(\cdot; t)$ . Thus, the adjoint problem associated with  $L_{\text{TL}}^{\mathcal{O}}(\cdot; t)$ , for a given  $t \in I$ , is analogous to the one presented in 3.3 and reads: find  $\mathbf{u}_t^{\text{d}} \in \mathcal{W}|_{[0,t]}$  such that

$$B_t(\mathbf{w}, \mathbf{u}_t^{\text{d}}) = L_{\text{TL}}^{\mathcal{O}}(\mathbf{w}; t) \quad \forall \mathbf{w} \in \mathcal{W}|_{[0,t]}, \quad (106)$$

where the bilinear form  $B_t(\cdot, \cdot)$  is defined as

$$\begin{aligned} B_t(\mathbf{v}, \mathbf{w}) := & \int_0^t (\rho(\ddot{\mathbf{v}}(\tau) + a_1\dot{\mathbf{v}}(\tau)), \dot{\mathbf{w}}(\tau)) \, d\tau + \int_0^t a(\mathbf{v}(\tau) + a_2\dot{\mathbf{v}}(\tau), \dot{\mathbf{w}}(\tau)) \, d\tau \\ & + (\rho\dot{\mathbf{v}}(0^+), \dot{\mathbf{w}}(0^+)) + a(\mathbf{v}(0^+), \mathbf{w}(0^+)), \end{aligned}$$

and the space  $\mathcal{W}|_{[0,t]}$  denotes the restriction of  $\mathcal{W}$  to the time interval  $[0, t]$ .

Note that the solution of equation (106) is denoted by  $\mathbf{u}_t^{\text{d}}$  emphasizing that there is a different solution for each time  $t$ . Consequently, equation (106) describes a family of problems, one for each time  $t$ .

Analogously as for the derivation of the adjoint problem for the scalar quantity of interest (26), the associated strong form of problem (106), for the functional  $L_{\text{TL}}^{\mathcal{O}}(\cdot)$  defined in (105), is readily derived as

$$\rho(\ddot{\mathbf{u}}_t^{\text{d}} - a_1\dot{\mathbf{u}}_t^{\text{d}}) - \nabla \cdot \boldsymbol{\sigma}_t^{\text{d}} = -\mathbf{f}^{\mathcal{O}} \quad \text{in } \Omega \times [0, t], \quad (107a)$$

$$\mathbf{u}_t^{\text{d}} = \mathbf{0} \quad \text{on } \Gamma_{\text{D}} \times [0, t], \quad (107b)$$

$$\boldsymbol{\sigma}_t^{\text{d}} \cdot \mathbf{n} = -\mathbf{g}^{\mathcal{O}} \quad \text{on } \Gamma_{\text{N}} \times [0, t], \quad (107c)$$

$$\mathbf{u}_t^{\text{d}} = \mathbf{u}^{\mathcal{O}} \quad \text{at } \Omega \times \{t\}, \quad (107d)$$

$$\dot{\mathbf{u}}_t^{\text{d}} = \mathbf{v}^{\mathcal{O}} \quad \text{at } \Omega \times \{t\}, \quad (107e)$$

with the constitutive law

$$\boldsymbol{\sigma}_t^{\text{d}} := \mathbf{C} : \boldsymbol{\varepsilon}(\mathbf{u}_t^{\text{d}} - a_2\dot{\mathbf{u}}_t^{\text{d}}). \quad (108)$$

Recall that the data  $\mathbf{f}^{\mathcal{O}}$ ,  $\mathbf{g}^{\mathcal{O}}$ ,  $\mathbf{u}^{\mathcal{O}}$  and  $\mathbf{v}^{\mathcal{O}}$  enters in the definition of  $L_{\text{TL}}^{\mathcal{O}}(\cdot; t)$  as indicated in (105). Note that for each time  $t$ , problem (107) is of the same type as (26) and therefore has to be integrated backwards in time. Thus, the family of adjoint problems associated with the timeline-dependent quantity  $L_{\text{TL}}^{\mathcal{O}}$  is a family of standard problems in elastodynamics.

For a particular instance of time  $t$ , the error representation of the timeline-dependent quantity of interest  $s^e(t)$  is similar to the standard scalar case but taking the adjoint solution  $\mathbf{u}_t^{\text{d}}$  related with the particular value  $t \in I$ , namely

$$s^e(t) = R_t(\mathbf{u}_t^{\text{d}}), \quad (109)$$

where

$$R_t(\mathbf{w}) := L_t(\mathbf{w}; t) - B_t(\tilde{\mathbf{u}}, \mathbf{w}) \quad \text{and}$$

$$L_t(\mathbf{w}) := \int_0^t l(\tau; \dot{\mathbf{w}}(\tau)) \, d\tau + (\rho\mathbf{v}_0, \dot{\mathbf{w}}(0^+)) + a(\mathbf{u}_0, \mathbf{w}(0^+)).$$

Hence, an estimate for  $s^e(t)$  is obtained injecting an enhanced adjoint approximation  $\tilde{\mathbf{u}}_t^d$  in equation (109)

$$s^e(t) \approx R_t(\tilde{\mathbf{u}}_t^d) =: \tilde{s}^e. \quad (110)$$

Obviously, it is not possible, in practice, to independently compute the infinite solutions  $\tilde{\mathbf{u}}_t^d$  (one for each time  $t \in I$ ) and then using them in equation (109) to assess  $s^e(t)$ . However, taking  $\mathbf{f}^\mathcal{O}$  and  $\mathbf{g}^\mathcal{O}$  constant in time (which accounts for a number of interesting cases), the different functions  $\mathbf{u}_t^d$  corresponding to different time instances are all equivalent after a time translation. Thus, if  $\mathbf{u}_t^d$  is properly computed for a particular value of  $t$ , for instance  $t = T$ , the general functions  $\mathbf{u}_t^d$  for  $t \neq T$  are easily recovered as a direct post-process of  $\mathbf{u}_T^d$ . This fundamental result, shown in the following theorem, is the crucial observation that allows the error estimation technique to be brought to fruition.

**Theorem 5.** *For a given  $t$ , let  $\mathbf{u}_t^d$  be the solution of the adjoint problem defined by equations (107). Assume that data  $\mathbf{f}^\mathcal{O}$  and  $\mathbf{g}^\mathcal{O}$  in (105) are constant in time, i.e.  $\mathbf{f}^\mathcal{O}(\mathbf{x}, t) = \mathbf{f}^\mathcal{O}(\mathbf{x})$  and  $\mathbf{g}^\mathcal{O}(\mathbf{x}, t) = \mathbf{g}^\mathcal{O}(\mathbf{x})$ .*

*Then,  $\mathbf{u}_t^d$  is related with the adjoint solution associated with the final time  $T$ ,  $\mathbf{u}_T^d$ , via the time translation*

$$\mathbf{u}_t^d(\tau) = \mathbf{u}_T^d(\tau + T - t). \quad (111)$$

A proof of this theorem may be found in [71].

Consequently, The adjoint approximations  $\tilde{\mathbf{u}}_t^d$  used in the error estimate (110) are computed applying the time shift (111) to the adjoint approximation  $\tilde{\mathbf{u}}_T^d$  associated with the final time  $T$

$$\tilde{\mathbf{u}}_t^d(\tau) := \tilde{\mathbf{u}}_T^d(\tau + T - t). \quad (112)$$

Thus, only one adjoint approximation  $\tilde{\mathbf{u}}_T^d$  has to be computed and the others are simply recovered by a time shift.

### 7.3 Modal-based adjoint approximation

The error estimate  $\tilde{s}^e(t)$  is computed once the approximation  $\tilde{\mathbf{u}}_T^d \approx \mathbf{u}_T^d$  is available. This section is devoted to the actual computation of  $\tilde{\mathbf{u}}_T^d$ . Note that  $\mathbf{u}_T^d$  coincides with the adjoint solution  $\mathbf{u}^d$  associated with the scalar quantity of interest  $L^\mathcal{O}(\cdot)$ . Consequently, computing  $\tilde{\mathbf{u}}_T^d$  is equivalent to compute an approximation  $\tilde{\mathbf{u}}^d \approx \mathbf{u}^d$ .

Function  $\tilde{\mathbf{u}}^d$  (or equivalently  $\tilde{\mathbf{u}}_T^d$ ) is obtained using the standard approximation techniques for elastodynamics. However, if  $\tilde{\mathbf{u}}^d$  has to be used for a timeline estimate  $\tilde{s}^e(t)$ , then, a better option is using *modal analysis*, see reference [71]. The modal based description of  $\tilde{\mathbf{u}}^d$  simplifies the time shift (111) required to assess the error in the timeline quantity and makes the actual computation of  $\tilde{s}^e(t)$  more efficient.

Approximating function  $\tilde{\mathbf{u}}^d$  with modal analysis requires introducing a semidiscrete version (discrete in space and exact in time) of the adjoint problem (26). The semidiscrete



problem reads: find  $\mathbf{u}^{\text{d},H,p+1}(t) \in \mathcal{V}_0^{H,p+1}$  verifying the final conditions  $\mathbf{u}^{\text{d},H,p+1}(T) = \mathbf{u}^\mathcal{O}$  and  $\dot{\mathbf{u}}^{\text{d},H,p+1}(T) = \mathbf{v}^\mathcal{O}$  and such that for all  $t \in I$

$$m(\ddot{\mathbf{u}}^{\text{d},H,p+1}(t) - a_1 \dot{\mathbf{u}}^{\text{d},H,p+1}(t), \mathbf{w}) + a(\mathbf{u}^{\text{d},H,p+1}(t) - a_2 \dot{\mathbf{u}}^{\text{d},H,p+1}(t), \mathbf{w}) = -l^\mathcal{O}(t; \mathbf{w}), \quad (113)$$

for all test function  $\mathbf{w} \in \mathcal{V}_0^{H,p+1}$ , where  $l^\mathcal{O}(t; \mathbf{w}) := (\mathbf{f}^\mathcal{O}(t), \mathbf{w}) + (\mathbf{g}^\mathcal{O}(t), \mathbf{w})_{\Gamma_N}$  and  $\mathcal{V}_0^{H,p+1}$  is the functional space obtained with  $p$ -refinement of the original functional space  $\mathcal{V}_0^H$ .

**Remark 6.** *The spacial resolution of the adjoint approximation  $\tilde{\mathbf{u}}^{\text{d}}$  has to be richer than the one of the numerical approximation  $\tilde{\mathbf{u}}$ . Otherwise, the error is underestimated when plugging the approximation  $\tilde{\mathbf{u}}^{\text{d}}$  into the residual  $R(\cdot)$  by an effect analogous to Galerkin orthogonality. For that reason the functional space used to define the semidiscrete problem (113) is  $\mathcal{V}_0^{H,p+1}$  instead of  $\mathcal{V}_0^H$ .*

A modal-based approximation of the problem (113) is obtained introducing the generalized eigenvalue problem: find  $(\tilde{\omega}, \tilde{\mathbf{q}}) \in \mathbb{R} \times \mathcal{V}_0^{H,p+1}$  such that

$$a(\tilde{\mathbf{q}}, \mathbf{w}) = (\tilde{\omega})^2 m(\tilde{\mathbf{q}}, \mathbf{w}) \quad \forall \mathbf{w} \in \mathcal{V}_0^{H,p+1}. \quad (114)$$

The  $i$ -th eigenpair solution of this problem is referred as  $(\tilde{\omega}_i, \tilde{\mathbf{q}}_i)$ . Note that the number of eigenpairs is the number of degrees of freedom in the functional space  $\mathcal{V}_0^{H,p+1}$ , denoted by  $N_{\text{dof}}$ . Typically, the eigenpairs are sorted from low to high frequencies, namely  $\tilde{\omega}_1 \leq \tilde{\omega}_2 \leq \dots \leq \tilde{\omega}_{N_{\text{dof}}}$ , and eigenvectors are normalized to be orthonormal with respect the product  $m(\cdot, \cdot)$ , i.e.

$$m(\tilde{\mathbf{q}}_i, \tilde{\mathbf{q}}_j) = \delta_{ij}, \quad 1 \leq i, j \leq N_{\text{dof}}. \quad (115)$$

The complexity of the system of ODEs resulting from (113) is considerably reduced by expressing the adjoint solution  $\mathbf{u}^{\text{d},H,p+1}(\mathbf{x}, t)$  as a combination of the eigenvectors  $\tilde{\mathbf{q}}_i, i = 1, \dots, N_{\text{dof}}$ , that is

$$\mathbf{u}^{\text{d},H,p+1}(\mathbf{x}, t) = \sum_{i=1}^{N_{\text{dof}}} \tilde{\mathbf{q}}_i(\mathbf{x}) \tilde{y}_i(t). \quad (116)$$

Thus, the system of ODEs (113) is transformed into the uncoupled set of scalar ordinary differential equations

$$\ddot{\tilde{y}}_i - [a_1 + a_2(\tilde{\omega}_i)^2] \dot{\tilde{y}}_i + (\tilde{\omega}_i)^2 \tilde{y}_i = \tilde{l}_i, \quad (117a)$$

$$\tilde{y}_i(T) = \tilde{u}_i, \quad (117b)$$

$$\dot{\tilde{y}}_i(T) = \tilde{v}_i, \quad (117c)$$

where the r.h.s. terms  $\tilde{l}_i$ ,  $\tilde{u}_i$  and  $\tilde{v}_i$  are computed using the data characterizing the quantity of interest (105) and the eigenvector  $\tilde{\mathbf{q}}_i$ ,

$$\tilde{l}_i(t) := (\mathbf{f}^\mathcal{O}(t), \tilde{\mathbf{q}}_i) + (\mathbf{g}^\mathcal{O}(t), \tilde{\mathbf{q}}_i)_{\Gamma_N}, \quad u_i := m(\mathbf{u}^\mathcal{O}, \tilde{\mathbf{q}}_i) \quad \text{and} \quad v_i := m(\mathbf{v}^\mathcal{O}, \tilde{\mathbf{q}}_i). \quad (118)$$

The cost of modal analysis scales as, see references [72, 73, 74],

$$\mathcal{O}(N_{\text{dof}} \cdot N_{\text{bw}}^2) + \mathcal{O}(N_{\text{dof}}^2 \cdot N_{\text{bw}}) + \mathcal{O}(N_{\text{dof}}^3),$$

where  $N_{\text{bw}}$  denotes the half-bandwidth of the finite element matrices associated with the functional space  $\mathcal{V}_0^{H,p+1}$ . Thus, the modal-based approach is not computationally affordable unless the modal description (116) is truncated up to the first  $M$  terms, being  $M \ll N_{\text{dof}}$ . Consequently, the adjoint approximation  $\tilde{\mathbf{u}}^{\text{d}}$  is defined as the truncated expansion

$$\tilde{\mathbf{u}}^{\text{d}}(\mathbf{x}, t) := \sum_{i=1}^M \tilde{\mathbf{q}}_i(\mathbf{x}) \tilde{y}_i(t). \quad (119)$$

Note that the number of required vibration modes  $M$  has to be selected such that the truncated high frequency modes (for  $i > M$ ) are negligible in (116). That is, such that  $\tilde{\mathbf{u}}^{\text{d}}$  is a good approximation to  $\mathbf{u}^{\text{d},H}$ . This is equivalent to assume that for  $i > M$  the values of  $\tilde{l}_i$ ,  $\tilde{u}_i$  and  $\tilde{v}_i$ , as defined in (118), are close to zero, and consequently  $\tilde{y}_i(t) \approx 0$ . This is guaranteed if the data  $\mathbf{f}^{\mathcal{O}}$ ,  $\mathbf{g}^{\mathcal{O}}$ ,  $\mathbf{u}^{\mathcal{O}}$  and  $\mathbf{v}^{\mathcal{O}}$  are well captured by the expansion of the first  $M$  eigenvectors.

Once  $\tilde{\mathbf{u}}^{\text{d}}$  (or equivalently  $\tilde{\mathbf{u}}_T^{\text{d}}$ ) is available, the adjoint family  $\tilde{\mathbf{u}}_t^{\text{d}}$  is recovered using the time shift (111). Then,  $\tilde{\mathbf{u}}_t^{\text{d}}$  is plugged in equation (110) furnishing the timeline error estimate  $\tilde{s}^e(t)$ .

**Remark 7.** (*Illustrative example*) This example illustrates the performance of the error estimate  $\tilde{s}^e(t)$ . The computational domain is the three dimensional structure plotted in figure 10 which is clamped at the supports and it is loaded with the time-dependent traction

$$\mathbf{g}(t) = \begin{cases} -g(t)\mathbf{e}_1 & \text{on } \Gamma_g, \\ \mathbf{0} & \text{elsewhere,} \end{cases}$$

where function  $g(t)$  is defined in figure 10 and the values  $g_{\text{max}} = 1 \cdot 10^3$  Pa and  $t_g = 1 \cdot 10^{-3}$  s are considered. The set  $\Gamma_g$  is the boundary where the load is applied, see figure 10. The structure is initially at rest ( $\mathbf{u}_0 = \mathbf{v}_0 = \mathbf{0}$ ) and the body force is zero ( $\mathbf{f} = \mathbf{0}$ ). The material properties are Young's modulus  $E = 2 \cdot 10^{10}$  Pa, Poisson's ratio  $\nu = 0.2$ , density  $\rho = 2.4 \cdot 10^3$  kg/m<sup>3</sup> and viscosity  $a_1 = a_2 = 0$ . The final time is  $T = 0.02$  s .

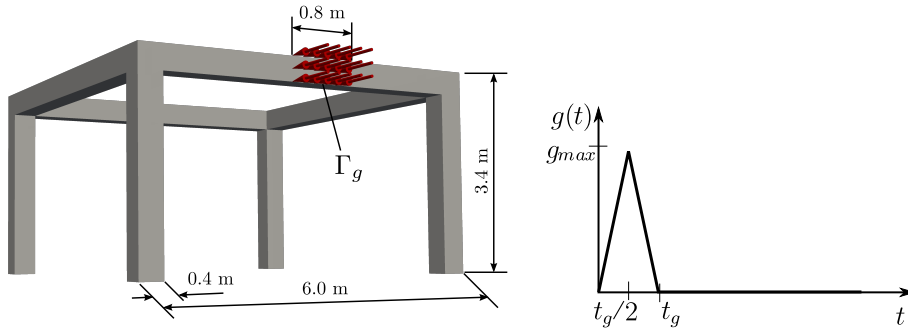


Figure 10: Problem geometry (left) and time description of the external load (right).

This example focuses in the timeline-dependent quantity of interest

$$s(t) := \frac{1}{\text{meas}(\Gamma_g)} (\mathbf{e}_1, \mathbf{u}(t))_{\Gamma_g},$$

which is the average of the  $x$ -component of the displacement in the boundary  $\Gamma_g$  at every time  $t \in I$ .

The problem is discretized with trilinear hexahedra in space and with the Newmark method in time with parameters  $\beta = 1/4$  and  $\gamma = 1/2$ . The approximated quantity of interest  $\tilde{s}(t) = L^{\mathcal{O}}(\tilde{\mathbf{u}}; t)$  is computed from the approximate solution  $\tilde{\mathbf{u}}$  obtained with the coarse finite element mesh plotted in figure 11 and with  $N = 400$  time steps. The reference quantity of interest  $s(t) = L^{\mathcal{O}}(\mathbf{u}; t)$  is obtained by assuming that the exact solution  $\mathbf{u}$  is fairly replaced by a reference solution obtained using the reference mesh in figure 11 and  $N = 1600$  time steps. The error in the quantity of interest is evaluated using the reference solution, namely  $s^e(t) = s(t) - \tilde{s}(t)$ . Finally, the error estimate  $\tilde{s}^e(t)$  is computed using up to  $M = 60$  vibration modes for approximating the adjoints.

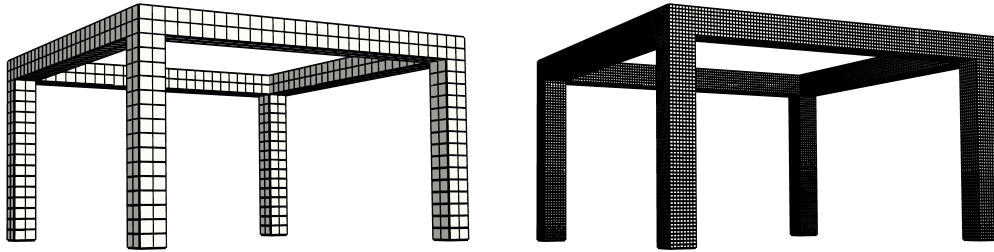


Figure 11: Coarse (left) and reference (right) meshes used in this example with 334 and 22016 elements respectively.

Figure 12 shows the computed and reference timeline-dependent quantities,  $\tilde{s}(t)$  and  $s(t)$ , along with the assessed and reference errors,  $\tilde{s}^e(t)$  and  $s^e(t)$ . Note that the quality of the error estimate  $\tilde{s}^e(t)$  increases with the number of vibration modes. For  $M = 60$  modes, the error estimate  $\tilde{s}^e(t)$  and the reference error  $s^e(t)$  are in very good agreement.

## 8 Closure

The most significant error assessment techniques for structural transient dynamics are reviewed, namely: recovery-based estimates, dual weighted residuals, constitutive relation error and error assessment for timeline-dependent quantities of interest.

The recovery-based estimates for transient dynamics are an extension of the recovery procedures available for steady state linear elasticity. The classical space recovery allows assessing only the space discretization error. Thus, to carry out adaptive procedures, the time discretization errors have to be accounted independently. Moreover, the standard stress recovery techniques are not sufficient to assess the kinetic energy of the error. Thus, a specific recovery procedure is also introduced for the velocities.

The dual weighted residuals approach produces accurate approximations to the error in the quantity of interest and also provides local error indicators for mesh adaptivity. The error estimate is obtained by plugging an enhanced approximation of the adjoint

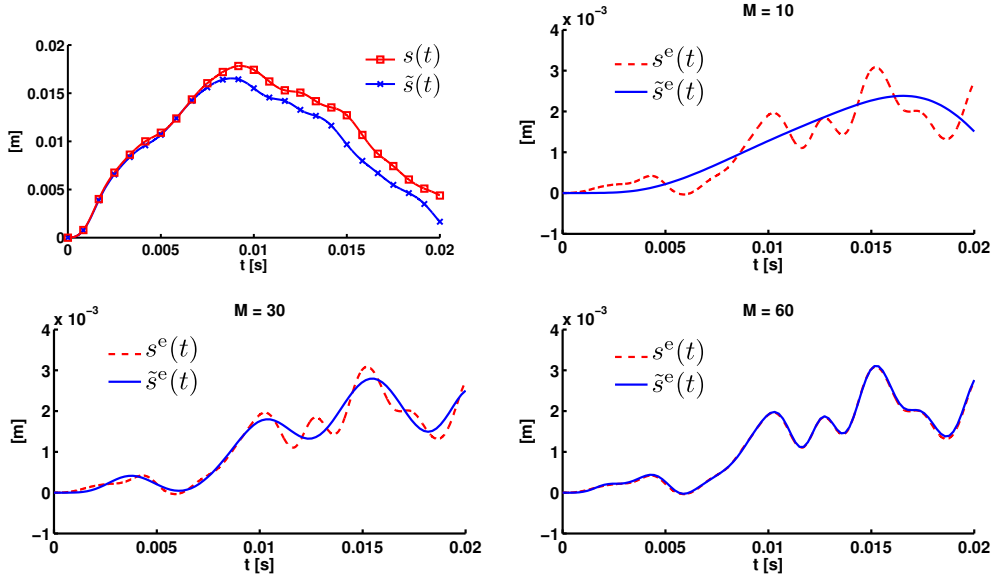


Figure 12: Approximated quantity of interest  $\tilde{s}(t)$  and reference quantity  $s(t)$  (top, left). Reference and assessed errors,  $s^e(t)$  and  $\tilde{s}^e(t)$ , for three different number of vibration modes for approximating the adjoints,  $M = 10$  (top, right),  $M = 30$  (bottom, left) and  $M = 60$  (bottom, right).

problem into the space-time weak residual associated with the numerical solution. This technique accounts for both the space and time discretization errors and it is used to adapt both space and time grids.

The constitutive relation error estimates furnish bounds of the error both in an energy measure and in the quantity of interest. The extension of this technique to elastodynamics is based in a key hypothesis: the formulation contains a certain amount of damping. Thus, the computed bounds degenerate as the value of the damping tends to zero. Computing the error bounds requires obtaining admissible stress fields for both the original and the adjoint problems.

Finally, an error estimate for the so-called timeline-dependent quantities of interest is described. This kind of quantities are scalar time-dependent functions and are specially well suited to analyze the outcome of transient problems. Although at the first sight this type of quantities require characterizing a family of adjoint problems, approximating the adjoint solution with a modal approach constitutes an efficient and affordable tool to assess them.

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## Paper B

# Computable bounds of functional outputs in linear visco-elastodynamics

F. Verdugo and P. Díez

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## Paper C

# Modal based goal-oriented error assessment for timeline-dependent quantities in transient dynamics

F. Verdugo, N. Parés and P. Díez

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Paper D

Goal-oriented space-time  
adaptivity for transient dynamics  
using a modal description  
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F. Verdugo, N. Parés and P. Díez

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## Goal-oriented space-time adaptivity for transient dynamics using a modal description of the adjoint solution\*

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### Abstract

This article presents a space-time adaptive strategy for transient elastodynamics. The method aims at computing an optimal space-time discretization such that the computed solution has an error in the quantity of interest below a user-defined tolerance. The methodology is based on a goal-oriented error estimate that requires accounting for an auxiliary adjoint problem. The major novelty of this paper is using modal analysis to obtain a proper approximation of the adjoint solution. The idea of using a modal-based description was introduced in a previous work for error estimation purposes. Here this approach is used for the first time in the context of adaptivity. With respect to the standard direct time-integration methods, the modal solution of the adjoint problem is highly competitive in terms of computational effort and memory requirements. The performance of the proposed strategy is tested in two numerical examples. The two examples are selected to be representative of different wave propagation phenomena, one being a 2D bulky continuum and the second a 2D domain representing a structural frame.

**Keywords:** elastodynamics, adaptivity, goal-oriented error assessment, adjoint problem, quantity of interest, modal analysis.

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## 1 Introduction

Computing high fidelity numerical approximations requires a fine discretization and leads to a large consumption of computational resources. Adaptivity aims at providing the optimal discretization (space mesh and time grid) guaranteeing some user-prescribed accuracy at a minimum computational cost. Many adaptive techniques have been developed with application to different problem types. These tools are particularly important in wave propagation problems, e.g. linear elastodynamics, because the features of the solution concentrate at the wave fronts and therefore a fine mesh is only required at specific regions of the domain.

Over the last three decades, a vast literature has been produced on adaptivity. Among the the pioneering works, references [1, 2] propose adaptive techniques for flow problems using curvature and gradient based error indicators. This type of heuristic error indicators are used to identify the parts of the solution requiring a finer mesh size. This approach is applicable to many problem types because error indicators do not rely on the problem properties, but in the geometrical features of the solution. This type of indicators detect properly the errors associated with inter-

polation but fail in capturing the error from other sources, e.g. pollution error.

A more reliable alternative to drive mesh adaptivity are a posteriori error estimators. They are used to efficiently control the accuracy of some output of the solution by means of refining the discretization only where is needed (in the zones where the error is emanating from). The available outputs for assessing the accuracy of the approximation are global norms, e.g. the energy or  $L^2$  norm [3, 4, 5], or *quantities of interest* [6, 7, 8, 9]. Error estimators considering quantities of interest are referred as *goal-oriented*.

Goal-oriented adaptivity is discussed in the literature for many problem types. For instance, for elliptic problems [6, 7, 10, 11, 12], for the convection-diffusion-reaction equation [13, 14], for non-linear structural problems [15, 16], for time-dependent parabolic problems [17, 18, 19] and for elastodynamics (or other 2nd order hyperbolic problems) [20, 21, 22, 23].

Goal-oriented adaptivity for elastodynamics is a very challenging topic and it is still ongoing research. The main difficulties are 1) solving the associated space-time adjoint solution accurately to estimate the error in the quantity of interest, 2) splitting the contributions of the space and time discretization errors and 3) transferring the solution

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from one mesh to another without loss of accuracy.

References [20, 21, 23] are among the few discussing goal-oriented adaptivity in elastodynamics. The input of the adaptive procedure is a desired error tolerance in some quantity of interest. The adjoint solution is computed with the same time-integration method as the original solution. This approach might be memory demanding because at least the original or the adjoint solution has to be stored as a whole (at each mesh node and time point) prior to evaluate the error estimate.

The adaptive strategy presented in this article is an alternative to the previous approach. Here, the adjoint problem is approximated using modal analysis, as suggested in reference [24], to preclude the costly adjoint approximation and storage. The modal-based adjoint approximation is particularly efficient for some quantities of interest. This is because the adjoint solution is stored for a few vibration modes instead that for all time steps. Moreover, the time description of the adjoint solution is known analytically once the vibration frequencies and modes are available. This simplifies the algorithmic complexity of the adaptive procedure.

The remainder of this paper is organized as follows. Section 2 presents the equations of elastodynamics. Section 3 presents the weak and discrete versions of the problem using the double field time-continuous Galerkin method. The modal-based error assessment approach is presented in section 4. Section 5 presents the space-time adaptive procedure. Finally, the methodology is illustrated in section 6 with two numerical examples. The paper is concluded with some remarks.

## 2 Problem statement

### 2.1 Governing equations

A visco-elastic body occupies an open bounded domain  $\Omega \subset \mathbb{R}^d$ ,  $d \leq 3$ , with boundary  $\partial\Omega$ . The boundary is divided in two disjoint parts,  $\Gamma_N$  and  $\Gamma_D$  such that  $\partial\Omega = \bar{\Gamma}_N \cup \bar{\Gamma}_D$  and the considered time interval is  $I := (0, T]$ . Under the assumption of small perturbations, the evolution of displacements  $\mathbf{u}(\mathbf{x}, t)$  and stresses  $\boldsymbol{\sigma}(\mathbf{x}, t)$ ,  $\mathbf{x} \in \Omega$  and  $t \in I$ , is described by the visco-elastodynamic equations,

$$\rho(\ddot{\mathbf{u}} + a_1 \dot{\mathbf{u}}) - \nabla \cdot \boldsymbol{\sigma} = \mathbf{f} \quad \text{in } \Omega \times I, \quad (1a)$$

$$\mathbf{u} = \mathbf{0} \quad \text{on } \Gamma_D \times I, \quad (1b)$$

$$\boldsymbol{\sigma} \cdot \mathbf{n} = \mathbf{g} \quad \text{on } \Gamma_N \times I, \quad (1c)$$

$$\mathbf{u} = \mathbf{u}_0 \quad \text{at } \Omega \times \{0\}, \quad (1d)$$

$$\dot{\mathbf{u}} = \mathbf{v}_0 \quad \text{at } \Omega \times \{0\}, \quad (1e)$$

where an upper dot indicates derivation with respect to time, that is  $(\dot{\bullet}) := \frac{d}{dt}(\bullet)$ , and  $\mathbf{n}$  denotes the outward unit normal to  $\partial\Omega$ . The input data includes the mass density  $\rho = \rho(\mathbf{x}) > 0$ , the first Rayleigh coefficient  $a_1 \geq 0$ , the body force  $\mathbf{f} = \mathbf{f}(\mathbf{x}, t)$  and the traction  $\mathbf{g} = \mathbf{g}(\mathbf{x}, t)$  acting on the Neumann boundary  $\Gamma_N \times I$ . The initial conditions

for displacements and velocities are  $\mathbf{u}_0 = \mathbf{u}_0(\mathbf{x})$  and  $\mathbf{v}_0 = \mathbf{v}_0(\mathbf{x})$  respectively. For the sake of simplicity and without any loss of generality, Dirichlet conditions (1b) are taken as homogeneous.

The set of equations (1) is closed with the constitutive law,

$$\boldsymbol{\sigma} := \mathbf{C} : \boldsymbol{\varepsilon}(\mathbf{u} + a_2 \dot{\mathbf{u}}), \quad (2)$$

where the parameter  $a_2 \geq 0$  is the second Rayleigh coefficient, the tensor  $\mathbf{C}$  is the standard 4th-order elastic Hooke tensor. The strains are given by the kinematic relation corresponding to small perturbations, that is  $\boldsymbol{\varepsilon}(\mathbf{w}) := \frac{1}{2}(\nabla \mathbf{w} + \nabla^T \mathbf{w})$ .

### 2.2 Numerical approximation

In order to properly split the space and time error components, the adaptive strategy presented in this paper requires that the numerical solution under consideration fulfills the discrete version of a variational formulation. Thus, a weak residual (integrated both in space and time) associated with the numerical solution is readily introduced. The splitting procedure uses the fact that the residual vanishes for the functions in the test space, that is Galerkin orthogonality holds.

Among the possible space-time variational formulations available for transient elastodynamics, the double field *time-continuous Galerkin method* [25, 20] is the numerical solver selected. Note however that the rationale of this article can be easily extended to other space-time variational formulations, for instance, the one proposed by Johnson [26] or the one proposed by Hulbert and Hughes [27, 28].

The definition of the weak form of the problem requires introducing the following functional spaces: the standard Sobolev space associated with static displacement fields

$$\mathbf{V}_0 := \{\mathbf{w} \in [H^1(\Omega)]^d : \mathbf{w} = \mathbf{0} \text{ on } \Gamma_D\} \quad (3)$$

and the Bochner space  $L^2(I; \mathbf{V}_0)$  associated with  $\mathbf{V}_0$  of square-integrable functions from  $I$  into  $\mathbf{V}_0$ . With these notations, the trial space  $\mathbf{W}$  for the double field time-continuous Galerkin method is defined as

$$\mathbf{W} := \{\mathbf{w} \in L^2(I; \mathbf{V}_0) : \dot{\mathbf{w}} \in L^2(I; \mathbf{V}'_0)\}.$$

Note that,  $\mathbf{w} \in \mathbf{W}$  implies that  $\mathbf{w} \in C^0(\bar{I}; [L^2(\Omega)]^d)$  and therefore functions in  $\mathbf{W}$  are continuous both in space and time, but they do not necessarily have a continuous time derivative.

The test space is associated with a partition of the time interval  $I$  defined as  $\mathcal{T} := \{t_0, t_1, \dots, t_N\}$ , with  $0 = t_0 < t_1 < \dots < t_N = T$ . The time points in  $\mathcal{T}$  define the time intervals  $I_n := (t_{n-1}, t_n]$ ,  $n = 1, \dots, N$ . The time step length for each interval is  $\Delta t_n := t_n - t_{n-1}$ ,  $n = 1, \dots, N$  and the characteristic time step length for the partition  $\mathcal{T}$  is  $\Delta t := \max_{1 \leq n \leq N} (\Delta t_n)$ .

The test space is defined as

$$\widehat{\mathbf{W}} := \{\mathbf{w} \in L^2(I; \mathbf{V}_0) : \mathbf{w}|_{I_n} \in L^2(I_n; \mathbf{V}_0) \text{ and } \dot{\mathbf{w}}|_{I_n} \in L^2(I_n; \mathbf{V}'_0), n = 1, \dots, N\}.$$

Functions in  $\widehat{\mathcal{W}}$  when restricted to a time interval  $I_n$  have the same regularity as functions in  $\mathcal{W}$ . However, functions in  $\widehat{\mathcal{W}}$  are allowed to be discontinuous-in-time at the points in  $\mathcal{T}$ . This property is needed to define a time marching scheme, computing the solution successively in each time interval.

Using these notations, the space-time weak form of problem (1) reads: find  $\mathbf{U} = [\mathbf{u}_u, \mathbf{u}_v] \in \mathcal{W} \times \mathcal{W}$  such that

$$B(\mathbf{U}, \mathbf{W}) = L(\mathbf{W}) \quad \forall \mathbf{W} := [\mathbf{w}_u, \mathbf{w}_v] \in \widehat{\mathcal{W}} \times \widehat{\mathcal{W}}, \quad (4)$$

where the bilinear form  $B(\cdot, \cdot)$  and the linear functional  $L(\cdot)$  are defined as

$$\begin{aligned} B(\mathbf{U}, \mathbf{W}) &:= \int_I m(\dot{\mathbf{u}}_v + a_1 \mathbf{u}_v, \mathbf{w}_v) dt \\ &\quad + \int_I a(\mathbf{u}_u + a_2 \mathbf{u}_v, \mathbf{w}_v) dt + m(\mathbf{u}_v(0), \mathbf{w}_v(0)) \\ &\quad + \int_I a(\dot{\mathbf{u}}_u - \mathbf{u}_v, \mathbf{w}_u) dt + a(\mathbf{u}_u(0), \mathbf{w}_u(0)), \\ L(\mathbf{W}) &:= \int_I l(t; \mathbf{w}_v) dt \\ &\quad + a(\mathbf{u}_0, \mathbf{w}_u(0)) + m(\mathbf{v}_0, \mathbf{w}_v(0)), \end{aligned}$$

where

$$\begin{aligned} a(\mathbf{v}, \mathbf{w}) &:= \int_{\Omega} \boldsymbol{\varepsilon}(\mathbf{v}) : \mathcal{C} : \boldsymbol{\varepsilon}(\mathbf{w}) d\Omega, \\ m(\mathbf{v}, \mathbf{w}) &:= \int_{\Omega} \rho \mathbf{v} \cdot \mathbf{w} d\Omega, \\ l(t; \mathbf{w}) &:= (\mathbf{f}(t), \mathbf{w}) + (\mathbf{g}(t), \mathbf{w})_{\Gamma_N}, \end{aligned}$$

and

$$(\mathbf{v}, \mathbf{w}) := \int_{\Omega} \mathbf{v} \cdot \mathbf{w} d\Omega, \quad (\mathbf{v}, \mathbf{w})_{\Gamma_N} := \int_{\Gamma_N} \mathbf{v} \cdot \mathbf{w} d\Gamma.$$

The weak problem (4) is a *double field* formulation, having two unknowns, displacements  $\mathbf{u}_u$  and velocities  $\mathbf{u}_v$ , which are a priori independent. That is, the velocity  $\mathbf{u}_v$  is not strongly enforced to coincide with  $\dot{\mathbf{u}}_u$ . However, the relation between displacements and velocities is weakly imposed by means of the term  $a(\dot{\mathbf{u}}_u - \mathbf{u}_v, \mathbf{w}_u)$ .

The initial conditions (1d) and (1e) are also weakly imposed introducing the terms  $a(\mathbf{u}_u(0) - \mathbf{u}_0, \mathbf{w}_u(0))$  and  $m(\mathbf{u}_v(0) - \mathbf{v}_0, \mathbf{w}_v(0))$  respectively. The weak problem (4) is consistent with the original strong problem (1) in the sense that the solution  $\mathbf{u}$  of problem (1) fulfills

$$B([\mathbf{u}, \dot{\mathbf{u}}], \mathbf{W}) = L(\mathbf{W}) \quad \forall \mathbf{W} \in \widehat{\mathcal{W}} \times \widehat{\mathcal{W}}.$$

The fully discrete version of problem (4) requires introducing a finite element partition of the domain  $\Omega$ , which in the framework of mesh adaptivity is allowed to be different at each time point in  $\mathcal{T}$ . The finite element mesh discretizing the spatial domain  $\Omega$  associated with time  $t_n \in \mathcal{T}$  is denoted in the following by  $\mathcal{P}_n$ . The associated finite element space of continuous, elementwise polynomials of degree  $p$  is referred as  $\mathcal{V}_0^H(\mathcal{P}_n) \subset \mathcal{V}_0$ . The notation emphasizing the dependence on  $\mathcal{P}_n$  highlights the fact that the finite

element space depends on the computational mesh. The upper-script  $H$  stands for the characteristic element size in the mesh and it is included in the notation to indicate the discrete character of the finite element space. In the case that different values of  $p$  have to be accounted for, the notation is completed adding  $p$  as upper-script, e.g. the spaces  $\mathcal{V}_0^{H,p}(\mathcal{P}_n)$  and  $\mathcal{V}_0^{H,p+1}(\mathcal{P}_n)$  are also used in the following.

The space meshes  $\mathcal{P}_n$  are built considering a hierarchical tree-based mesh refinement strategy [29, 30, 31]. In this framework, the computational meshes are obtained recursively splitting the elements of an initial background mesh denoted as  $\mathcal{P}_{\text{bg}}$  as shown in figure 1. Thus,  $\mathcal{V}_0^H(\mathcal{P}_{\text{bg}}) \subset \mathcal{V}_0^H(\mathcal{P}_n)$  for all the spatial meshes  $n = 0, \dots, N$ .

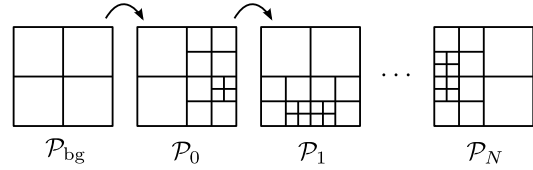


Figure 1: A hierarchical tree-based technique is used to build the space meshes  $\mathcal{P}_n$ ,  $n = 0, \dots, N$  from the background mesh  $\mathcal{P}_{\text{bg}}$ .

The tree-based structure enormously facilitates the mesh refinement and unrefinement operations as well as the data transfer between different meshes. However, this approach requires dealing with a conforming approximation on an irregular spatial meshes involving *hanging* or *irregular* nodes. A constrained finite element approximation is used to enforce the continuity of the finite element solution across the edges of the mesh containing hanging nodes (introducing constraints on the local basis functions). A detailed description is given in appendix A.

The fully discrete problem is obtained replacing in (4) the trial and test spaces  $\mathcal{W}$  and  $\widehat{\mathcal{W}}$  by their discrete counterparts. For the sake of simplicity and without loss of generality, the method is presented here for piecewise linear (in time) trial functions. Hence, the time dependence of the approximations for displacements and velocities corresponds to a linear interpolation inside the time intervals  $I_n$  (piecewise linear in  $I$ ). The space dependence is inherited from the spaces  $\mathcal{V}_0^H(\mathcal{P}_n)$ . The resulting discrete space-time functional spaces read

$$\begin{aligned} \mathcal{W}_u^{H,\Delta t} &:= \{\mathbf{w} \in \mathcal{W} : \mathbf{w}(0) = \mathbf{u}_0, \\ \mathbf{w}(t) &= \sum_{n=0}^N \theta_n(t) \mathbf{w}(t_n), \\ \mathbf{w}(t_n) &\in \mathcal{V}_0^H(\mathcal{P}_n), \quad n = 0, \dots, N\}, \end{aligned}$$



and

$$\begin{aligned} \mathcal{W}_v^{H,\Delta t} &:= \{\mathbf{w} \in \mathcal{W} : \mathbf{w}(0) = \mathbf{v}_0, \\ \mathbf{w}(t) &= \sum_{n=0}^N \theta_n(t) \mathbf{w}(t_n), \\ \mathbf{w}(t_n) &\in \mathcal{V}_0^H(\mathcal{P}_n), n = 0, \dots, N\}, \end{aligned}$$

where  $\theta_n(t)$  are the linear shape functions associated with the time grid  $\mathcal{T}$ . Note that functions in  $\mathcal{W}_u^{H,\Delta t}$  and  $\mathcal{W}_v^{H,\Delta t}$  are continuous piecewise polynomials fulfilling the initial conditions for displacements and velocities respectively. Functions  $\mathbf{w} \in \mathcal{W}_u^{H,\Delta t}$  are such that at the points of the time grid,  $t_n \in \mathcal{T}$ , they belong to one of the standard Finite Element spaces, namely  $\mathbf{w}(t_n) \in \mathcal{V}_0^H(\mathcal{P}_n)$ . At an intermediate time  $t \in I_n$ ,  $t \neq t_n$ , function  $\mathbf{w}(t)$  belongs to  $\mathcal{V}_0^H(\mathcal{P}_{n-1}) + \mathcal{V}_0^H(\mathcal{P}_n)$ , that is the space generated by the superposition of the two meshes  $\mathcal{P}_n$  and  $\mathcal{P}_{n+1}$ , see figure 2. The same holds for functions in  $\mathcal{W}_v^{H,\Delta t}$ .

The fully discrete test space  $\widehat{\mathcal{W}}^{H,\Delta t}$  is defined as

$$\widehat{\mathcal{W}}^{H,\Delta t} := \{\mathbf{w} \in \widehat{\mathcal{W}} : \mathbf{w}|_{I_n} \in \mathbb{P}^0(I_n; \mathcal{V}_0^H(\mathcal{P}_n)), \\ n = 1, \dots, N\},$$

where  $\mathbb{P}^0(I_n; \mathcal{V}_0^H(\mathcal{P}_n))$  denotes the space of constant functions taking values in  $I_n$  and returning a value in  $\mathcal{V}_0^H(\mathcal{P}_n)$ . Functions in  $\widehat{\mathcal{W}}^{H,\Delta t}$  are continuous piecewise polynomials in space and piecewise constants in time. Function  $\mathbf{w} \in \widehat{\mathcal{W}}^{H,\Delta t}$  is such that, for a time  $t \in I_n$ ,  $\mathbf{w}(t) \in \mathcal{V}_0^H(\mathcal{P}_n)$ , see figure 2. The polynomial dependence in time of functions in  $\widehat{\mathcal{W}}^{H,\Delta t}$  is one degree lower (piecewise constants) than the polynomial dependence in time of the trial space  $\mathcal{W}_u^{H,\Delta t}$  (piecewise linear). In this case, the trial and test spaces have the same number of degrees of freedom.

Using the discrete trial and test spaces, the fully discrete problem reads: find  $\widetilde{\mathbf{U}} := [\widetilde{\mathbf{u}}_u, \widetilde{\mathbf{u}}_v] \in \mathcal{W}_u^{H,\Delta t} \times \mathcal{W}_v^{H,\Delta t}$  such that

$$B(\widetilde{\mathbf{U}}, \mathbf{W}) = L(\mathbf{W}) \quad \forall \mathbf{W} \in \widehat{\mathcal{W}}^{H,\Delta t} \times \widehat{\mathcal{W}}^{H,\Delta t}. \quad (5)$$

Problem (5) is integrated over the whole space-time domain  $\Omega \times I$ . However, having selected discontinuous test functions results in a time marching scheme that solves successively  $N$  problems in the time slabs  $\Omega \times I_n$ ,  $n = 1, \dots, N$ . Note that the step by step computational methodology resembles the classical time integration methods based on finite differences (i.e. Crank Nicholson, Newmark, etc.). In fact, if the mesh does not change, then the discrete displacements and velocities  $\widetilde{\mathbf{u}}_u$  and  $\widetilde{\mathbf{u}}_v$  at times  $t_n$ ,  $n = 1, \dots, N$ , coincide with the approximation given by the Newmark method with parameters  $\beta = 1/4$  and  $\gamma = 1/2$ , see [20] for a detailed proof. The actual resolution of problem (5) is detailed in appendix A.

### 2.3 Discretization error and error equation

The discretization error associated with  $\widetilde{\mathbf{U}}$  is defined as

$$\mathbf{E} := \mathbf{U} - \widetilde{\mathbf{U}} = [\mathbf{e}_u, \mathbf{e}_v] = [\mathbf{u} - \widetilde{\mathbf{u}}_u, \dot{\mathbf{u}} - \widetilde{\mathbf{u}}_v] \in \mathcal{W} \times \mathcal{W},$$

where  $\mathbf{e}_u$  and  $\mathbf{e}_v$  are the errors in displacements and velocities respectively. The error  $\mathbf{E}$  fulfills the following residual equation: find  $\mathbf{E} = [\mathbf{e}_u, \mathbf{e}_v] \in \mathcal{W} \times \mathcal{W}$  such that

$$B(\mathbf{E}, \mathbf{W}) = R(\mathbf{W}) := L(\mathbf{W}) - B(\widetilde{\mathbf{U}}, \mathbf{W}) \\ \forall \mathbf{W} \in \widehat{\mathcal{W}} \times \widehat{\mathcal{W}}, \quad (6)$$

which is derived replacing the exact solution  $\mathbf{U}$  by  $\widetilde{\mathbf{U}} + \mathbf{E}$  in (4) and using linearity of the forms  $B(\cdot, \cdot)$  and  $L(\cdot)$ .

The residual  $R(\cdot)$  fulfills the Galerkin orthogonality property

$$R(\mathbf{W}) = 0 \quad \text{for all } \mathbf{W} \in \widehat{\mathcal{W}}^{H,\Delta t} \times \widehat{\mathcal{W}}^{H,\Delta t}. \quad (7)$$

Although the Galerkin orthogonality property of the residual  $R(\cdot)$  is not necessary to derive an error estimate for the error in the quantity of interest, it is required in the space-time adaptive strategy in order to properly split the space and time error contributions.

## 3 Goal-oriented modal-based error assessment

### 3.1 Quantity of interest and adjoint problem

The proposed a posteriori error estimation adaptive strategy aims at assessing and controlling the discretization error  $\mathbf{E}$  measured using some specific quantity of interest. The quantity of interest is defined by means of a bounded linear functional  $L^\mathcal{O} : \mathcal{W} \times \mathcal{W} \rightarrow \mathbb{R}$  which extracts a single representative scalar value of the whole space-time solution, namely

$$L^\mathcal{O}(\mathbf{W}) := L_u^\mathcal{O}(\mathbf{w}_u) + L_v^\mathcal{O}(\mathbf{w}_v), \quad (8)$$

where  $L_u^\mathcal{O} : \mathcal{W} \rightarrow \mathbb{R}$  and  $L_v^\mathcal{O} : \mathcal{W} \rightarrow \mathbb{R}$  are linear functionals representing quantities of interest for displacements and velocities respectively.

The estimation of the value  $s^e := L^\mathcal{O}(\mathbf{E})$  requires introducing an auxiliary problem associated with the functional  $L^\mathcal{O}(\cdot)$ , usually denoted by *adjoint* or *dual* problem. The variational form of the adjoint problem reads: find  $\mathbf{U}^d := [\mathbf{u}_u^d, \mathbf{u}_v^d] \in \mathcal{W} \times \mathcal{W}$  such that

$$B(\mathbf{W}, \mathbf{U}^d) = L^\mathcal{O}(\mathbf{W}) \quad \forall \mathbf{W} \in \widehat{\mathcal{W}} \times \widehat{\mathcal{W}}. \quad (9)$$

The adjoint solution characterizes the quantity of interest  $L^\mathcal{O}(\cdot)$  in the sense that, if  $\mathbf{U}^d$  is available, then the functional  $L^\mathcal{O}(\cdot)$  coincides with  $B(\cdot, \mathbf{U}^d)$ , and in particular the computable quantity  $L(\mathbf{U}^d)$  is equal to the quantity of interest  $L^\mathcal{O}(\mathbf{U})$ .

In practice, the functional  $L^\mathcal{O}(\cdot)$  is selected with the same structure as  $L(\cdot)$ , namely

$$L_u^\mathcal{O}(\mathbf{w}_u) := a(\mathbf{u}^\mathcal{O}, \mathbf{w}_u(T)) \quad \text{and} \quad (10a)$$

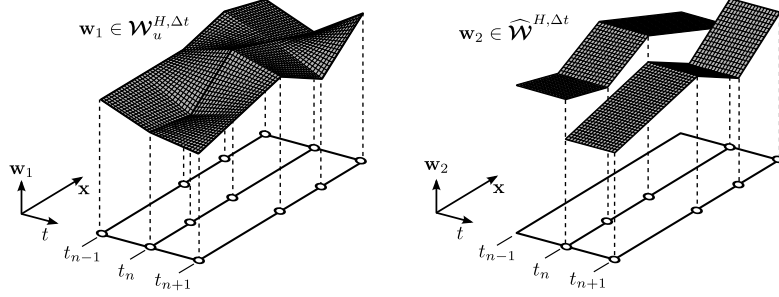


Figure 2: Illustration of two generic functions,  $\mathbf{w}_1 \in \mathcal{W}_u^{H, \Delta t}$  (left) and  $\mathbf{w}_2 \in \widehat{\mathcal{W}}^{H, \Delta t}$  (right), inside the time interval  $(t_{n-1}, t_{n+1}]$  when the time points  $t_{n-1}$ ,  $t_n$  and  $t_{n+1}$  have different computational meshes. The active nodes in meshes  $\mathcal{P}_{n-1}$ ,  $\mathcal{P}_n$  and  $\mathcal{P}_{n+1}$  are marked with circles ( $\circ$ ) on the  $x$ - $t$  plane.

$$L_v^{\mathcal{O}}(\mathbf{w}_v) := \int_0^T (\mathbf{f}^{\mathcal{O}}(t), \mathbf{w}_v(t)) dt + \int_0^T (\mathbf{g}^{\mathcal{O}}(t), \mathbf{w}_v(t))_{\Gamma_N} dt + m(\mathbf{v}^{\mathcal{O}}, \mathbf{w}_v(T)), \quad (10b)$$

where  $\mathbf{f}^{\mathcal{O}}$ ,  $\mathbf{g}^{\mathcal{O}}$ ,  $\mathbf{v}^{\mathcal{O}}$  and  $\mathbf{u}^{\mathcal{O}}$  are the data characterizing the quantity of interest. The functions  $\mathbf{f}^{\mathcal{O}}$  and  $\mathbf{g}^{\mathcal{O}}$  extract global or localized averages of velocities in  $\Omega$  and  $\Gamma_N$ , respectively, integrated over the whole time interval  $[0, T]$ . The fields  $\mathbf{v}^{\mathcal{O}}$  and  $\mathbf{u}^{\mathcal{O}}$  play the role of weighting functions to compute averages of velocities and strains at the final simulation time  $T$ .

For the description of  $L^{\mathcal{O}}(\cdot)$  given in (10), the weak adjoint problem (9) is equivalent to the following strong equation for the adjoint displacement  $\mathbf{u}^d$ ,

$$\rho(\ddot{\mathbf{u}}^d - a_1 \dot{\mathbf{u}}^d) - \nabla \cdot \boldsymbol{\sigma}^d(\mathbf{u}^d) = -\mathbf{f}^{\mathcal{O}} \quad \text{in } \Omega \times I, \quad (11a)$$

$$\mathbf{u}^d = \mathbf{0} \quad \text{on } \Gamma_D \times I, \quad (11b)$$

$$\boldsymbol{\sigma}^d(\mathbf{u}^d) \cdot \mathbf{n} = -\mathbf{g}^{\mathcal{O}} \quad \text{on } \Gamma_N \times I, \quad (11c)$$

$$\mathbf{u}^d = \mathbf{u}^{\mathcal{O}} \quad \text{at } \Omega \times \{T\}, \quad (11d)$$

$$\dot{\mathbf{u}}^d = \mathbf{v}^{\mathcal{O}} \quad \text{at } \Omega \times \{T\}, \quad (11e)$$

with the constitutive law

$$\boldsymbol{\sigma}^d(\mathbf{u}^d) := \mathcal{C} : \boldsymbol{\varepsilon}(\mathbf{u}^d - a_2 \dot{\mathbf{u}}^d). \quad (12)$$

The strong problem (11) has the same structure as the original one (1) except that the terms affected by  $a_1$  and  $a_2$  have opposite sign and the conditions (11d) and (11e) are stated for  $t = T$  instead that for  $t = 0$  (final conditions instead of initial). Thus, the adjoint problem is solvable and stable if integrated backwards in time. The change of sign in the time direction brings the adjoint problem back to the same features and properties as the direct one.

### 3.2 Error representation

The adjoint problem allows rewriting the error in the quantity of interest in terms of residuals, combining the original and adjoint problems. Indeed, taking  $\mathbf{W} = \mathbf{U}^d$  in

the error equation (6) and using the definition of the adjoint problem, the following representation for  $s^e$  is found

$$s^e := R(\mathbf{U}^d). \quad (13)$$

This error representation is useful because states that the error in the quantity of interest can be exactly computed if the adjoint solution  $\mathbf{U}^d$  is available. Moreover, in an error estimation setup where the exact adjoint solution is not known, replacing  $\mathbf{U}^d$  by a computable approximation  $\tilde{\mathbf{U}}^d$  in (13) gives an accurate approximation of the error in the quantity of interest

$$s^e \approx R(\tilde{\mathbf{U}}^d) =: \tilde{s}^e. \quad (14)$$

The scalar estimate  $\tilde{s}^e$  provides a single scalar quantity accounting both for the total error associated with the space and time discretizations and therefore, it does not directly provide enough information to adapt separately the space and time discretizations.

The error representation (13) is rewritten in such a way that the contributions of the space and time discretization errors are separated. This is achieved by introducing projection operators  $\Pi^H$  and  $\Pi^{\Delta t}$  associated with the space and time discretizations.

The spatial projection  $\Pi^H$  is defined for a function in  $\mathbf{W} \in \widehat{\mathcal{W}} \times \widehat{\mathcal{W}}$  and provides a function which is discrete in space. The spatial discretization (the mesh) varies along the time but it is constant in a time interval  $I_n$ . Thus, the operator  $\Pi^H$  is defined for  $t \in I_n$ ,  $n = 1, \dots, N$ , as

$$[\Pi^H \mathbf{W}](t) := [\pi_n^H \mathbf{w}_u(t), \pi_n^H \mathbf{w}_v(t)],$$

being  $\pi_n^H$  the standard interpolation operator from  $\mathcal{V}_0$  into  $\mathcal{V}_0^H(\mathcal{P}_n)$ . On the other hand, the projection in time operator  $\Pi^{\Delta t}$  maps the time-dependent function  $\mathbf{W} \in \widehat{\mathcal{W}} \times \widehat{\mathcal{W}}$  into a piecewise constant in time function. This projection is defined by taking the average of its displacement and velocity components inside each time interval  $I_n$

$$[\Pi^{\Delta t} \mathbf{W}]|_{I_n} := [\pi_n^{\Delta t} \mathbf{w}_u, \pi_n^{\Delta t} \mathbf{w}_v],$$

where

$$\pi_n^{\Delta t} \mathbf{w} := \frac{1}{\text{meas}(I_n)} \int_{I_n} \mathbf{w} dt.$$

**Remark 1.** Figure 3 illustrates the projection operators  $\Pi^H$  and  $\Pi^{\Delta t}$  using a generic function  $\mathbf{W} \in \widehat{\mathcal{W}} \times \widehat{\mathcal{W}}$ . Function  $\Pi^H \mathbf{W}$  belongs to the space  $\widehat{\mathcal{W}}^H \times \widehat{\mathcal{W}}^H$ , where

$$\widehat{\mathcal{W}}^H := \{\mathbf{w} \in \widehat{\mathcal{W}} : \mathbf{w}|_{I_n} \in L^2(I_n; \mathcal{V}_0^H(\mathcal{P}_n)), \text{ and} \\ \dot{\mathbf{w}}|_{I_n} \in L^2(I_n; (\mathcal{V}_0^H(\mathcal{P}_n))') \quad n = 1, \dots, N\}.$$

Note that  $\Pi^H \mathbf{W}$  is discrete in space: for each particular time  $t \in I$ , function  $[\Pi^H \mathbf{W}](t)$  belongs to one of the discrete finite elements spaces  $\mathcal{V}_0^H(\mathcal{P}_n) \times \mathcal{V}_0^H(\mathcal{P}_n)$ . However, the time description of  $\Pi^H \mathbf{W}$  is infinite dimensional: for a given  $\mathbf{x} \in \Omega$ ,  $\Pi^H \mathbf{W}(\mathbf{x}, \cdot) \in L^2(I) \times L^2(I)$ .

On the other hand, the function  $\Pi^{\Delta t} \mathbf{W}$  belong to  $\widehat{\mathcal{W}}^{\Delta t} \times \widehat{\mathcal{W}}^{\Delta t}$ , where

$$\widehat{\mathcal{W}}^{\Delta t} := \{\mathbf{w} \in \widehat{\mathcal{W}} : \mathbf{w}|_{I_n} \in \mathbb{P}^0(I_n; \mathcal{V}_0), \\ n = 1, \dots, N\}.$$

Note that  $\Pi^{\Delta t} \mathbf{W}$  is piecewise constant in time, but its spatial description is infinite dimensional, namely  $\Pi^{\Delta t} \mathbf{W}(\cdot, t) \in \mathcal{V}_0 \times \mathcal{V}_0$ .

Once the space and time projections are introduced, the space and time errors are separated adding the value  $R(\Pi^H \mathbf{U}^d) - R(\Pi^H \mathbf{U}^d) + R(\Pi^H \Pi^{\Delta t} \mathbf{U}^d)$  in the right hand side of (13) (the latter term vanishes due to the Galerkin orthogonality property because  $\Pi^H \Pi^{\Delta t} \mathbf{U}^d \in \widehat{\mathcal{W}}^{H, \Delta t} \times \widehat{\mathcal{W}}^{H, \Delta t}$ ). That is,

$$s^e = \underbrace{R(\mathbf{U}^d - \Pi^H \mathbf{U}^d)}_{=: s_s^e} + \underbrace{R(\Pi^H (\mathbf{U}^d - \Pi^{\Delta t} \mathbf{U}^d))}_{=: s_t^e}. \quad (15)$$

The terms  $s_s^e$  and  $s_t^e$  are associated with the space and time discretization errors respectively. Note that  $s_s^e$  tends to zero as the space discretization is refined because  $\Pi^H \mathbf{U}^d$  tends to  $\mathbf{U}^d$ . Similarly,  $s_t^e$  tends to zero with  $\Delta t$  because  $\Pi^{\Delta t} \mathbf{U}^d$  tends to  $\mathbf{U}^d$ . The space and time error components  $s_s^e$  and  $s_t^e$  are used as refinement indicators because they can be reduced independently by respectively enriching the space and time discretizations.

The space and time splitting is straightforwardly transformed to the estimated version of the error  $\tilde{s}^e$ , replacing  $\mathbf{U}^d$  by the computable approximation  $\tilde{\mathbf{U}}^d$  in equation (15), namely

$$\tilde{s}^e = \tilde{s}_s^e + \tilde{s}_t^e, \quad (16)$$

where  $\tilde{s}_s^e := R(\tilde{\mathbf{U}}^d - \Pi^H \tilde{\mathbf{U}}^d)$  and  $\tilde{s}_t^e := R(\Pi^H \tilde{\mathbf{U}}^d - \Pi^H \Pi^{\Delta t} \tilde{\mathbf{U}}^d)$  are the computable space and time error contributions.

### 3.3 Modal-based adjoint approximation

The error estimate  $\tilde{s}^e$  is computable once the adjoint approximation  $\tilde{\mathbf{U}}^d$  is available. Typically, the adjoint approximation is computed using the same code used for the original problem (1), i.e. using direct time-integration methods, see reference [20]. An alternative approach proposed

in [24] considers modal analysis to compute the adjoint approximation. The modal-based strategy is particularly well suited for some particular quantities of interest and allows effectively computing and storing the adjoint problem. In that case, the adjoint solution is stored for each vibration mode instead of for each time step.

Modal analysis requires introducing the semidiscrete equation (discrete in space but exact in time) associated with the adjoint problem (11). Consequently, a discrete version of the functional space  $\mathcal{V}_0$  is required. The semidiscrete problem is defined using the finite element space  $\mathcal{V}_0^{H, p+1}(\mathcal{P}_{\text{bg}})$ , that stands for the finite element space associated with the mesh  $\mathcal{P}_{\text{bg}}$  of degree of interpolation  $p+1$  (a  $p$ -refined version of  $\mathcal{V}_0^H(\mathcal{P}_{\text{bg}})$ ). Having a  $p+1$  degree approximation of the adjoint solution,  $\tilde{\mathbf{U}}^d$ , precludes the Galerkin orthogonality effect and the corresponding underestimation of the error, see [24]. Recall that, along the adaptive process, the background mesh is used as the base to build up all the adapted meshes by local refinement. Thus, the representation of  $\tilde{\mathbf{U}}^d$  in the adapted mesh is simplified if  $\tilde{\mathbf{U}}^d$  is in  $\mathcal{V}_0^{H, p+1}(\mathcal{P}_{\text{bg}})$ .

With these definitions, the semidiscrete problem reads: find  $\mathbf{u}^{d, H, p+1}(t) \in \mathcal{V}_0^{H, p+1}(\mathcal{P}_{\text{bg}})$  verifying the final conditions  $\mathbf{u}^{d, H, p+1}(T) = \mathbf{u}^{\mathcal{O}}$  and  $\dot{\mathbf{u}}^{d, H, p+1}(T) = \mathbf{v}^{\mathcal{O}}$  and such that for all  $t \in I$

$$m(\ddot{\mathbf{u}}^{d, H, p+1}(t) - a_1 \dot{\mathbf{u}}^{d, H, p+1}, \mathbf{w}) \\ + a(\mathbf{u}^{d, H, p+1}(t) - a_2 \dot{\mathbf{u}}^{d, H, p+1}(t), \mathbf{w}) = \\ - l^{\mathcal{O}}(t; \mathbf{w}) \quad \forall \mathbf{w} \in \mathcal{V}_0^{H, p+1}(\mathcal{P}_{\text{bg}}), \quad (17)$$

where  $l^{\mathcal{O}}(t; \mathbf{w}) := (\mathbf{f}^{\mathcal{O}}(t), \mathbf{w}) + (\mathbf{g}^{\mathcal{O}}(t), \mathbf{w})_{\Gamma_N}$ .

Equation (17) leads to an algebraic system of second order ordinary differential equations which is conveniently rewritten using the eigenvalues and eigenfunctions of the problem: find  $(\tilde{\omega}, \tilde{\mathbf{q}}) \in \mathbb{R} \times \mathcal{V}_0^{H, p+1}(\mathcal{P}_{\text{bg}})$  such that

$$a(\tilde{\mathbf{q}}, \mathbf{w}) = \tilde{\omega}^2 m(\tilde{\mathbf{q}}, \mathbf{w}) \quad \forall \mathbf{w} \in \mathcal{V}_0^{H, p+1}(\mathcal{P}_{\text{bg}}). \quad (18)$$

Note that the number of eigenpairs solution of this problem is the number of degrees of freedom in the finite element space  $\mathcal{V}_0^{H, p+1}(\mathcal{P}_{\text{bg}})$ , denoted by  $N_{\text{dof}}$ . The eigenpairs are sorted from low to high frequencies, namely  $\tilde{\omega}_1 \leq \tilde{\omega}_2 \leq \dots \leq \tilde{\omega}_{N_{\text{dof}}}$ , and the eigenfunctions are normalized to be orthonormal with respect the mass product, i.e.

$$m(\tilde{\mathbf{q}}_i, \tilde{\mathbf{q}}_j) = \delta_{ij}, \quad 1 \leq i, j \leq N_{\text{dof}}. \quad (19)$$

The semidiscrete approximation  $\mathbf{u}^{d, H, p+1}$  is expressed as a linear combination of the eigenfunctions  $\tilde{\mathbf{q}}_i$

$$\mathbf{u}^{d, H, p+1}(\mathbf{x}, t) = \sum_{i=1}^{N_{\text{dof}}} \tilde{\mathbf{q}}_i(\mathbf{x}) \tilde{y}_i(t). \quad (20)$$

Thus, the new unknowns of the problem are the time-dependent coefficients  $\tilde{y}_i(t)$ ,  $i = 1, \dots, N_{\text{dof}}$ . The representation in terms of the unknowns  $\tilde{y}_i(t)$  given in (20) allows uncoupling the system (17) into a set of ordinary

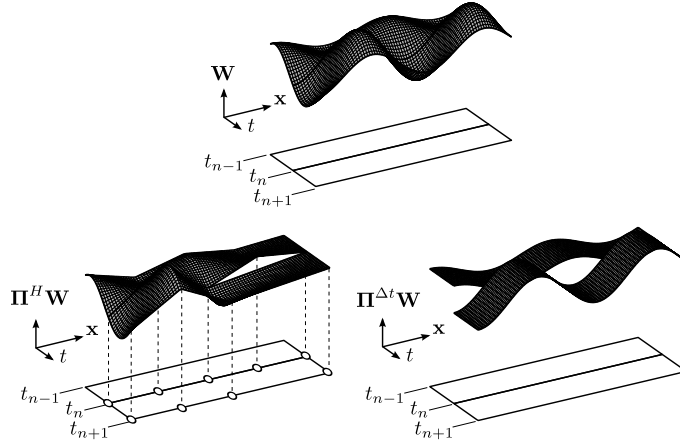


Figure 3: Illustration of the projection operators  $\Pi^H$  and  $\Pi^{\Delta t}$ . The figure displays (one field of) the original function  $\mathbf{W} \in \widehat{\mathcal{W}} \times \widehat{\mathcal{W}}$  inside the time intervals  $I_n = (t_{n-1}, t_n]$  and  $I_{n+1} = (t_n, t_{n+1}]$  (top) along with its projections in space and time  $\Pi^H \mathbf{W} \in \widehat{\mathcal{W}}^H \times \widehat{\mathcal{W}}^H$  (left) and  $\Pi^{\Delta t} \mathbf{W} \in \widehat{\mathcal{W}}^{\Delta t} \times \widehat{\mathcal{W}}^{\Delta t}$  (right).

differential equations, namely

$$\ddot{y}_i - [a_1 + a_2(\tilde{\omega}_i)^2]\dot{y}_i + (\tilde{\omega}_i)^2 y_i = \tilde{l}_i, \quad (21a)$$

$$y_i^H(T) = \tilde{u}_i, \quad (21b)$$

$$\dot{y}_i^H(T) = \tilde{v}_i, \quad (21c)$$

where the r.h.s. terms  $l_i$ ,  $u_i$  and  $v_i$  are computed using the data characterizing the quantity of interest (10) and the eigenfunction  $\tilde{\mathbf{q}}_i$

$$\begin{aligned} \tilde{l}_i(t) &:= (\mathbf{f}^\mathcal{O}(t), \tilde{\mathbf{q}}_i) + (\mathbf{g}^\mathcal{O}(t), \tilde{\mathbf{q}}_i)_{\Gamma_N}, \\ \tilde{u}_i &:= m(\mathbf{u}^\mathcal{O}, \tilde{\mathbf{q}}_i) \text{ and } \tilde{v}_i := m(\mathbf{v}^\mathcal{O}, \tilde{\mathbf{q}}_i). \end{aligned} \quad (22)$$

The time dependent coefficients  $\tilde{y}_i(t)$ ,  $i = 1, \dots, N_{\text{dof}}$ , may be computed analytically for many particular cases of the forcing data. The particular solution for constant-in-time data is given in [24]. Therefore the value of the adjoint solution  $\mathbf{u}^{\text{d},H,p+1}$  at any time  $t \in I$  is easily reconstructed from the computed eigenfunctions  $\tilde{\mathbf{q}}_i$  and the analytically computed time-dependent functions  $\tilde{y}_i(t)$  using expression (20).

In practice, it is not feasible to compute all the eigenpairs  $(\tilde{\omega}_i, \tilde{\mathbf{q}}_i)$ ,  $i = 1, \dots, N_{\text{dof}}$  and consequently the modal expansion (20) has to be truncated to the first  $M \ll N_{\text{dof}}$  terms, namely

$$\tilde{\mathbf{u}}^{\text{d}}(\mathbf{x}, t) := \sum_{i=1}^M \tilde{\mathbf{q}}_i(\mathbf{x}) \tilde{y}_i(t). \quad (23)$$

The number of required vibration modes  $M$  has to be selected such that the truncated high frequency modes (for  $i > M$ ) are negligible. That is,  $M$  is such that  $\tilde{\mathbf{u}}^{\text{d}}$  is a good approximation to  $\mathbf{u}^{\text{d},H,p+1}$ . This is equivalent to assume that for  $i > M$  the values of  $\tilde{l}_i$ ,  $\tilde{u}_i$  and  $\tilde{v}_i$ , as defined in (22), are close to zero, and consequently  $\tilde{y}_i(t) \approx 0$ . This is

guaranteed if the data  $\mathbf{f}^\mathcal{O}$ ,  $\mathbf{g}^\mathcal{O}$ ,  $\mathbf{u}^\mathcal{O}$  and  $\mathbf{v}^\mathcal{O}$  are well captured by the expansion of the first  $M$  eigenvectors. Consequently, a quantity of interest can be easily treated with the modal-based approach if its associated data  $\mathbf{f}^\mathcal{O}$ ,  $\mathbf{g}^\mathcal{O}$ ,  $\mathbf{u}^\mathcal{O}$  and  $\mathbf{v}^\mathcal{O}$  are well captured by the expansion of the first  $M$  eigenfunctions.

Once the computable adjoint approximation  $\tilde{\mathbf{u}}^{\text{d}}$  is available, the double field approximation  $\tilde{\mathbf{U}}^{\text{d}}$  used in the error estimate  $\tilde{s}^e$  given in (14) is readily defined as  $\tilde{\mathbf{U}}^{\text{d}} := [\tilde{\mathbf{u}}^{\text{d}}, \tilde{\mathbf{u}}^{\text{d}}]$ .

## 4 Space-time Adaptivity

### 4.1 Adaptivity framework

The space-time adaptive strategy aims at finding a time discretization  $\mathcal{T}$  and a space discretization  $\mathcal{P}_n$  at each time point  $t_n \in \mathcal{T}$  such that 1) they keep the error  $s^e$  below a user-prescribed tolerance  $s_{\text{tol}}^e$  and 2) they are optimal in the sense that they minimize the computational cost. In practice, the accuracy prescription is enforced for the estimated error and the property which is actually achieved is

$$|\tilde{s}^e| \leq s_{\text{tol}}^e. \quad (24)$$

Changing the space discretization at each time step  $t_n \in \mathcal{T}$  is not computationally affordable. This is because remeshing operations, matrix assembly and data transfer between different meshes are costly operations and cannot, in general, be performed at each time step. Here, an adaptive strategy organized in time-blocks, similar to the one proposed in reference [19], is adopted in order to reduce the number of mesh changes.

The blockwise adaptive strategy consist in splitting the time interval  $I$  into  $N^{\text{bk}}$  time intervals (or time blocks)

The time interval  $I$  is split into  $N^{\text{bk}}$  time intervals (or time blocks)

$$I_m^{\text{bk}} := \left( \frac{T}{N^{\text{bk}}}(m-1), \frac{T}{N^{\text{bk}}}m \right], \quad m = 1, \dots, N^{\text{bk}}.$$

The blockwise adaptive strategy consists taking the same space mesh inside each time interval  $I_m^{\text{bk}}$ , this mesh is denoted as  $\mathcal{P}_m^{\text{bk}}$  for  $m = 1, \dots, N^{\text{bk}}$ , see figure 4. Note that with this definition the computational meshes  $\mathcal{P}_n$  associated with the time points  $t_n \in I_m^{\text{bk}}$  are such that  $\mathcal{P}_n = \mathcal{P}_m^{\text{bk}}$ . A generic element of the mesh  $\mathcal{P}_m^{\text{bk}}$  is denoted by  $\Omega_k^m$ ,  $k = 1, \dots, N_m^{\text{el}}$ , where  $N_m^{\text{el}}$  is the number of elements in  $\mathcal{P}_m^{\text{bk}}$ .

Additionally, the time step length is assumed to be constant inside the intervals  $I_m^{\text{bk}}$  and denoted by  $\Delta t_m^{\text{bk}}$ . Consequently, the time step length  $\Delta t_n$  associated with times  $t_n \in I_m^{\text{bk}}$  are such that  $\Delta t_n = \Delta t_m^{\text{bk}}$ , see figure 4.

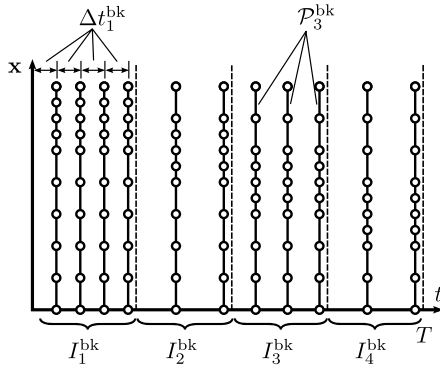


Figure 4: The space mesh is assumed to be constant inside the time intervals  $I_m^{\text{bk}}$ . Analogously, the time step length is taken constant inside each interval  $I_m^{\text{bk}}$ .

Following this approach and notation, the adaptive strategy is reformulated as computing the optimal space meshes  $\mathcal{P}_m^{\text{bk}}$  and time step lengths  $\Delta t_m^{\text{bk}}$ , for all the time intervals  $I_m^{\text{bk}}$ ,  $m = 1, \dots, N^{\text{bk}}$  such that the associated numerical solution fulfills (24).

Once the adjoint solution is computed and stored in the  $p+1$  version of the background mesh (keeping the same geometry and topology but increasing the degree of polynomials from  $p$  to  $p+1$ ), the main stages of the adaptive procedure are summarized as follows. The numerical solution is computed sequentially starting from the first time block  $I_1^{\text{bk}}$  until the last one  $I_{N^{\text{bk}}}^{\text{bk}}$ . In each time slab  $I_m^{\text{bk}}$ , the numerical solution is computed and the corresponding local error contributions are estimated. The computed solution in  $I_m^{\text{bk}}$  is *accepted* or *rejected* using the information given by the local error contributions. The specific acceptability criterion is detailed later. If the solution is accepted, the loop goes to the following time interval  $I_{m+1}^{\text{bk}}$ . Else, the space or time discretization (or both) associated with the interval  $I_m^{\text{bk}}$  are adapted using the local error information and the solution is re-computed in  $I_m^{\text{bk}}$ . The process of adapting the discretization and computing the numerical

solution is repeated in the interval  $I_m^{\text{bk}}$  until the solution is accepted.

The forthcoming subsections describe in detail 1) the local error contributions driving the adaptive process, 2) the criterion used to accept or reject the solution in each interval  $I_m^{\text{bk}}$  and 3) how to adapt the space and time discretizations when required.

## 4.2 Local error contributions

The space and time error estimates  $\tilde{s}_s^e$  and  $\tilde{s}_t^e$  are decomposed into contributions associated with the time blocks  $I_m^{\text{bk}}$ ,  $m = 1, \dots, N^{\text{bk}}$ , namely

$$\eta_m^s := R_{I_m^{\text{bk}}}(\tilde{\mathbf{U}}^d - \mathbf{\Pi}^H \tilde{\mathbf{U}}^d), \quad \text{and}$$

$$\eta_m^t := R_{I_m^{\text{bk}}}(\mathbf{\Pi}^H(\tilde{\mathbf{U}}^d - \mathbf{\Pi}^{\Delta t} \tilde{\mathbf{U}}^d))$$

such that

$$\tilde{s}_s^e = \sum_{m=1}^{N^{\text{bk}}} \eta_m^s \quad \text{and} \quad \tilde{s}_t^e = \sum_{m=1}^{N^{\text{bk}}} \eta_m^t.$$

The local residual  $R_{I_m^{\text{bk}}}(\cdot)$  is the restriction of the residual  $R(\cdot)$  to the time interval  $I_m^{\text{bk}}$ ,

$$\begin{aligned} R_{I_m^{\text{bk}}}(\mathbf{W}) &:= \int_{I_m^{\text{bk}}} [(\mathbf{f}, \mathbf{w}_v) + (\mathbf{g}, \mathbf{w}_v)_{\Gamma_N}] \, dt \\ &\quad - \int_{I_m^{\text{bk}}} m(\dot{\tilde{\mathbf{u}}}_v + a_1 \tilde{\mathbf{u}}_v, \mathbf{w}_v) \, dt \\ &\quad + \int_{I_m^{\text{bk}}} a(\tilde{\mathbf{u}}_u + a_2 \tilde{\mathbf{u}}_v, \mathbf{w}_v) \, dt \\ &\quad - \int_{I_m^{\text{bk}}} a(\dot{\tilde{\mathbf{u}}}_u - \tilde{\mathbf{u}}_v, \mathbf{w}_u) \, dt. \end{aligned}$$

The indicator  $\eta_m^t$  is used to decide if the time discretization inside  $I_m^{\text{bk}}$  has to be modified. The criteria on whether the time grid has to be modified and how it has to be modified are presented in section 4.3.

The value of  $\eta_m^s$  is the indicator used to decide if the space mesh  $\mathcal{P}_m^{\text{bk}}$  in the time interval  $I_m^{\text{bk}}$  has to be further adapted. Again, the detailed criteria are introduced in section 4.3. In the case the mesh is to be adapted, the required local error indicators are obtained by restricting the space integrals involved in  $\eta_m^s$  to the elements  $\Omega_k^m$ . That is,

$$\eta_{m,k}^s := R_{\Omega_k^m \times I_m^{\text{bk}}}(\tilde{\mathbf{U}}^d - \mathbf{\Pi}^H \tilde{\mathbf{U}}^d),$$

where

$$\begin{aligned} R_{\Omega_k^m \times I_m^{\text{bk}}}(\mathbf{W}) &:= \int_{I_m^{\text{bk}}} [(\mathbf{f}, \mathbf{w}_v)_{\Omega_k^m} + (\mathbf{g}, \mathbf{w}_v)_{\partial\Omega_k^m \cap \Gamma_N}] \, dt \\ &\quad - \int_{I_m^{\text{bk}}} m(\dot{\tilde{\mathbf{u}}}_v + a_1 \tilde{\mathbf{u}}_v, \mathbf{w}_v)_{\Omega_k^m} \, dt \\ &\quad + \int_{I_m^{\text{bk}}} a(\tilde{\mathbf{u}}_u + a_2 \tilde{\mathbf{u}}_v, \mathbf{w}_v)_{\Omega_k^m} \, dt \\ &\quad - \int_{I_m^{\text{bk}}} a(\dot{\tilde{\mathbf{u}}}_u - \tilde{\mathbf{u}}_v, \mathbf{w}_u)_{\Omega_k^m} \, dt. \end{aligned}$$

Note that the error estimate  $\bar{s}^e$  is expressed as the sum of the local error contributions defined above

$$\bar{s}^e = \sum_{m=1}^{N^{bk}} \left( \sum_{k=1}^{N_m^{el}} \eta_{m,k}^s \right) + \sum_{m=1}^{N^{bk}} \eta_m^t.$$

### 4.3 Acceptability and remeshing criteria

Following references [20, 19], the total target error  $s_{tol}^e$  is split into two error targets,  $\alpha_s s_{tol}^e$  and  $\alpha_t s_{tol}^e$ , associated with the space and time errors. The coefficients  $\alpha_s$  and  $\alpha_t$  are two user-defined positive values such that  $\alpha_s + \alpha_t = 1$  used to balance the space and time contributions to the total error. This leaves a free parameter to be tuned by the user, who must decide the amount of the total error  $s_{tol}^e$  assigned to the space and time discretizations. Discussing the optimal values for  $\alpha_s$  and  $\alpha_t$  is beyond the scope of this paper.

Thus, in order to achieve the accuracy prescription stated in (24), the adaptive strategy is designed aiming at finding a numerical solution such that

$$|\bar{s}_s^e| \leq \alpha_s s_{tol}^e \quad \text{and} \quad |\bar{s}_t^e| \leq \alpha_t s_{tol}^e. \quad (25)$$

Note that (25) guarantees that equation (24) holds, because

$$|\bar{s}^e| = |\bar{s}_s^e + \bar{s}_t^e| \leq |\bar{s}_s^e| + |\bar{s}_t^e| \leq \alpha_s s_{tol}^e + \alpha_t s_{tol}^e = s_{tol}^e.$$

The conditions (25) are more restrictive than (24). This is because in (24)  $\bar{s}_s^e$  and  $\bar{s}_t^e$  with different sign may cancel each other. The error compensation is not accounted for in (25) and therefore the resulting criterion is more demanding.

The error contributions are assumed to be uniformly distributed in time. That is, the space and time error tolerances,  $\alpha_s s_{tol}^e$  and  $\alpha_t s_{tol}^e$ , are divided into equal contributions associated with each time block  $I_m^{bk}$ . Thus, the solution is considered to be acceptable if

$$|\eta_m^s| \leq \frac{\alpha_s s_{tol}^e}{N^{bk}}, \quad (26a)$$

$$|\eta_m^t| \leq \frac{\alpha_t s_{tol}^e}{N^{bk}}. \quad (26b)$$

If the restrictions (26) hold, then the inequalities (25) are fulfilled, because

$$\begin{aligned} |\bar{s}_s^e| &= \left| \sum_{m=1}^{N^{bk}} \eta_m^s \right| \leq \sum_{m=1}^{N^{bk}} |\eta_m^s| \leq \alpha_s s_{tol}^e \quad \text{and} \\ |\bar{s}_t^e| &= \left| \sum_{m=1}^{N^{bk}} \eta_m^t \right| \leq \sum_{m=1}^{N^{bk}} |\eta_m^t| \leq \alpha_t s_{tol}^e. \end{aligned} \quad (27)$$

Similarly as when splitting the space and time contributions, criteria (26) are stronger than (25). This is more relevant for large values of  $N^{bk}$ , because the effect of the *triangular* inequalities in the equations (27) is more important. Thus, the adapted numerical solution might be very conservative if the number of blocks  $N^{bk}$  is large.

An additional condition is added to (26) in order to allow unrefinement (mesh coarsening). Note that the conditions (26) indicate only if the solution is acceptable and, if not, if the mesh has to be refined. They do not provide a criterion to unrefine the discretization when the error indicators  $\eta_m^s$  and  $\eta_m^t$  are small enough. Following reference [20], a lower bound based acceptability criterion is added to (26):

$$\beta_s \frac{\alpha_s s_{tol}^e}{N^{bk}} \leq |\eta_m^s|, \quad (28a)$$

$$\beta_t \frac{\alpha_t s_{tol}^e}{N^{bk}} \leq |\eta_m^t|, \quad (28b)$$

where the coefficients  $\beta_s$  and  $\beta_t$  are two user-defined values such that  $\beta_s, \beta_t \in [0, 1)$ . If the solution does not fulfill condition (28b), then the time discretization is modified (in this case unrefined). If (28a) is violated, then the space mesh is modified and it is expected to be globally coarsened. However, the space mesh adaption is performed locally and may result in refining some parts of the domain while others are unrefined. The space remeshing criterion is described below. The coarsening criterion (28) is only checked once for each time block. This is because the need of unrefining the space or the time grid is expected to be detected with the first discretization. Moreover, checking for unrefining at each adaptive step may result in an unstable scheme.

As previously said, conditions (26) and (28) are the criteria allowing to decide if the numerical solution is accepted or rejected inside the interval  $I_m^{bk}$ . If conditions (26) and (28) hold (or only (26) after the first adaptive iteration), then the solution is accepted. Otherwise, the space and/or time discretizations are modified.

The time adaptivity is carried out, depending on the value of  $\eta_m^t$ , by either refining the discretization by halving the time step  $\Delta t_m^{bk}$  (if (26b) is violated) or doubling it (if (28b) is violated). If both (26b) and (28b) hold, the time discretization is unchanged.

If either (26a) or (28a) are not fulfilled, the space mesh is to be modified. Then, local criterion is required to decide which elements have to be refined or unrefined, depending on the value of the local indicators  $\eta_{m,k}^s$ ,  $k = 1, \dots, N_m^{el}$  (for a given  $m = 1, \dots, N^{bk}$ ). Similarly as for the time discretization, the elements to be refined are subdivided (the element size divided by two) while the elements to be coarsened are collapsed with the neighboring elements, doubling the element size. In order to set up a space remeshing criterion, the optimal mesh is assumed to yield a uniform error distribution. Thus, the local versions (restricted to the contributions associated with element  $\Omega_k^m$ ) of the conditions (26a) and (28a) read

$$\gamma_m \beta_s \frac{\alpha_s s_{tol}^e}{N^{bk} N_m^{el}} \leq |\eta_{m,k}^s| \leq \gamma_m \frac{\alpha_s s_{tol}^e}{N^{bk} N_m^{el}}, \quad (29)$$

where

$$\gamma_m := \frac{\sum_{k=1}^{N_m^{el}} |\eta_{m,k}^s|}{\sum_{k=1}^{N_m^{el}} \eta_{m,k}^s} \geq 1.$$

The coefficient  $\gamma_m$  is introduced in order to mitigate the cancellation effect, see reference [32]. It is worth noting

that introducing the factor  $\gamma_m$  does not introduce a distortion in the criterion: if all the local element error contributions fulfill (29), then equation (28a) holds. This is shown by noting that

$$|\eta_m^s| = \left| \sum_{k=1}^{N_m^{\text{el}}} \eta_{m,k}^s \right| = \frac{1}{\gamma_m} \left( \sum_{k=1}^{N_m^{\text{el}}} |\eta_{m,k}^s| \right)$$

and therefore

$$\beta_s \frac{\alpha_s s_{\text{tol}}^e}{N^{\text{bk}}} = \frac{1}{\gamma_m} \left( \sum_{k=1}^{N_m^{\text{el}}} \gamma_m \beta_s \frac{\alpha_s s_{\text{tol}}^e}{N^{\text{bk}} N_m^{\text{el}}} \right) \leq \frac{1}{\gamma_m} \left( \sum_{k=1}^{N_m^{\text{el}}} |\eta_{m,k}^s| \right) = |\eta_m^s|$$

and

$$\frac{\alpha_s s_{\text{tol}}^e}{N^{\text{bk}}} = \frac{1}{\gamma_m} \left( \sum_{k=1}^{N_m^{\text{el}}} \gamma_m \frac{\alpha_s s_{\text{tol}}^e}{N^{\text{bk}} N_m^{\text{el}}} \right) \geq \frac{1}{\gamma_m} \left( \sum_{k=1}^{N_m^{\text{el}}} |\eta_{m,k}^s| \right) = |\eta_m^s|.$$

The complete space-time adaptive strategy is summarized in algorithm 1.

## 5 Numerical Examples

### 5.1 Example 1: perforated plate under impulse loading

This example illustrates the performance of the proposed space-time adaptive strategy in a 2D wave propagation problem. The computational domain  $\Omega$  is a perforated rectangular plate,  $\Omega := (-0.5, 0.5) \times (0, 0.5) \setminus \Omega_0$  m<sup>2</sup>, with  $\Omega_0 := \{(x, y) \in \mathbb{R}^2 : x^2 + (y - 0.25)^2 \leq 0.025^2\}$  m<sup>2</sup>, see figure 5. The plate is clamped at the bottom side and the horizontal displacement is blocked at both vertical sides. The plate is initially at rest,  $\mathbf{u}_0 = \mathbf{v}_0 = \mathbf{0}$ , and loaded with the time dependent traction

$$\mathbf{g}(t) = \begin{cases} -g(t)\mathbf{e}_2 & \text{on } \Gamma_g, \\ 0 & \text{elsewhere,} \end{cases} \quad (30)$$

where  $\Gamma_g := (-0.025, 0.025) \times \{0.5\}$  m,  $\mathbf{e}_2 := (0, 1)$  and  $g(t)$  is the impulsive time-dependent function defined in figure 5 with parameters  $g_{\text{max}} = 30$  Pa and  $t_g = 0.005$  s. No body force is acting in this example,  $\mathbf{f} = \mathbf{0}$ . The material properties of the plate are Young's modulus  $E = 8/3$  Pa, Poisson's ratio  $\rho = 1/3$ , the density  $\rho = 1$  kg/m<sup>3</sup> and the damping coefficients  $a_1 = 0$  s<sup>-1</sup>,  $a_2 = 10^{-4}$  s. The final simulation time is  $T = 0.25$  s.

The background mesh  $\mathcal{P}_{\text{bg}}$  for the quadtree remeshing strategy is plotted in figure 6. Note that only half of the domain is discretized due to the problem's symmetry by introducing proper symmetry boundary conditions. The finite element spaces  $\mathbf{V}_0^{H,1}(\mathcal{P}_n)$ ,  $n = 1, \dots, N$ , used for

#### Data:

*Problem statement:* Problem geometry ( $\Omega$ ,  $\Gamma_N$ ,  $\Gamma_D$ ), final time ( $T$ ), material data ( $E$ ,  $\nu$ ,  $\rho$ ), loads and initial conditions ( $\mathbf{f}$ ,  $\mathbf{g}$ ,  $\mathbf{u}_0$ ,  $\mathbf{v}_0$ ).

*Problem discretization:* background computational mesh ( $\mathcal{P}_{\text{bg}}$ ).

*Error control:* data defining the quantity of interest ( $\mathbf{f}^{\mathcal{O}}$ ,  $\mathbf{g}^{\mathcal{O}}$ ,  $\mathbf{u}^{\mathcal{O}}$ ,  $\mathbf{v}^{\mathcal{O}}$ ) and number of vibration modes  $M$ .

*Adaptivity parameters:* Number of time blocks ( $N^{\text{bk}}$ ), prescribed error ( $s_{\text{tol}}^e$ ), error splitting coefficients ( $\alpha_s$ ,  $\alpha_t$ ), unrefinement parameters ( $\beta_s$ ,  $\beta_t$ ).

**Result:** Numerical approximation  $\tilde{\mathbf{U}}$  and error estimate  $\tilde{s}^e$  fulfilling  $|\tilde{s}^e| \leq s_{\text{tol}}^e$ .

// Modal analysis

Generate higher order space  $\mathbf{V}_0^{H,p+1}(\mathcal{P}_{\text{bg}})$ ;

Compute the eigenpairs  $(\tilde{\omega}_i, \tilde{\mathbf{q}}_i)$ ,  $i = 1, \dots, M$  in the space  $\mathbf{V}_0^{H,p+1}(\mathcal{P}_{\text{bg}})$ ;

// Adjoint problem (modal solution)

Compute the values  $\tilde{l}_i, \tilde{u}_i, \tilde{v}_i$  (using  $\mathbf{f}^{\mathcal{O}}$ ,  $\mathbf{g}^{\mathcal{O}}$ ,  $\mathbf{u}^{\mathcal{O}}$ ,  $\mathbf{v}^{\mathcal{O}}$  and  $\tilde{\mathbf{q}}_i$ ,  $i = 1, \dots, M$ );

Compute the time dependent functions  $\tilde{y}_i(t)$  (using  $\tilde{l}_i, \tilde{u}_i, \tilde{v}_i$  and  $\tilde{\omega}_i$ ,  $i = 1, \dots, M$ );

// Problem computation, error assessment and adaptivity

Initialize discretization:  $\mathcal{P}_1^{\text{bk}} = \mathcal{P}_{\text{bg}}$ ,  $\Delta t_1^{\text{bk}} = T/N^{\text{bk}}$ ;

for  $m = 1 \dots N^{\text{bk}}$  do

repeat

    // Compute solution and error estimate

    Compute solution  $\tilde{\mathbf{U}}$  in the time interval  $I_m^{\text{bk}}$  and the error indicators  $\eta_m^s$ ,  $\eta_{m,k}^s$  and  $\eta_m^t$ ;

    // Mesh adaptivity

    if The acceptability criteria for  $\eta_m^s$  or  $\eta_m^t$  are not fulfilled then

        Refine/unrefine the spatial mesh  $\mathcal{P}_m^{\text{bk}}$  (using  $\eta_{m,k}^s$ ) and/or the time step  $\Delta t_m^{\text{bk}}$  (using  $\eta_m^t$ );

    end

until The acceptability criteria for  $\eta_m^s$  and  $\eta_m^t$  are fulfilled;

Set initial discretization for the next time interval:  $\mathcal{P}_{m+1}^{\text{bk}} = \mathcal{P}_m^{\text{bk}}$ ,  $\Delta t_{m+1}^{\text{bk}} = \Delta t_m^{\text{bk}}$ ;

end

**Algorithm 1:** Algorithm for problem approximation with error control and space-time mesh adaptivity.

solving the direct problem are build using bilinear elements (quadrilaterals with 4 nodes, i.e.  $p = 1$ ) while the finite element space for the adjoint,  $\mathbf{V}_0^{H,2}(\mathcal{P}_n)$ , is build using serendipity elements (quadrilaterals with 8 nodes, i.e.  $p = 2$ ).

The quantity of interest considered in this example is a weighted average of the vertical velocities in the region

$$\Omega^{\mathcal{O}} := \{(x, y \in \mathbb{R}^2 : x^2 + (y - 0.1)^2 < 0.075^2)\} \text{ m}^2,$$

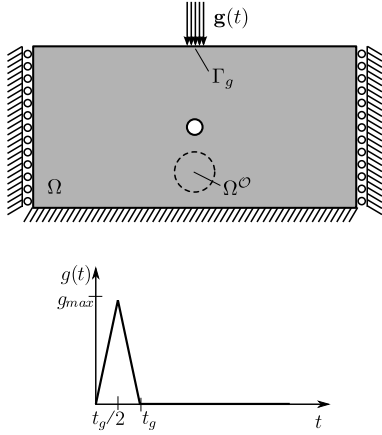


Figure 5: Example 1: Definition of the problem geometry (top) and time-dependence of the external load (bottom).

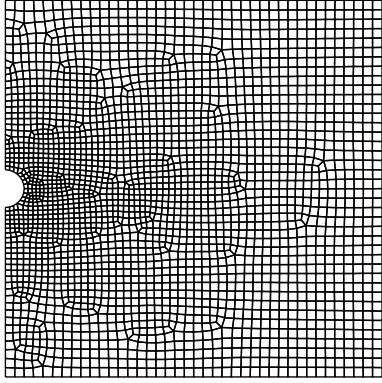


Figure 6: Example 1: Background mesh  $\mathcal{P}_{bg}$  with 2452 elements for the quadtree remeshing strategy and for the adjoint problem approximation. Only half of the domain is discretized due to the problem's symmetry.

see figure 5. Specifically, the quantity of interest is defined as

$$L^{\mathcal{O}}(\mathbf{W}) := m(\mathbf{v}^{\mathcal{O}}, \mathbf{w}_v(T)),$$

corresponding to  $\mathbf{f}^{\mathcal{O}} = \mathbf{g}^{\mathcal{O}} = \mathbf{u}^{\mathcal{O}} = \mathbf{0}$  in (8). The weighting function  $\mathbf{v}^{\mathcal{O}}$  with local support in  $\Omega^{\mathcal{O}}$  is  $\mathbf{v}^{\mathcal{O}} = [0, v_{aux}(\sqrt{x^2 + (y - 0.1)^2})]$  for

$$v_{aux}(r) = \begin{cases} \frac{10}{3\pi R^2 \rho} \left( 2 \left( \frac{r}{R} - 1 \right)^3 + 3 \left( \frac{r}{R} - 1 \right)^2 \right) & \text{for } 0 \leq r \leq R, \\ 0 & \text{for } R < r, \end{cases}$$

$R = 0.075$  being the radius of the region of interest. Note that since the  $x$ -component of  $\mathbf{v}^{\mathcal{O}}$  is zero, the quantity of interest gives an average of the vertical velocity in the region of interest  $\Omega^{\mathcal{O}}$  and at time  $t = T$ .

The adjoint problem associated to the quantity of interest is approximated using a truncated modal based approxi-

mation where only the first 60 vibration modes are kept. This corresponds to slightly modify the quantity of interest of the problem. In the following, the function  $\mathbf{v}^{\mathcal{O}}$  in the exact quantity of interest  $L^{\mathcal{O}}(\mathbf{W}) = m(\mathbf{v}^{\mathcal{O}}, \mathbf{w}_v(T))$  is replaced by its projection onto the first  $M = 60$  vibration modes  $\tilde{\mathbf{q}}_i$ ,  $i = 1, \dots, M$ , namely

$$\mathbf{v}^{\mathcal{O},M}(\mathbf{x}) := \sum_{i=1}^M v_i \tilde{\mathbf{q}}_i(\mathbf{x}), \quad \text{where } v_i := m(\mathbf{v}^{\mathcal{O}}, \tilde{\mathbf{q}}_i).$$

Figure 7 shows that the truncated discrete approximation  $\mathbf{v}^{\mathcal{O},M}$  provides a fairly good approximation of the exact weighting function  $\mathbf{v}^{\mathcal{O}}$ . It is worth noting that the quantity of interest is no longer strictly measuring only the vertical velocity of the solution and has no longer a local support. However, as can be seen, the influence of the horizontal velocity and the average outside  $\Omega^{\mathcal{O}}$  are small.

The exact solution  $\mathbf{U}$  (and therefore the exact quantity of interest  $s$ ) are unknown in this example. The exact solution is replaced here by an overkill approximation of the problem, namely  $\mathbf{U}^{ovk}$ , computed with a finite elements mesh with  $N^{el} = 627712$  elements and  $N = 1600$  time steps. The overkill discretization is finest discretization considered in this example. The exact value of the quantity of interest is approximated using the overkill approximation,  $s \approx s^{ovk} := L^{\mathcal{O}}(\mathbf{U}^{ovk}) = 2.4227 \cdot 10^{-2}$  m/s.

The behavior of the adaptive strategy is first analyzed for a prescribed target error  $s_{tol}^e = 5 \cdot 10^{-5}$  m/s. The user-prescribed parameters for the simulation are set to  $N^{bk} = 20$  for the number of space-time blocks,  $\alpha_s = 0.9$  and  $\alpha_t = 0.1$  for the coefficients used to split the total error budget into space and time and  $\beta_s = 0.5$  and  $\beta_t = 0.1$  for the lower bound factors.

Figure 8 shows several snapshots of an adapted numerical solution obtained with the proposed methodology. The quantity of interest associated with the numerical solution is  $\tilde{s} := L^{\mathcal{O}}(\tilde{\mathbf{U}}) = 2.4242 \cdot 10^{-2}$  m/s with an assessed error of  $\tilde{s}^e = -1.5756 \cdot 10^{-5}$  m/s. Thus the prescribed target error  $s_{tol}^e = 5 \cdot 10^{-5}$  m/s is fulfilled quite sharply, that is,  $|\tilde{s}^e| \leq s_{tol}^e$ , and  $|\tilde{s}^e|$  and  $s_{tol}^e$  are of the same order of magnitude. Moreover, the error with respect to the overkill solution, namely  $s_{ovk}^e := s^{ovk} - \tilde{s} = -1.5125 \cdot 10^{-5}$  m/s, is also below (in absolute value) the user-defined value  $s_{tol}^e$ . Note that the assessed error is a good approximation of the overkill error. That is, the effectivity of the error estimate,  $\tilde{s}^e/s_{ovk}^e = 1.041$ , is fairly close to the unity.

Figure 9 shows the history of the number of elements and the time step length along the adapted computation. Note that the number of elements increases with time as the elastic waves spread along the plate and therefore a larger area has to be refined. The time step is refined only when the external load is acting at the beginning of the computation. Additionally, figure 9 also shows the number of iterations performed in each space-time block until reaching convergence. As can be seen, convergence is reached for the whole computation with at most four iterations per block.

The performance of the space-time adaptive strategy is also tested versus a uniform refinement. Three non-



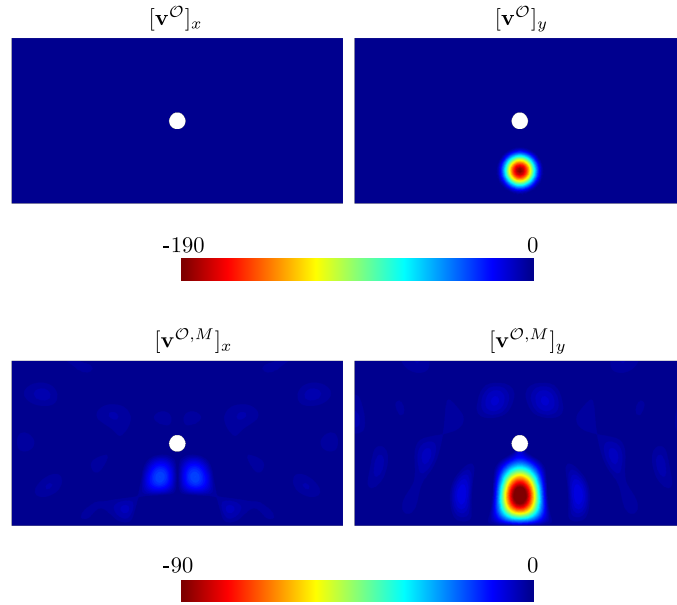


Figure 7: Example 1: Exact (top) and truncated (bottom) weighting functions  $\mathbf{v}^O$  defining the quantity of interest  $L^O(\cdot)$ .

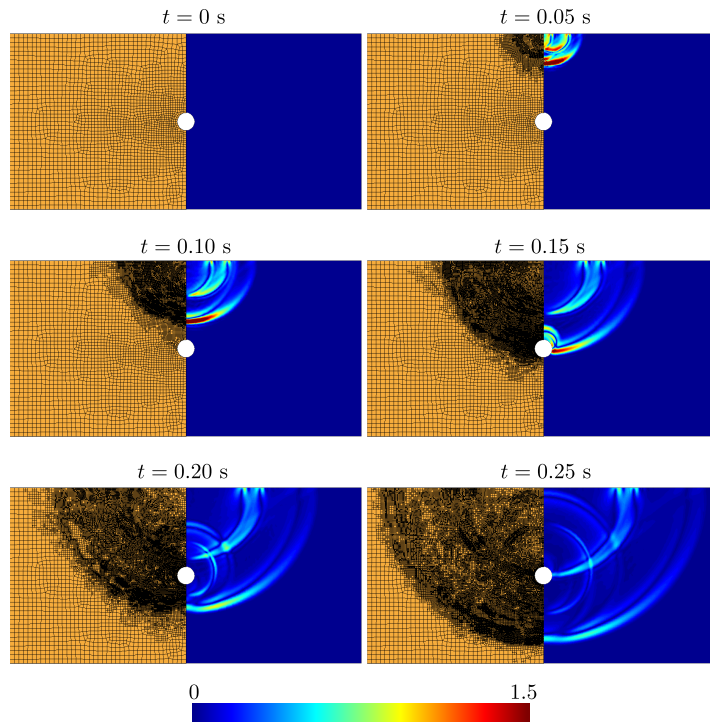


Figure 8: Example 1: Snapshots of the computed solution (magnitude of velocities in m/s) and the computational mesh at several time points for the adapted solution verifying the prescribed target error  $s_{\text{tol}}^e = 5 \cdot 10^{-5}$  m/s.

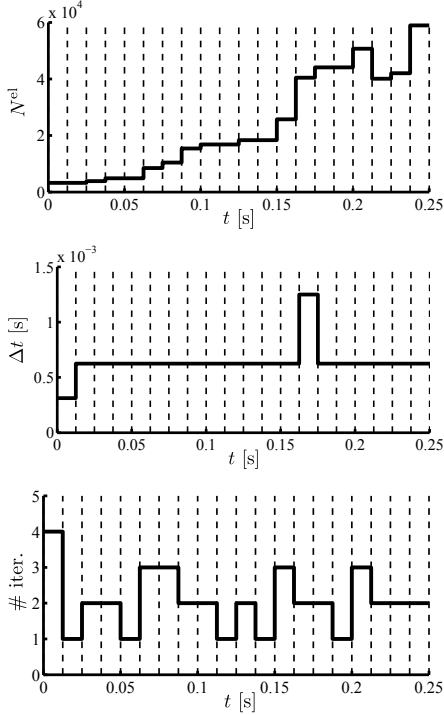


Figure 9: Example 1: History of the number of elements (top) and of the time step (middle). Number of iterations to achieve convergence in each block (bottom).

adapted (uniform) approximations are computed using three different space meshes and three different number of time steps  $N$ , see table 1. The initial space mesh corresponds to the background mesh showed in figure 6 which is recursively refined to obtain the other spatial meshes (each quadrilateral element is recursively subdivided into four new ones). The ratio  $H/\Delta t$ , or equivalently the ratio  $N/(N^{\text{el}})^{\frac{1}{2}}$ , is kept constant in the three uniform approximations. This is to ensure that the space and time errors are reduced at the same ratio taking into account that the space discretization error scales as  $H^2$  and the time discretization error as  $\Delta t^2$ , see [32] and [20].

Table 1: Example 1: Space and time discretizations for the three uniform solutions.

	$N^{\text{el}}$	# nodes	$N$
1	2452	2547	100
2	9808	9997	200
3	39232	39609	400

On the other hand, the space-time adaptive computations are performed prescribing similar total target errors as the errors obtained using uniform refinements. Specifically, four different simulations are performed setting  $s_{\text{tol}}^e = 1 \cdot 10^{-3}$ ,  $5 \cdot 10^{-4}$ ,  $1 \cdot 10^{-4}$  and  $5 \cdot 10^{-5}$  m/s combined with three different values for the number of blocks,  $N^{\text{bk}} =$

5, 10 and 20. The additional parameters of the adaptive procedure are  $\alpha_s = 0.9$ ,  $\beta_s = 0.5$  and  $\alpha_t = \beta_t = 0.1$ . The computational complexity of the simulations is measured here using the number of space-time elements (or cells), namely

$$N^{\text{cells}} := \sum_{m=1}^{N^{\text{bk}}} N_m^{\text{el}} \frac{T}{N^{\text{bk}} \Delta t_m^{\text{bk}}},$$

corresponding to sum up the number of space-time elements used inside each time interval  $I_m^{\text{bk}}$ ,  $m = 1, \dots, N^{\text{bk}}$ . Note that if a single space mesh is considered in the whole simulation time, then the number of space-time cells  $N^{\text{cells}}$  coincides with  $N^{\text{cells}} = N^{\text{el}} N$ .

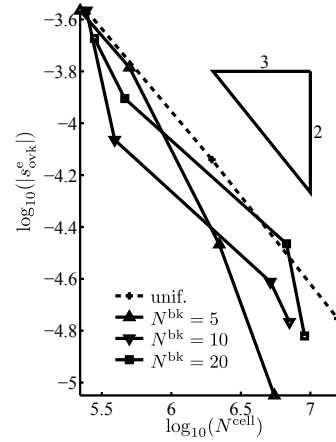


Figure 10: Example 1: Error convergence for the adapted and uniform computations. The adapted solutions are obtained using three different values of the number of time blocks  $N^{\text{bk}}$ .

Figure 10 shows the convergence of the estimates. The estimates obtained for the uniform refinement meet the expected a-priori convergence rate of  $-2/3$ . This expected convergence rate is obtained considering the a-priori estimates of the error  $s^e \propto H^2 + (\Delta t)^2$ , the relation  $N^{\text{cells}} \propto (H^2 \Delta t)^{-1}$  and noting that if the ratio  $H/\Delta t$  is constant, then  $H$  and  $\Delta t$  can be written as  $H = \kappa H^*$  and  $\Delta t = \kappa \Delta t^*$ , where  $H^*$  and  $\Delta t^*$  are the element and step length of the coarsest uniform discretization and  $\kappa$  is a refinement factor. It is then straightforward that,  $s^e \propto (N^{\text{cells}})^{-2/3}$  since  $(H^2 \Delta t)^{2/3} = C(H^2 + (\Delta t)^2) \propto s^e$  for  $C = ((H^*)^2 \Delta t^*)^{2/3} / ((H^*)^2 + (\Delta t^*)^2)$ . From figure 10 and table 2 it can be seen that besides converging at the correct convergence rate, the estimates are really accurate since their effectivities are very close to 1.

As expected, the use of an adaptive refinement strategy leads to better approximations for the quantity of interest with less computational cost. The adapted solutions have a lower error than the uniform approximations for the same number of space-time cells.

Table 2: Example 1: Performance of the estimate for both the uniform and adaptive strategies. The overkill value of the quantity of interest is  $s^{\text{ovk}} = 2.4227 \cdot 10^{-2}$  m/s obtained with  $N^{\text{cell}} = 1004339200$  space-time elements.

	$s_{\text{tol}}^e$ [m/s]	$N^{\text{cell}}$	$\bar{s}$ [m/s]	$\tilde{s}^e$ [m/s]	$s_{\text{ovk}}^e$ [m/s]	$\tilde{s}^e/s_{\text{ovk}}^e$
uniform	–	245200	$2.4498 \cdot 10^{-2}$	$-2.7186 \cdot 10^{-4}$	$-2.7180 \cdot 10^{-4}$	1.000
	–	1961600	$2.4299 \cdot 10^{-2}$	$-7.1606 \cdot 10^{-5}$	$-7.2345 \cdot 10^{-5}$	0.989
	–	15692800	$2.4244 \cdot 10^{-2}$	$-1.7813 \cdot 10^{-5}$	$-1.7659 \cdot 10^{-5}$	1.008
$N^{\text{bk}} = 5$	$1 \cdot 10^{-3}$	220680	$2.4498 \cdot 10^{-2}$	$-2.7186 \cdot 10^{-4}$	$-2.7180 \cdot 10^{-4}$	1.000
	$5 \cdot 10^{-4}$	499920	$2.4391 \cdot 10^{-2}$	$-1.6337 \cdot 10^{-4}$	$-1.6403 \cdot 10^{-4}$	0.996
	$1 \cdot 10^{-4}$	2211720	$2.4261 \cdot 10^{-2}$	$-3.3703 \cdot 10^{-5}$	$-3.4096 \cdot 10^{-5}$	0.988
	$5 \cdot 10^{-5}$	5511720	$2.4236 \cdot 10^{-2}$	$-1.0823 \cdot 10^{-5}$	$-8.9160 \cdot 10^{-6}$	1.213
$N^{\text{bk}} = 10$	$1 \cdot 10^{-3}$	245200	$2.4498 \cdot 10^{-2}$	$-2.7186 \cdot 10^{-4}$	$-2.7180 \cdot 10^{-4}$	1.000
	$5 \cdot 10^{-4}$	391280	$2.4313 \cdot 10^{-2}$	$-8.6724 \cdot 10^{-5}$	$-8.6226 \cdot 10^{-5}$	1.005
	$1 \cdot 10^{-4}$	5158120	$2.4251 \cdot 10^{-2}$	$-2.4351 \cdot 10^{-5}$	$-2.4455 \cdot 10^{-5}$	0.995
	$5 \cdot 10^{-5}$	7074440	$2.4244 \cdot 10^{-2}$	$-1.5773 \cdot 10^{-5}$	$-1.7111 \cdot 10^{-5}$	0.921
$N^{\text{bk}} = 20$	$1 \cdot 10^{-3}$	279900	$2.4439 \cdot 10^{-2}$	$-2.1096 \cdot 10^{-4}$	$-2.1219 \cdot 10^{-4}$	0.994
	$5 \cdot 10^{-4}$	462735	$2.4351 \cdot 10^{-2}$	$-1.2062 \cdot 10^{-4}$	$-1.2446 \cdot 10^{-4}$	0.969
	$1 \cdot 10^{-4}$	6732720	$2.4261 \cdot 10^{-2}$	$-3.6194 \cdot 10^{-5}$	$-3.4268 \cdot 10^{-5}$	1.056
	$5 \cdot 10^{-5}$	9080750	$2.4242 \cdot 10^{-2}$	$-1.5756 \cdot 10^{-5}$	$-1.5125 \cdot 10^{-5}$	1.041

## 5.2 Example 2: 2D structure

Consider the structure given in figure 11. The structure is initially at rest ( $\mathbf{u}_0 = \mathbf{v}_0 = \mathbf{0}$ ), clamped at the supports and subjected to the time-dependent traction

$$\mathbf{g} = \begin{cases} g(t)\mathbf{e}_1 & \text{on } \Gamma_g, \\ 0 & \text{elsewhere.} \end{cases}$$

The set  $\Gamma_g$  is the region of the Neumann boundary where the load is applied, vector  $\mathbf{e}_1 := (1, 0)$  is the first cartesian unit vector and function  $g(t)$  describes the time evolution of  $\mathbf{g}$  given in figure 11. The traction  $\mathbf{g}$  is the only external loading in this example (that is  $\mathbf{f} = \mathbf{0}$ ). Other material and geometric parameters univocally defining the problem are reported in table 3.

Table 3: Example 2: Problem parameterization

Geometry (data in m)	Physical properties
$P_1 := (0.55, 0.00)$	$E = 2 \cdot 10^{11}$ Pa
$P_2 := (0.45, 0.45)$	$\nu = 0.2$
$P_3 := (0.45, 0.55)$	$\rho = 8 \cdot 10^3$ kg/m <sup>3</sup>
$P_4 := (0.45, 1.45)$	$a_1 = 0$ s <sup>-1</sup>
$P_5 := (0.55, 1.55)$	$a_2 = 1 \cdot 10^{-5}$ s
$P_6 := (-0.55, 1.55)$	$T = 2 \cdot 10^{-3}$ s
$P_7 := (-0.45, 1.45)$	
$\Gamma_g := \{-0.55\} \times (1.45, 1.55)$	
External load	
$g_{\text{max}} = 10^8$ Pa	
$t_g = 2 \cdot 10^{-4}$ s	

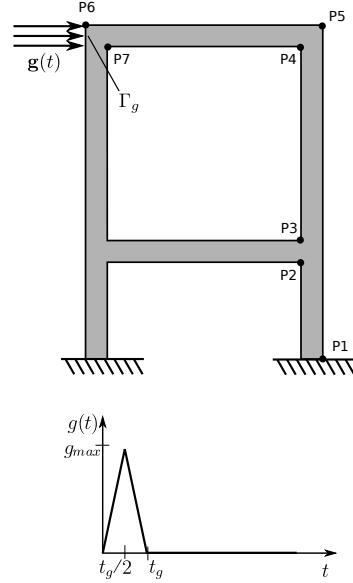


Figure 11: Example 2: Problem statement (top) and time dependent loading at  $\Gamma_g$  (bottom).

This example focusses in the quantity of interest

$$L^{\mathcal{O}}(\mathbf{W}) := \frac{1}{\text{meas}(\Gamma_g)} (\mathbf{e}_1, \mathbf{w}_u(T))_{\Gamma_g}, \quad (31)$$

which is the average of the final displacement in the boundary  $\Gamma_g$  where the external load is applied. Note that this quantity is not accounted in the generic quantity of interest given in equation (10). Consequently, quantity (31) is rewritten as

$$L^{\mathcal{O}}(\mathbf{W}) = a(\mathbf{u}^{\mathcal{O}}, \mathbf{w}_u(T)),$$

where  $\mathbf{u}^\mathcal{O}$  is the *exact* solution of the static linear elasticity problem: find  $\mathbf{u}^\mathcal{O} \in \mathcal{V}_0$  such that

$$a(\mathbf{u}^\mathcal{O}, \mathbf{w}) = \frac{1}{\text{meas}(\Gamma_g)} (\mathbf{e}_1, \mathbf{w})_{\Gamma_g}, \quad \forall \mathbf{w} \in \mathcal{V}_0. \quad (32)$$

After this reformulation, the quantity of interest is a particular case of the ones included in (10) and therefore the associated adjoint problem has the same structure as the original one. In particular, the function  $\mathbf{u}^\mathcal{O}$  is the final displacement condition for the adjoint problem. The other forcing data for the adjoint are zero in this case, namely  $\mathbf{v}^\mathcal{O} = \mathbf{f}^\mathcal{O} = \mathbf{g}^\mathcal{O} = \mathbf{0}$ . Note that function  $\mathbf{u}^\mathcal{O}$  is solution of an infinite dimensional problem and therefore it is unknown. In this example, the unknown function  $\mathbf{u}^\mathcal{O}$  is replaced by the computable one  $\tilde{\mathbf{u}}^\mathcal{O}$  obtained by solving problem (32) in the discrete space  $\mathcal{V}_0^{H,p+1}(\mathcal{P}_{\text{bg}})$  associated with the background mesh of the adaptive process. Three different background meshes are used in this example, see figure 12.

The quantity of interest (31) is well suited for the modal based approach because the weighting function  $\tilde{\mathbf{u}}^\mathcal{O}$  is well captured by the expansion of few eigenvectors. This ensures that the adjoint solution is also properly represented using few vibration modes. The projection of  $\tilde{\mathbf{u}}^\mathcal{O}$  into the expansion of the first  $M$  eigenvectors is defined as

$$\tilde{\mathbf{u}}^{\mathcal{O},M} := \sum_{i=1}^M \tilde{\mathbf{u}}_i \tilde{\mathbf{q}}_i,$$

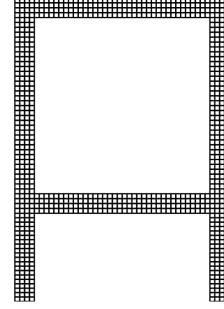
where  $\tilde{u}_i := m(\tilde{\mathbf{u}}^\mathcal{O}, \tilde{\mathbf{q}}_i)$ ,  $i = 1, \dots, M$ . Thus, the relative error in the projection is

$$\varepsilon^M := \frac{\|\tilde{\mathbf{u}}^\mathcal{O} - \tilde{\mathbf{u}}^{\mathcal{O},M}\|_m}{\|\tilde{\mathbf{u}}^\mathcal{O}\|_m},$$

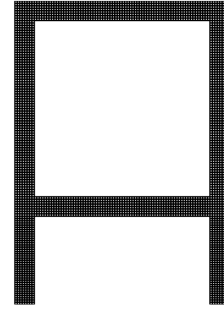
where  $\|\cdot\|_m := (m(\cdot, \cdot))^{1/2}$ . Figure 13 shows the error  $\varepsilon^M$  as a function of the number of eigenvectors  $M$ . Note that the error  $\varepsilon^M$  rapidly decreases as  $M$  increases. The number of eigenvectors considered in this example is  $M = 60$  and the associated projection error is  $\varepsilon^{60} = 5.94 \cdot 10^{-5}$ .

The exact value of the quantity of interest is unknown in this example. An overkill approximation of the quantity of interest,  $s^{\text{ovk}} := 1.2086 \cdot 10^{-3}$  m, is computed using a finite element mesh of  $N^{\text{el}} = 204800$  elements and  $N = 6400$  time steps. This discretization is the richest one considered in this example.

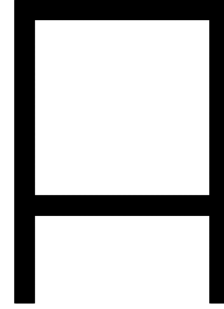
Figures 14 and 15 show snapshots of the computed solution and the computational mesh at several time points. This particular solution is obtained using the background mesh number 2, taking  $N^{\text{bk}} = 10$  time blocks and prescribing the error to the value  $s_{\text{tol}}^e = 5 \cdot 10^{-6}$  m. The coefficients used to split the total error budget into space and time are  $\alpha_s = 0.9$  and  $\alpha_t = 0.1$  and the unrefinement factors are taken as  $\beta_s = 0.5$  and  $\beta_t = 0.1$ . The computed quantity of interest is  $\tilde{s} = 1.2069 \cdot 10^{-3}$  m and the assessed error is  $\tilde{s}^e = 8.8942 \cdot 10^{-7}$  m. Note that the restriction  $|\tilde{s}^e| \leq s_{\text{user}}^e$  is also fulfilled in this example. Moreover, the error with respect the overkill solution,  $s_{\text{ovk}}^e = 1.7516 \cdot 10^{-6}$  m, is also below the user-defined value  $s_{\text{tol}}^e$ .



Background mesh 1



Background mesh 2



Background mesh 3

Figure 12: Example 2: Background meshes used in this example. The number of elements in each of them is 800, 3200 and 12800 respectively.

Figure 16 shows the history of number of elements in the computational mesh and the time step length for this particular computation. Note that the number of mesh elements increases in time because the stress waves spread in the structure. Note also that the time step length is smaller at the beginning of the computation due to the effect of the external load acting at the initial simulation time. Figure 16 also shows the number of iterations until achieve convergence in each time block. Note that the number of iterations is always equal or less than four.

The performance of the adaptive strategy is compared with respect to uniform mesh refinement. To this end, the uniform refined computations are obtained using the meshes plotted in figure 12 and three different number

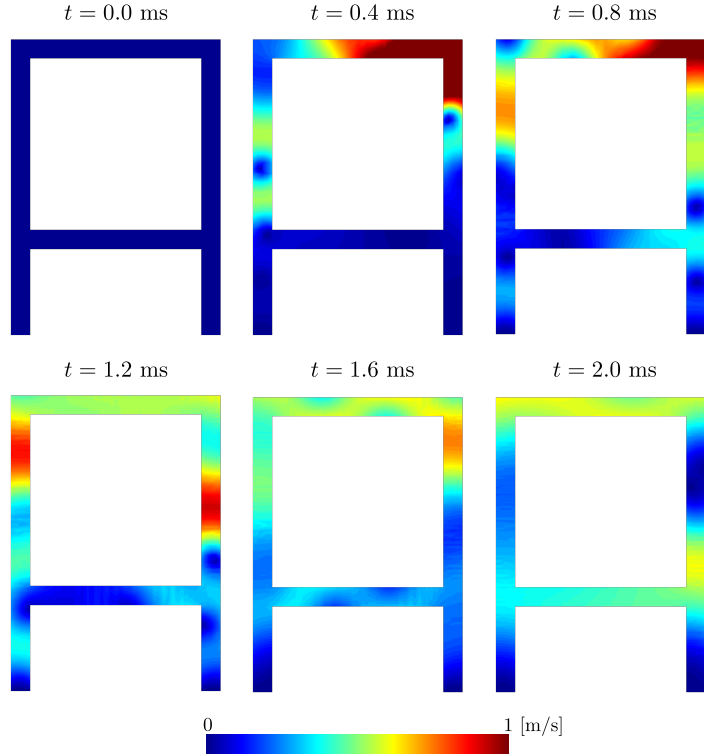


Figure 14: Example 2: Snapshots of the computed solution (magnitude of velocities in m/s) at several time points.

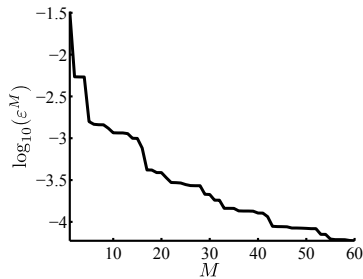


Figure 13: Example 2: Error in projecting the weighting function  $\tilde{\mathbf{u}}$  into the expansion of the first  $M$  eigenvectors  $\tilde{\mathbf{q}}_1, \dots, \tilde{\mathbf{q}}_M$ . The eigenvectors  $\tilde{\mathbf{q}}_i$  and the weighting function  $\tilde{\mathbf{u}}^{\mathcal{O}}$  are computed in the space  $\mathcal{V}_0^{H,p+1}(\mathcal{P}_{\text{bg}})$  associated with the background mesh number 2 plotted in figure 12.

of time steps  $N$ , see table 4. Note that the ratio  $H/\Delta t$  is also kept constant in this example to ensure that the space and time errors are reduced at the same rate. On the other hand, the adapted solutions are obtained using  $N^{\text{bk}} = 10$  and four different values of the prescriber error,  $s_{\text{tol}}^e = 5 \cdot 10^{-5}, 1 \cdot 10^{-5}, 5 \cdot 10^{-6}$  and  $1 \cdot 10^{-6}$  m. The dependence of the results on the chosen background mesh is studied by computing the adaptive solutions using the three background meshes plotted in figure 12. Twelve

adaptive solutions are computed all together (one for each value of the prescribed error and one for each background mesh).

Table 4: Example 2: Space and time discretizations for the three uniform solutions.

	$N^{\text{el}}$	# nodes	$N$
1	200	300	200
2	800	1000	400
3	3200	3600	800
4	12800	13600	1600

Table 5 and figure 17 and give the results for the adaptive and non-adaptive solutions. The convergence curves in figure 17 shows that the adapted solutions achieve a smaller error than the non-adapted solutions for the same number of space-time elements  $N^{\text{cells}}$ . The effectivity of the error estimate, namely  $\tilde{s}^e/s^e$ , is also shown in figure 17. Note that the computed effectivity (i.e. the quality of the error estimate) is better the finer is the background mesh. This is because the adjoint problem and the extractor  $\tilde{\mathbf{u}}^{\mathcal{O}}$  are computed using the background mesh. Thus, the finer the background mesh, the better the quality of the adjoint and, therefore, the better the quality of the error estimate. Note that the computed effectivities in this example are slightly worse than the ones obtained in the first numeri-

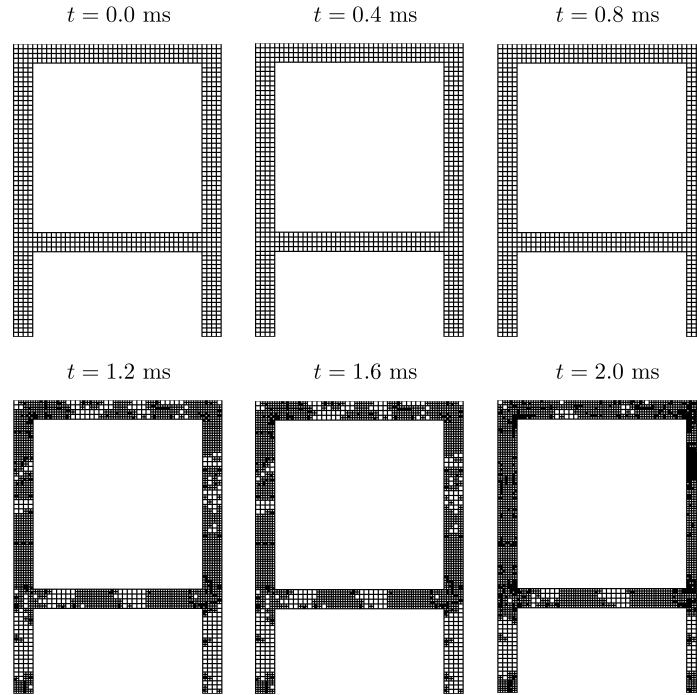


Figure 15: Example 2: Snapshots of the computational mesh at several time points.

cal example. Even though, the adaptive computations give more accurate results than the non-adapted solutions for the same number of space-time elements.

## 6 Closure

This article presents a goal-oriented space-time adaptive methodology for linear elastodynamics. The strategy aims at computing an optimal space-time discretization such that the numerical solution has an error in the quantity of interest below some user-defined tolerance. The space-time adaptation is driven by a goal-oriented error estimate that requires approximating an auxiliary adjoint problem.

The major novelty of this work is computing the adjoint solution with modal analysis instead of the standard direct time-integration methods. The modal-based approach is particularly efficient for some quantities of interest, because it allows to efficiently compute and store the adjoint solution.

The numerical examples show that the proposed strategy furnishes adapted solutions fulfilling the user-defined error tolerance. That is, both the assessed and computed errors are below the user-defined error value. Moreover, the discretizations obtained with the proposed adaptive strategy are more efficient than the ones obtained with a uniform refinement of all mesh elements and time steps.

The adaptive discretizations provide more accurate results than uniform remeshing, for the same number of space-time elements.

The proposed error estimate accounts for both the space and time discretization errors. The global error estimate is split into two contributions corresponding to the space and time errors using the Galerkin orthogonality property of the residual. This applies for space-time finite elements like time-continuous Galerkin methods. The extension of the approach to tackle other time-integration schemes, e.g. the ones based on finite differences and/or explicit methods with lumped mass matrix, requires further investigation.

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## A Linear system to be solved at each time step

This appendix details how the time-continuous Galerkin approximation is computed when the space mesh changes

Table 5: Example 2: Performance of the estimate for both the uniform and adaptive strategies (for four different background meshes). The overkill value for the quantity of interest  $s^{\text{ovk}} = 1.2086 \cdot 10^{-3}$  m is obtained using a uniform spatial mesh of  $N^{\text{cell}} = 1310720000$  space-time elements.

	$s_{\text{tol}}^e$ [m]	$N^{\text{cell}}$	$\tilde{s}$ [m]	$\tilde{s}^e$ [m]	$s_{\text{ovk}}^e$ [m]	$\tilde{s}^e/s_{\text{ovk}}^e$
uniform	–	40000	$1.1914 \cdot 10^{-3}$	$1.4558 \cdot 10^{-5}$	$1.7262 \cdot 10^{-5}$	0.843
	–	320000	$1.2037 \cdot 10^{-3}$	$3.6897 \cdot 10^{-6}$	$4.8973 \cdot 10^{-6}$	0.753
	–	2560000	$1.2068 \cdot 10^{-3}$	$1.3679 \cdot 10^{-6}$	$1.8601 \cdot 10^{-6}$	0.735
	–	20480000	$1.2079 \cdot 10^{-3}$	$5.6641 \cdot 10^{-7}$	$7.0564 \cdot 10^{-7}$	0.802
bg. mesh 1	$5 \cdot 10^{-5}$	28696	$1.2048 \cdot 10^{-3}$	$1.4506 \cdot 10^{-6}$	$3.8845 \cdot 10^{-6}$	0.373
	$1 \cdot 10^{-5}$	86181	$1.2068 \cdot 10^{-3}$	$3.0110 \cdot 10^{-7}$	$1.8337 \cdot 10^{-6}$	0.164
	$5 \cdot 10^{-6}$	153536	$1.2067 \cdot 10^{-3}$	$4.2316 \cdot 10^{-7}$	$1.9152 \cdot 10^{-6}$	0.220
	$1 \cdot 10^{-6}$	239936	$1.2067 \cdot 10^{-3}$	$5.8772 \cdot 10^{-7}$	$1.9421 \cdot 10^{-6}$	0.302
bg. mesh 2	$5 \cdot 10^{-5}$	90400	$1.2025 \cdot 10^{-3}$	$4.9028 \cdot 10^{-6}$	$6.1079 \cdot 10^{-6}$	0.802
	$1 \cdot 10^{-5}$	113004	$1.2066 \cdot 10^{-3}$	$1.0330 \cdot 10^{-6}$	$2.0820 \cdot 10^{-6}$	0.496
	$5 \cdot 10^{-6}$	174672	$1.2069 \cdot 10^{-3}$	$8.8942 \cdot 10^{-7}$	$1.7516 \cdot 10^{-6}$	0.507
	$1 \cdot 10^{-6}$	1212673	$1.2079 \cdot 10^{-3}$	$1.5956 \cdot 10^{-7}$	$7.4439 \cdot 10^{-7}$	0.214
bg. mesh 3	$5 \cdot 10^{-5}$	368000	$1.2056 \cdot 10^{-3}$	$2.5760 \cdot 10^{-6}$	$3.0675 \cdot 10^{-6}$	0.839
	$1 \cdot 10^{-5}$	380800	$1.2063 \cdot 10^{-3}$	$1.8447 \cdot 10^{-6}$	$2.3364 \cdot 10^{-6}$	0.789
	$5 \cdot 10^{-6}$	435724	$1.2071 \cdot 10^{-3}$	$1.3130 \cdot 10^{-6}$	$1.5426 \cdot 10^{-6}$	0.851
	$1 \cdot 10^{-6}$	3024438	$1.2083 \cdot 10^{-3}$	$1.5152 \cdot 10^{-7}$	$3.7024 \cdot 10^{-7}$	0.409
bg. mesh 4	$5 \cdot 10^{-5}$	1472000	$1.2065 \cdot 10^{-3}$	$1.9816 \cdot 10^{-6}$	$2.1207 \cdot 10^{-6}$	0.934
	$1 \cdot 10^{-5}$	1523200	$1.2073 \cdot 10^{-3}$	$1.1698 \cdot 10^{-6}$	$1.3089 \cdot 10^{-6}$	0.893
	$5 \cdot 10^{-6}$	1676800	$1.2077 \cdot 10^{-3}$	$7.8304 \cdot 10^{-7}$	$9.2219 \cdot 10^{-7}$	0.849
	$1 \cdot 10^{-6}$	4461564	$1.2084 \cdot 10^{-3}$	$1.8781 \cdot 10^{-7}$	$2.2230 \cdot 10^{-7}$	0.844

between times slabs.

Recall that the numerical approximation  $\tilde{\mathbf{U}}$  solution of the discrete problem (5) is computed sequentially starting from the first time slab  $I_1$  until the last one  $I_N$ . Specifically, assuming that the solution at the time-slab  $I_{n-1}$  is known, the approximation  $\tilde{\mathbf{U}}$  restricted to the slab  $I_n$  is found solving the problem: find  $\tilde{\mathbf{U}}|_{I_n} \in \mathcal{W}_u^{H,\Delta t}|_{I_n} \times \mathcal{W}_v^{H,\Delta t}|_{I_n}$  such that

$$\begin{aligned} \int_{I_n} \left[ m(\dot{\tilde{\mathbf{u}}}_v + a_1 \tilde{\mathbf{u}}_v, \mathbf{w}_v) + a(\tilde{\mathbf{u}}_u + a_2 \tilde{\mathbf{u}}_v, \mathbf{w}_v) \right] dt \\ = \int_{I_n} l(t; \mathbf{w}_v) dt, \quad \forall \mathbf{w}_v \in \mathcal{V}_0^H(\mathcal{P}_n), \end{aligned} \quad (33a)$$

$$\int_{I_n} a(\dot{\tilde{\mathbf{u}}}_u - \dot{\tilde{\mathbf{u}}}_v, \mathbf{w}_u) dt = 0, \quad \forall \mathbf{w}_u \in \mathcal{V}_0^H(\mathcal{P}_n), \quad (33b)$$

$$\tilde{\mathbf{U}}(t_{n-1}^+) = \tilde{\mathbf{U}}(t_{n-1}), \quad (33c)$$

where, for  $n > 1$ ,  $\tilde{\mathbf{U}}(t_{n-1})$  is the solution at the end of the previous interval  $I_{n-1}$  and, for  $n = 1$ ,  $\tilde{\mathbf{U}}(t_{n-1} = t_0)$  is defined using the initial conditions,  $\tilde{\mathbf{U}}(t_0) = [\mathbf{u}_0, \mathbf{v}_0]$ .

From the definition of the discrete spaces  $\mathcal{W}_u^{H,\Delta t}$  and  $\mathcal{W}_v^{H,\Delta t}$ , the numerical displacements and velocities  $\tilde{\mathbf{u}}_u$  and  $\tilde{\mathbf{u}}_v$  inside the interval  $I_n$  are expressed as a combination of the values at times  $t_{n-1}$  and  $t_n$ , namely

$$\tilde{\mathbf{u}}_u|_{I_n} = \tilde{\mathbf{u}}_u(t_{n-1})\theta_{n-1}(t) + \tilde{\mathbf{u}}_u(t_n)\theta_n(t), \quad (34a)$$

$$\tilde{\mathbf{u}}_v|_{I_n} = \tilde{\mathbf{u}}_v(t_{n-1})\theta_{n-1}(t) + \tilde{\mathbf{u}}_v(t_n)\theta_n(t). \quad (34b)$$

Thus, using the initial conditions for the interval (33c), the values  $\tilde{\mathbf{u}}_u(t_{n-1})$  and  $\tilde{\mathbf{u}}_v(t_{n-1}) \in \mathcal{V}_0^H(\mathcal{P}_{n-1})$  are known and the only unknowns to be determined are  $\tilde{\mathbf{u}}_u(t_n)$  and  $\tilde{\mathbf{u}}_v(t_n) \in \mathcal{V}_0^H(\mathcal{P}_n)$ . These unknowns are found inserting the representation (34) in equation (33) and noting that the following properties of the time-shape functions hold,

$$\int_{I_n} \theta_{n-1}(t) dt = \int_{I_n} \theta_n(t) dt = \frac{\Delta t_n}{2} \quad \text{and}$$

$$-\int_{I_n} \dot{\theta}_{n-1}(t) dt = \int_{I_n} \dot{\theta}_n(t) dt = 1.$$

Specifically,  $[\tilde{\mathbf{u}}_u(t_n), \tilde{\mathbf{u}}_v(t_n)] \in \mathcal{V}_0^H(\mathcal{P}_n) \times \mathcal{V}_0^H(\mathcal{P}_n)$  is such that

$$\begin{aligned} m(\tilde{\mathbf{u}}_v(t_n), \mathbf{w}_v) + \frac{\Delta t_n}{2} c(\tilde{\mathbf{u}}_v(t_n), \mathbf{w}_v) + \frac{\Delta t_n}{2} a(\tilde{\mathbf{u}}_u(t_n), \mathbf{w}_v) \\ = l_{v,n}(\mathbf{w}_v), \quad \forall \mathbf{w}_v \in \mathcal{V}_0^H(\mathcal{P}_n), \end{aligned} \quad (35a)$$

and

$$\begin{aligned} a(\tilde{\mathbf{u}}_u(t_n), \mathbf{w}_u) - \frac{\Delta t_n}{2} a(\tilde{\mathbf{u}}_v(t_n), \mathbf{w}_u) \\ = l_{u,n}(\mathbf{w}_u), \quad \forall \mathbf{w}_u \in \mathcal{V}_0^H(\mathcal{P}_n), \end{aligned} \quad (35b)$$

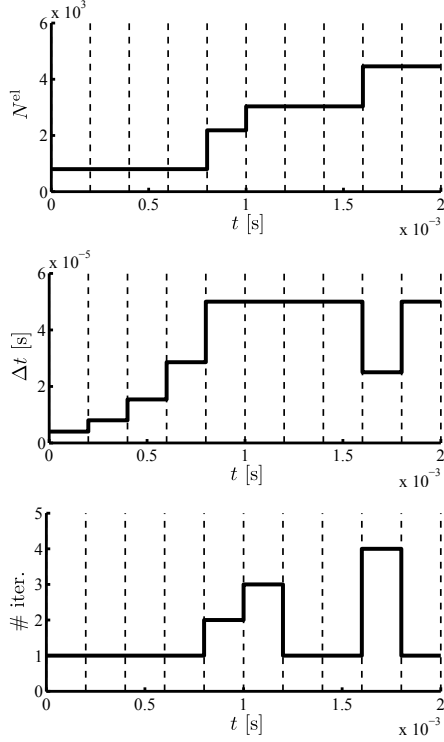


Figure 16: Example 2: Evolution along the adaptive process of the number of elements (top) and the time step (center). Number of remeshing iterations to achieve convergence in each block (bottom).

where

$$\begin{aligned}
 l_{v,n}(\mathbf{w}) &:= \int_{I_n} l(t; \mathbf{w}) \, dt + m(\tilde{\mathbf{u}}_v(t_{n-1}), \mathbf{w}) \\
 &\quad - \frac{\Delta t_n}{2} c(\tilde{\mathbf{u}}_v(t_{n-1}), \mathbf{w}) - \frac{\Delta t_n}{2} a(\tilde{\mathbf{u}}_u(t_{n-1}), \mathbf{w}), \\
 l_{u,n}(\mathbf{w}) &:= a(\tilde{\mathbf{u}}_u(t_{n-1}), \mathbf{w}) + \frac{\Delta t_n}{2} a(\tilde{\mathbf{u}}_v(t_{n-1}), \mathbf{w}), \\
 c(\mathbf{v}, \mathbf{w}) &:= m(a_1 \mathbf{v}, \mathbf{w}) + a(a_2 \mathbf{v}, \mathbf{w}).
 \end{aligned}$$

Note that since the values  $\tilde{\mathbf{u}}_u(t_{n-1})$  and  $\tilde{\mathbf{u}}_v(t_{n-1})$  are known, the terms associated with this values are placed in the right hand side of the equations.

The computation of the terms appearing in the left hand side of (35) entails no difficulty since all the spatial functions belong to  $\mathcal{V}_0^H(\mathcal{P}_n)$ . On the contrary, if different spatial computational meshes are used at times  $t_{n-1}$  and  $t_n$ , the computation of the nodal force vectors associated with  $l_{u,n}(\cdot)$  and  $l_{v,n}(\cdot)$  involves computing mass and energy products of functions defined in the mesh at time  $t_{n-1}$  and functions defined in the mesh at time  $t_n$ , e.g.  $m(\tilde{\mathbf{u}}_v(t_{n-1}), \mathbf{w}_v)$ .

The use of different spatial meshes is efficiently handled by solving the discrete problem (35) using the auxiliary *union* mesh  $\mathcal{P}_{n-1,n}$  containing in each zone of the domain

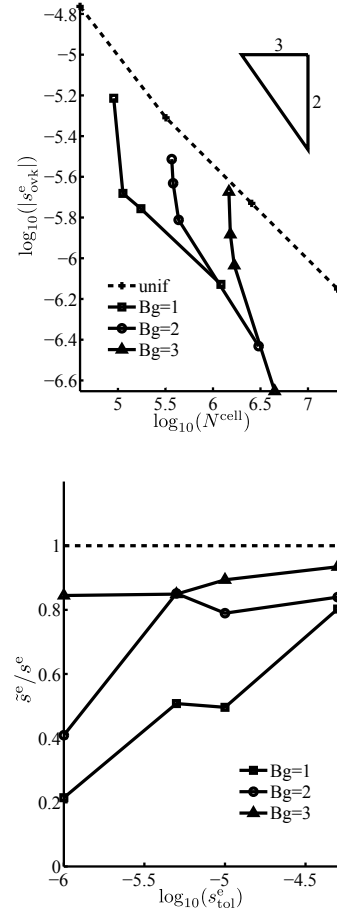


Figure 17: Example 2: Error convergence for the adapted and uniform computations (top) and computed effectivity of the error estimate (bottom). The adapted solutions are obtained using three background meshes.

the finer elements either in  $\mathcal{P}_{n-1}$  or  $\mathcal{P}_n$ , see figure 18, namely

$$\mathcal{P}_{n-1,n} := \{\omega = \Delta \cap \Delta' \text{ for } \Delta \in \mathcal{P}_{n-1}, \Delta' \in \mathcal{P}_n\}.$$

Note that, any function belonging either to  $\mathcal{V}_0^H(\mathcal{P}_{n-1})$  or  $\mathcal{V}_0^H(\mathcal{P}_n)$  can be represented in the finite element space associated to  $\mathcal{P}_{n-1,n}$ , namely  $\mathcal{V}_0^H(\mathcal{P}_{n-1,n})$ , without loss of information. Thus, the products involving functions in different meshes are efficiently computed after projecting the functions in the space  $\mathcal{V}_0^H(\mathcal{P}_{n-1,n})$ . However, discretizing problem (35) using the mesh  $\mathcal{P}_{n-1,n}$  requires introducing additional constraints to enforce that the computed fields  $\tilde{\mathbf{u}}_u(t_n)$  and  $\tilde{\mathbf{u}}_v(t_n)$  belong to  $\mathcal{V}_0^H(\mathcal{P}_n)$  and not to  $\mathcal{V}_0^H(\mathcal{P}_{n-1,n})$ . That is, problem (35) leads to the following system of equations when discretized in the auxiliary finite



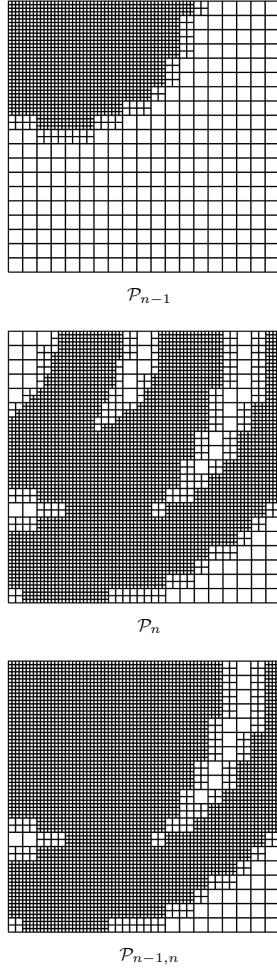


Figure 18: Illustration of the computational meshes  $\mathcal{P}_{n-1}$ ,  $\mathcal{P}_n$  and their union  $\mathcal{P}_{n-1,n}$ .

element mesh  $\mathcal{P}_{n-1,n}$ :

$$\begin{bmatrix} \mathbf{K}_n & -\frac{\Delta t_n}{2} \mathbf{K}_n & \mathbf{A}_n^T & \mathbf{0} \\ \frac{\Delta t_n}{2} \mathbf{K}_n & \mathbf{M}_n + \frac{\Delta t_n}{2} \mathbf{C}_n & \mathbf{0} & \mathbf{A}_n^T \\ \mathbf{A}_n & \mathbf{0} & \mathbf{0} & \mathbf{0} \\ \mathbf{0} & \mathbf{A}_n & \mathbf{0} & \mathbf{0} \end{bmatrix} \begin{bmatrix} \mathbf{U}_{u,n} \\ \mathbf{U}_{v,n} \\ \boldsymbol{\lambda}_{u,n} \\ \boldsymbol{\lambda}_{v,n} \end{bmatrix} = \begin{bmatrix} \mathbf{F}_{u,n} \\ \mathbf{F}_{v,n} \\ \mathbf{0} \\ \mathbf{0} \end{bmatrix} \quad (36)$$

where

$$\mathbf{F}_{u,n} := \mathbf{K}_n \mathbf{U}_{u,n-1} + \frac{\Delta t_n}{2} \mathbf{K}_n \mathbf{U}_{v,n-1},$$

$$\mathbf{F}_{v,n} := \left( \mathbf{M}_n - \frac{\Delta t_n}{2} \mathbf{C}_n \right) \mathbf{U}_{v,n-1} - \frac{\Delta t_n}{2} \mathbf{K}_n \mathbf{U}_{u,n-1} + \int_{I_n} \mathbf{F}(t) dt,$$

and  $\mathbf{C}_n := a_1 \mathbf{M}_n + a_2 \mathbf{K}_n$ . The matrices  $\mathbf{M}_n$  and  $\mathbf{K}_n$  and the vector  $\mathbf{F}(t)$  are the discrete counterparts of the bilinear forms  $m(\cdot, \cdot)$  and  $a(\cdot, \cdot)$  and the linear form  $l(t; \cdot)$  in the space  $\mathcal{V}_0^H(\mathcal{P}_{n-1,n})$  and the vectors  $\mathbf{U}_{u,n}$ ,  $\mathbf{U}_{v,n}$ ,  $\mathbf{U}_{u,n-1}$

and  $\mathbf{U}_{v,n-1}$  contain the degrees of freedom of functions  $\tilde{\mathbf{u}}_u(t_n)$ ,  $\tilde{\mathbf{u}}_v(t_n)$ ,  $\tilde{\mathbf{u}}_u(t_{n-1})$  and  $\tilde{\mathbf{u}}_v(t_{n-1})$  expressed in the discrete space  $\mathcal{V}_0^H(\mathcal{P}_{n-1,n})$ . Note that the linear constraints  $\mathbf{A}_n \mathbf{U}_{u,n} = \mathbf{0}$  and  $\mathbf{A}_n \mathbf{U}_{v,n} = \mathbf{0}$  are introduced in order to ensure that the computed fields  $\tilde{\mathbf{u}}_u(t_n)$  and  $\tilde{\mathbf{u}}_v(t_n)$  belong to  $\mathcal{V}_0^H(\mathcal{P}_n)$  and also to impose continuity of the solution at the hanging nodes, see figure 19. The vectors  $\boldsymbol{\lambda}_{u,n}$  and  $\boldsymbol{\lambda}_{v,n}$  are the associated Lagrange multipliers.

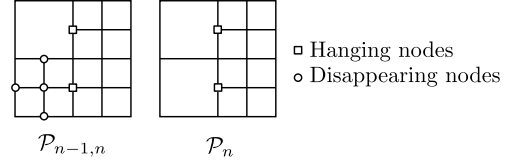


Figure 19: The numerical solution is constrained at the nodes of the mesh  $\mathcal{P}_{n-1,n}$  corresponding to hanging nodes in the mesh  $\mathcal{P}_n$  and also at the nodes of  $\mathcal{P}_{n-1,n}$  which disappear in mesh  $\mathcal{P}_n$ .

Note that system (36) is at the first sight of double size than the one associated with the Newmark method. However, system (36) can be rewritten in a more convenient way by subtracting to the second row of the matrix in (36) the first row multiplied by  $\frac{\Delta t_n}{2}$ . That is,

$$\begin{bmatrix} \mathbf{K}_n & -\frac{\Delta t_n}{2} \mathbf{K}_n & \mathbf{A}_n^T & \mathbf{0} \\ \mathbf{0} & \mathbf{M}_n + \frac{\Delta t_n}{2} \mathbf{C}_n + \frac{\Delta t_n^2}{4} \mathbf{K}_n & -\frac{\Delta t_n}{2} \mathbf{A}_n^T & \mathbf{A}_n^T \\ \mathbf{A}_n & \mathbf{0} & \mathbf{0} & \mathbf{0} \\ \mathbf{0} & \mathbf{A}_n & \mathbf{0} & \mathbf{0} \end{bmatrix} \begin{bmatrix} \mathbf{U}_{u,n} \\ \mathbf{U}_{v,n} \\ \boldsymbol{\lambda}_{u,n} \\ \boldsymbol{\lambda}_{v,n} \end{bmatrix} = \begin{bmatrix} \mathbf{F}_{u,n} \\ \mathbf{F}_{v,n} - \frac{\Delta t_n}{2} \mathbf{F}_{u,n} \\ \mathbf{0} \\ \mathbf{0} \end{bmatrix}.$$

This reformulation allows to compute the velocities separately from the displacements solving a system of the same size as the usual system arising in the Newmark method, namely,

$$\begin{bmatrix} \mathbf{M}_n + \frac{\Delta t_n}{2} \mathbf{C}_n + \frac{\Delta t_n^2}{4} \mathbf{K}_n & \mathbf{A}_n^T \\ \mathbf{A}_n & \mathbf{0} \end{bmatrix} \begin{bmatrix} \mathbf{U}_{v,n} \\ \boldsymbol{\lambda}_n^* \end{bmatrix} = \begin{bmatrix} \mathbf{F}_{v,n} - \frac{\Delta t_n}{2} \mathbf{F}_{u,n} \\ \mathbf{0} \end{bmatrix},$$

with  $\boldsymbol{\lambda}_n^* := \boldsymbol{\lambda}_{v,n} - \frac{\Delta t_n}{2} \boldsymbol{\lambda}_{u,n}$ . Once the velocities  $\mathbf{U}_{v,n}$  are known, the displacements are obtained solving the system

$$\begin{bmatrix} \mathbf{K}_n & \mathbf{A}_n^T \\ \mathbf{A}_n & \mathbf{0} \end{bmatrix} \begin{bmatrix} \mathbf{U}_{u,n} \\ \boldsymbol{\lambda}_{u,n} \end{bmatrix} = \begin{bmatrix} \mathbf{F}_{u,n} + \frac{\Delta t_n}{2} \mathbf{K}_n \mathbf{U}_{v,n} \\ \mathbf{0} \end{bmatrix}. \quad (37)$$

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## Paper E

# An algorithm for mesh refinement and un-refinement in fast transient dynamics

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